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EVALUATION OF SILICA FUME HIGH DENSITY THIN BONDED OVERLAYS

**Interim Report
February 2001**

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16. ABSTRACT <p>In September 1999, Bridges "A" and "B" of contract TBOI-0035-1(110)044 were overlaid with a silica fume concrete surface. ODOT has been exploring ways to stop or slow chloride-induced corrosion of uncoated reinforcing steel in bridge decks. Silica fume concrete additive has demonstrated success in reducing chloride intrusion.</p> <p>After one year of testing, the ride quality decreased slightly, skid test results show an adequate amount of friction characteristics, and both bridges in good condition base on based on reinforcing steel corrosion activity. Overall, silica fume modified PCC overlay is performing well.</p>			
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SI (METRIC) CONVERSION FACTORS

<i>Approximate Conversions to SI Units</i>					<i>Approximate Conversions from SI Units</i>				
Symbol	When you know	Multiply by	To Find	Symbol	Symbol	When you know	Multiply by	To Find	Symbol
LENGTH					LENGTH				
in	inches	25.40	millimeters	mm	mm	millimeters	0.0394	inches	in
ft	feet	0.3048	meters	m	m	meters	3.281	feet	ft
yd	yards	0.9144	meters	m	m	meters	1.094	yards	yd
mi	miles	1.609	kilometers	km	km	kilometers	0.6214	miles	mi
AREA					AREA				
in ²	square inches	645.2	square millimeters	mm ²	mm ²	square millimeters	0.00155	square inches	in ²
ft ²	square feet	0.0929	square meters	m ²	m ²	square meters	10.764	square feet	ft ²
yd ²	square yards	0.8361	square meters	m ²	m ²	square meters	1.196	square yards	yd ²
ac	acres	0.4047	hectares	ha	ha	hectares	2.471	acres	ac
mi ²	square miles	2.590	square kilometers	km ²	km ²	square kilometers	0.3861	square miles	mi ²
VOLUME					VOLUME				
fl oz	fluid ounces	29.57	milliliters	mL	mL	milliliters	0.0338	fluid ounces	fl oz
gal	gallons	3.785	liters	L	L	liters	0.2642	gallons	gal
ft ³	cubic feet	0.0283	cubic meters	m ³	m ³	cubic meters	35.315	cubic feet	ft ³
yd ³	cubic yards	0.7645	cubic meters	m ³	m ³	cubic meters	1.308	cubic yards	yd ³
MASS					MASS				
oz	ounces	28.35	grams	g	g	grams	0.0353	ounces	oz
lb	pounds	0.4536	kilograms	kg	kg	kilograms	2.205	pounds	lb
T	short tons (2000 lb)	0.907	megagrams	Mg	Mg	megagrams	1.1023	short tons (2000 lb)	T
TEMPERATURE (exact)					TEMPERATURE (exact)				
°F	degrees Fahrenheit	(°F-32)/1.8	degrees Celsius	°C	°C	degrees Celsius	9/5+32	degrees Fahrenheit	°F
FORCE and PRESSURE or STRESS					FORCE and PRESSURE or STRESS				
lbf	poundforce	4.448	Newtons	N	N	Newtons	0.2248	poundforce	lbf
lbf/in ²	poundforce per square inch	6.895	kilopascals	kPa	kPa	kilopascals	0.1450	poundforce per square inch	lbf/in ²

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INTRODUCTION

In September 1999, the Oklahoma Department of Transportation (ODOT) placed silica fume modified PCC overlays on two bridges on I-35 in Carter County. The first year of testing was completed in October 2000.

ODOT has been exploring ways to stop or slow chloride-induced corrosion of uncoated reinforcing steel in bridge decks. One method widely used to prevent chloride intrusion is placement of thin, low permeability PCC overlays on the existing decks. Silica fume is a commonly used PCC additive where high density and low permeability is desired. Added benefits of overlays are improved smoothness and friction characteristics. Other agencies have reported that silica fume modified PCC overlays have demonstrated success in reducing chloride intrusion and restoring driving surfaces of bridge decks(2).

INVESTIGATION

Various researchers have expressed concern that the difficulty of curing silica fume mixes might lead to cracking, which in turn, can be a cause of delamination(1). Cracking of the overlay was measured and mapped. The following observations regarding cracking were made.

The south bound bridge has a longitudinal construction joint which travels the length of the project. The joint is located on the inside half of the outside (west) lane. A crack formed over this joint which has a maximum width of 6 mm ($\frac{1}{4}$ inch). All other cracks were randomly spaced over approximately 10 percent of the bridge deck of the southbound bridge. These are "hairline" cracks size in accordance with SHRP-3-338 manual, a fracture that is very narrow in width, less than 3mm ($\frac{1}{8}$ inch).

The north bound bridge also had a construction joint in the corresponding location (inside half of the outside, or east, lane). Maximum width of the crack over this joint was 3 mm ($\frac{1}{8}$ inch). Hairline cracks were randomly located over approximately 25 percent of the northbound bridge. Crack maps of each of the two bridges are included in Appendix A.

All four lanes of the two bridge decks were chain-dragged tested to detect the presence of delamination or debonding. Deck areas where the deck has a horizontal fracture plane give a hollow sound when a chain is dragged on their surface. Generally, it is not possible to distinguish between delamination and debonding using manual sounding techniques (3). The longitudinal construction joints on both bridges gave the hollow sound during chain drag testing. These were the only locations, on either bridge, where delamination or debonding was indicated. No debonding was noted at these locations during the testing done after completion of the overlays (September 1998), indicating that any delamination or debonding present has occurred since then.

Half-cell testing (ASTM C - 876 - 80) was done on both bridges to detect electrical potentials associated with corrosion. Locations where the individual half-cell potentials were measured, and the amounts of the readings, are shown in Appendix A. Half-cell readings are divided into three classes, based on the measured potential. These are listed below:

<u>Class</u>	<u>Potential(v)</u>
• Class "A"	- .250 to - .350
• Class "B"	-360 to - .400
• Class "C"	-.410 and more negative.

Seven percent of the readings on the southbound bridge and 2.9 percent of the readings done on the northbound bridge were in the Class "A" range. No readings were classified as Class "B" or "C".

Smoothness of the bridge was measured with an Ames 4000 - B profilograph and a K. J. Law T-6400 Lightweight Profilometer. Each lane of each bridge deck was measured with each of the instruments. Measurements from both the profilograph and the profilometer are listed in Appendix

B. On the southbound bridge, the profile index for the outside lane was 25.71 in/mi. and 17.41 in/mi. for the inside lane. The average profile index (both lanes) was 21.56 in/mi. The northbound bridge had a profile index of 20.00 in/mi in the outside lane and 7.92 in/mi in the inside lane. The average of both lanes on this bridge was 13.96 in/mi.

Skid trailer measurements were also taken. The skid trailer was able to get one measurement for each lane of each bridge. Skid trailer measurements are expressed as skid numbers (SN). Measured values were 43, 52, 42, 46. Skid data sheets are included in Appendix B.

The ODOT interstate maintenance yard for the area containing the two bridges used 5:1 sand to salt mixture for deicing bridge and highway surfaces. Thirty light applications of sand/salt mixture has been placed on each bridge since completion of the silica fume modified overlay. The mixture was applied at a rate of less than 20 lb per square yard.

CONCLUSION

One year of test data has been summarized in this report. Regarding chloride ion transport through concrete, the presence of interconnected pores, cracks, hairline cracks, and aggregate particles will affect the ability of the chloride ions to migrate into the concrete(4).

Hairline cracking was the major distress found on the bridge decks. The 1999 half-cell readings on the southbound bridge had 2.9 percent in the Class "A" category. By 2000, Class "A" readings had increased to 7.0 percent. Locations with hairline cracking and relatively high half-cell readings tend to occur in the same general areas, indicating that chloride intrusion through the hairline cracking is the likely cause of the higher readings.

The ride quality decreased slightly, skid test results show an adequate amount of friction

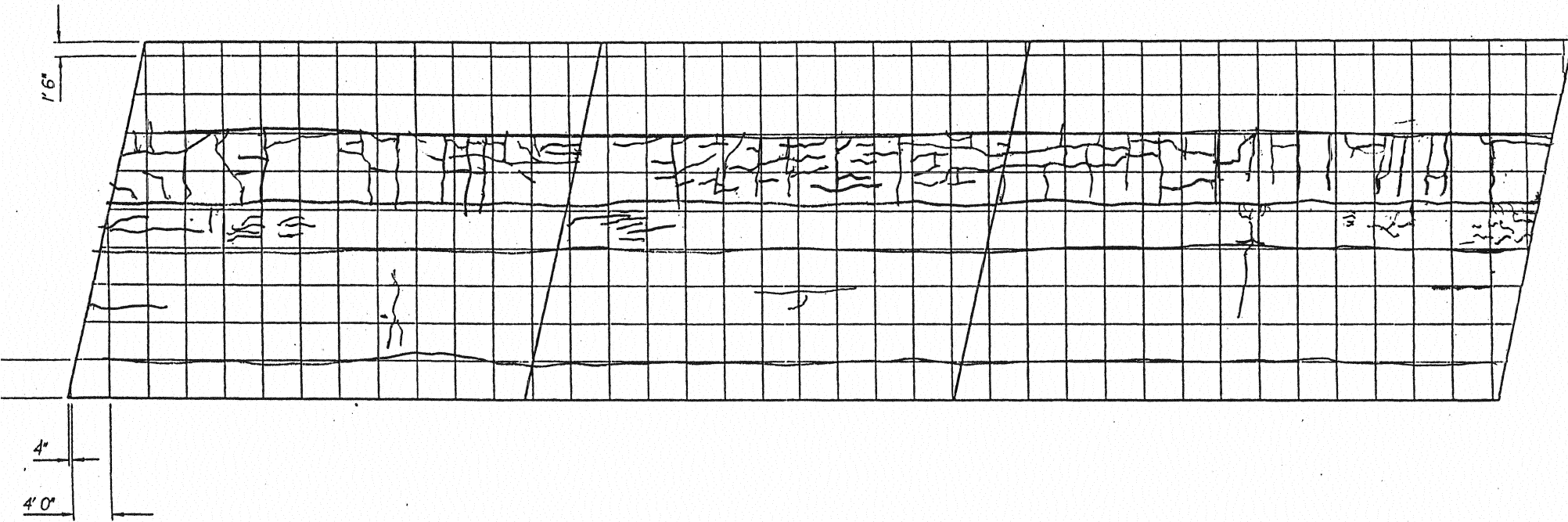
characteristics. Carter county had a mild winter during the last evaluation period (less freeze-thaw cycle than the rest of Oklahoma). Results of half-cell testing indicates that both bridges are in good condition, based on reinforcing steel corrosion activity. Overall, the two silica fume modified PCC overlays are in good condition and performing well.

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2. Wilson B. Brewer, Jr. and Gary Williams, "Evaluation Of Silica Fume High Density Thin Bonded Overlays-Construction Report". Research & Development Division, Oklahoma Department of Transportation, Oklahoma City, OK, March 2000.
3. Osama A. Abdulshafi and Micheal G. Fitch, "Field and Laboratory Evaluation of Silica Fume Modified Concrete Bridge Deck Overlays in Ohio", CTL Engineering, Inc, Ohio Department of Transportation, Columbus, OH, July 1995.
4. D. Whiting and R. Detwiler, "Silica Fume Concrete for Bridge Decks, National Cooperative Highway Research Program, Transportation Research Council, Washington D.C.. 1998.

APPENDIX A
CRACK MAPS

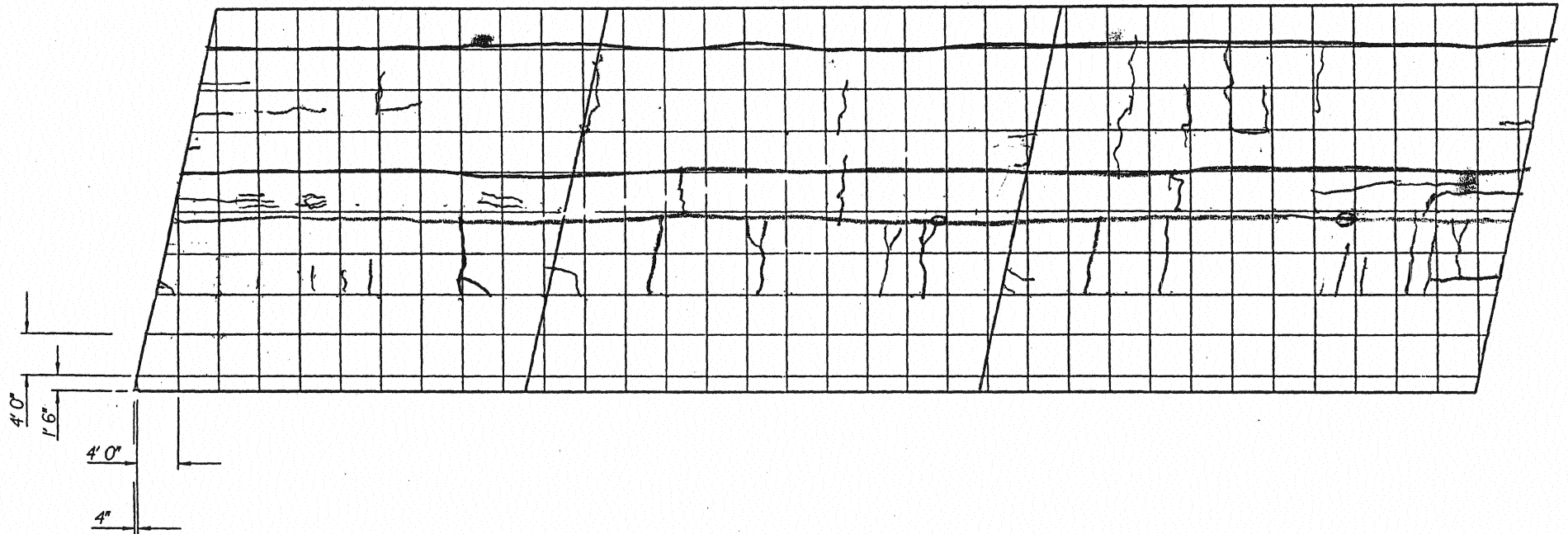
CRACK MAP OF SOUTH BOUND BRIDGE



A-1

SB

CRACK MAP OF NORTH BOUND BRIDGE



A-2

NB

APPENDIX B
HALF CELL TESTING

Oklahoma Department of Transportation

Research and Development Division

Date October 6,2000

To Wilson Brewer

From Bryan W. Cooper

Subject Half Cell testing and deck analysis on Silica Fume bridge project in Carter County
Item No. 2132

I have analyzed the results of the half-cell testing that was conducted on October 3, 2000. I have concluded from the results of this testing that there is very little corrosion in the reinforcing steel of these particular bridge decks.

Half-cell test readings are divided into three classes. They are Class "A", Class "B", and Class "C". The classification of these readings are specified within these following ranges:

Class "A": .250 - .350

Class "B": .360 - .400

Class "C": .410 and greater

A plan view of these bridge decks has been provided and displays the grid layout that was used in this testing and the readings that were gathered at each point.

The results of this analysis shows that there was only 2.9% Class "A" readings on the Northbound bridge and no Class "B" or "C" data. The testing on the Southbound bridge indicated 7.0% of Class "A" readings and no Class "B" or "C" indications, Class "C" being the most critical. The 1999 results indicated 2.6% for the Northbound and 4.2% for the Southbound.

It is my belief that your crack mapping results will indicate that the cracking in the southbound bridge, is a major contributor to the increase of the percentage of Class "A" readings on this bridge.

The results seem to indicate that we still have two bridges in good shape in reference to the corrosion of the reinforcing steel.

Cc: Gary Williams

CLASS "A" % : 7.0%
 CLASS "B" % : 0%
 CLASS "C" % : 0%

	218	218	171	191	167	192	096	224	185	147	153	154	117	097	085	180	121	137	107	135	253	130	113	141	122	086	138	115	138	152	103	154	147	118	091	087	095	096	240
242	218	176	257	203	292	174	213	224	185	161	165	178	130	174	248	262	249	202	160	199	191	204	245	185	177	255	234	250	247	221	237	203	186	193	187	257	234	240	
165	146	161	152	146	152	183	137	145	129	133	130	155	123	139	172	198	160	141	142	172	151	160	193	181	201	269	141	140	137	137	130	169	110	113	123	132	132	155	
249	189	248	176	123	127	165	142	143	151	157	159	189	163	166	180	169	201	183	153	211	270	270	230	188	215	198	148	145	127	142	110	137	114	118	192	124	137	159	
214	214	164	190	229	164	142	163	150	145	137	127	125	147	151	212	164	208	219	218	179	181	220	224	199	210	234	171	234	195	178	172	164	177	164	144	151	144	188	
265	198	211	156	136	132	155	141	142	113	116	149	121	134	164	137	149	159	283	193	198	210	342	195	215	278	230	191	213	206	262	248	170	210	216	198	226	186	198	
267	147	205	156	171	152	156	144	156	165	175	184	156	155	175	163	202	205	200	203	194	207	191	206	189	197	262	189	180	184	190	211	178	183	263	186	189	170	160	
233	160	217	170	142	194	169	143	218	181	182	226	168	142	156	276	201	254	221	244	264	167	254	205	236	172	230	179	217	200	230	198	230	273	260	173	209	143	158	
172	142	174	185	130	193	166	140	110	132	174	177	089	072	134	124	195	182	201	194	145	151	241	147	213	221	158	192	206	201	281	137	179	154	157	134	157	144	164	
144	144	218	136	223	127	173	180	175	203	199	142	146	101	077	171	122	129	122	168	207	163	181	269	181	285	196	160	155	215	169	238	140	133	171	183	217	194	203	

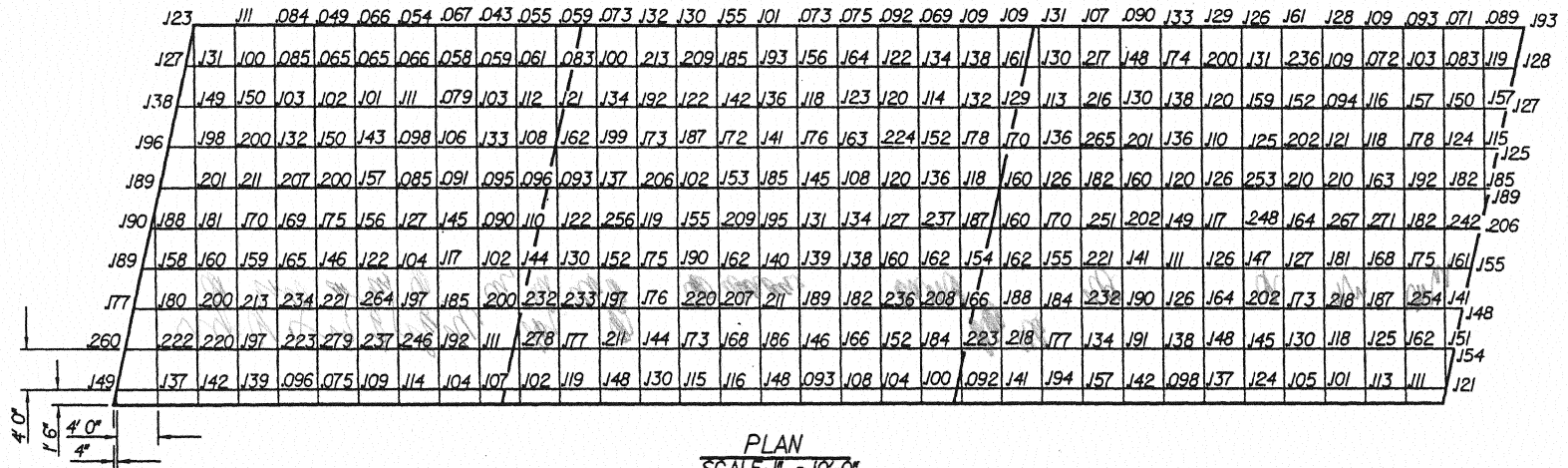
PLAN
 SCALE: 1" = 10'-0"

B-2

I-35 Southbound		Carter County	Design
SILICA FUME DECK ANALYSIS		Drawn	6
		Check	
		Exp.	
STATE OF OKLAHOMA		DEPARTMENT OF TRANSPORT	
		MAY 18 2/32 36	

CLASS "A" % : 29%
 CLASS "B" % : 0 %
 CLASS "C" % : 0 %

CLASS "A" : 25 - 35
 CLASS "B" : 36 - 40
 CLASS "C" : 41 and Above



B-3

I-35 Northbound		Carter County	Design
SILICA FUME DECK ANALYSIS			Field
			Check
STATE OF OKLAHOMA			Sign
			Exp.
DEPARTMENT OF TRANSPORTATION		FILE NO. 2132	13

APPENDIX C
PROFILE RESULTS

Oklahoma Department of Transportation
 Materials and Research Division

Date October 31, 2000

To Gary Williams

From Bryan K. Hurst *BKH*

Subject 2000 Silica Fume Overlay Bridge Profiles
 Item 2132

On Wednesday, October 25, 2000, profilograph and KJ-Law Profilometer testing was performed on the Silica Fume Overlay Bridges on I-35 near Ardmore, Ok. The two southbound lanes were tested first that morning and the two northbound lanes were tested that afternoon. New software has been installed on the KJ-Law Profilometer since the 1999 testing on these bridges, and with it comes a change in the filter wavelength setting from 300 to 100. This setting slightly changes roughness results making them appear smoother. The 1999 results were amended with the new software and were included in the "1999 Silica Fume Overlay Bridge Field Report."

Roughness figures were obtained from both the Ames Profilograph and the KJ-Law Profilometer, for 2000. The table below contains the project results for the 1999 testing and the results from the 2000 testing:

LOCATION	ROUGHNESS / INCHES PER MILE							
	1999				2000			
	Ames Profilograph		KJ-Law Profilometer		Ames Profilograph		KJ-Law Profilometer	
	TOTAL PROJECT	BRIDGE DECK	TOTAL PROJECT	BRIDGE DECK	TOTAL PROJECT	BRIDGE DECK	TOTAL PROJECT	BRIDGE DECK
SOUTHBOUND RT. LN. / LWP	20.43	27.14	25.27	32.93	31.90	25.71	21.83	31.91
SOUTHBOUND LT. LN. / RWP	12.49	17.5	11.79	18.44	10.00	17.14	8.61	16.31
NORTHBOUND RT. LN. / LWP	6.33	19.58	7.5	N/A	10.52	20.00	12.15	18.15
NORTHBOUND LT. LN. / RWP	5.21	7.92	6.02	7.57	6.13	7.92	5.00	4.76

APPENDIX D
SKID TEST DATA

SITE: 035-10-36 Carter
 LANE: NBOL
 DATE: 11/09/2000
 TIME: 15:29:11
 DRIVER: RMB
 OPERATOR: JAL

REF POST	SN	WHEEL	SN PEAK	AIR TEMP	TEST TIME	CYCLE NUMBER	EVENT
20.327	49.4	Left	86.9	49.4	15:28:57	510036068	
20.366	42.8	Right	84.8	49.1	15:29:00	510036069	Bridge
20.409	45.0	Left	79.1	49.1	15:29:04	510036070	

Left Wheel	N = 2	M = 47.2	SD = 3.11	H = 49.4	L = 45.0
Right Wheel	N = 1	M = 42.8	SD = 0.00	H = 42.8	L = 42.8
Total	N = 3	M = 45.7	SD = 3.37	H = 49.4	L = 42.8

SKID TEST DISCLAIMER

This test is conducted solely for the purpose of generating input data for priority programming of maintenance and construction projects. Tests are performed by field personnel not trained nor expert in scientific testing procedure. While every effort is made to conduct tests accurately, tests are not subject to rigorous scientific control. The test results are calculated as the product of a mechanical test wherein a skid trailer tire interfaces with the road surface providing an approximate value which may be converted to a coefficient of friction only for that portion of the road surface actually in contact with the tire of the test trailer. The calculated coefficient of friction has value only as to the surface actually tested and no attempt should be made to use this test as a means of evaluation of untested surface areas or for correlation of this test with tests of other tested surface areas.

TEST IS PERFORMED SOLEY FOR THE PURPOSES INDICATED AND NO REPRESENTATIONS AS TO ITS ACCURACY, RELIABILITY, OR APPLICABILITY FOR OTHER PURPOSES ARE EXPRESSED OR IMPLIED.

SITE: 035-10-36 Carter
 LANE: NBIL
 DATE: 11/09/2000
 TIME: 15:32:20
 DRIVER: RMB
 OPERATOR: JAL

REF POST	SN	WHEEL	SN PEAK	AIR TEMP	TEST TIME	CYCLE NUMBER	EVENT
20.294	53.7	Left	86.0	49.1	15:32:03	510036074	
20.253	46.1	Right	100.5	49.1	15:32:06	510036075	Bridge
20.213	52.0	Left	84.7	49.1	15:32:10	510036076	

Left Wheel	N = 2	M = 52.8	SD = 1.16	H = 53.7	L = 52.0
Right Wheel	N = 1	M = 46.1	SD = 0.00	H = 46.1	L = 46.1
Total	N = 3	M = 50.6	SD = 3.99	H = 53.7	L = 46.1

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SITE: 035-10-36 Carter
 LANE: SBOL
 DATE: 11/09/2000
 TIME: 15:27:26
 DRIVER: RMB
 OPERATOR: JAL

REF POST	SN	WHEEL	SN PEAK	AIR TEMP	TEST TIME	CYCLE NUMBER	EVENT
20.410	50.7	Left	86.7	48.2	15:26:36	510036065	
20.357	43.4	Right	91.1	48.2	15:26:41	510036066	Bridge
20.316	41.0	Left	77.4	48.2	15:26:44	510036067	

Left Wheel	N = 2	M = 45.9	SD = 6.88	H = 50.7	L = 41.0
Right Wheel	N = 1	M = 43.4	SD = 0.00	H = 43.4	L = 43.4
Total	N = 3	M = 45.1	SD = 5.07	H = 50.7	L = 41.0

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SITE: 035-10-36 Carter
 LANE: SBIL
 DATE: 11/09/2000
 TIME: 15:30:54
 DRIVER: RMB
 OPERATOR: JAL

REF POST	SN	WHEEL	SN PEAK	AIR TEMP	TEST TIME	CYCLE NUMBER	EVENT
20.393	51.5	Left	87.1	48.2	15:30:39	510036071	
20.352	52.2	Right	81.0	48.2	15:30:43	510036072	Bridge
20.315	50.5	Left	81.6	48.2	15:30:46	510036073	

Left Wheel	N = 2	M = 51.0	SD = 0.76	H = 51.5	L = 50.5
Right Wheel	N = 1	M = 52.2	SD = 0.00	H = 52.2	L = 52.2
Total	N = 3	M = 51.4	SD = 0.89	H = 52.2	L = 50.5

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