

Evaluation of Bonded Overlay and Dowel Bar Retrofit on I-40

Construction Report May, 1999

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16. ABSTRACT					
The Oklahoma Department of Transport	tation (OD	OT) has completed a rel	abilita	ation project on a 1,000) foot-long section of
existing jointed PCC pavement. The se	ection is lo	cated in the westbound to	raffic l	anes of I-40, in Canad	ian County.
As jointed PCC pavements age, many dride quality restored, by grinding, but trequire addressing structural factors, such common problem associated with aging ply grinding or by placing an overlay.	his often p h as void f	proves to be only a short formation in the subgrade	term s	solution. Longer lastin ack of load transfer bet	g solutions generally ween slabs. Another
Traditional methods of improving load it, drilling holes into the adjacent slab and new concrete poured to replace the remo	fastening	dowel bars into the holes	with o	epoxy. The joint then l	nas to re-formed, and
Work done on the rehabilitated section of the slabs, and placing a thin, bonded, fil			Units	(dowel bars with expa	ansion caps) between
The rehabilitation done on this project I improving surface friction characteristic past. The construction operations will be with the goal of projecting the long-term	s using a fevaluated	aster, less labor-intensive from a constructability state	e proc ndpoir	ess. ODOT has not use t. The completed over	ed this method in the
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SI (METRIC) CONVERSION FACTORS

Approximate Conversions to SI Units					Approximate Conversions from SI Units				
Symbol	When you know	Multiply by	To Find	Symbol	Symbol	When you know	Multiply by	To Find	Symbol
	parameter and the second secon	LENGTH					LENGTH	NONENGUALU SENIONO PERMUNINAS PERMUNINAS ESPANIOS (PERMUNINAS ESPANIOS PERMUNINAS ESPA PERMUNINAS ESPANIOS PERMUNINAS ESPANIOS PERMUNINAS ESPANIOS PERMUNINAS ESPANIOS PERMUNINAS ESPANIOS PERMUNINAS ESPANIOS PERMUNINAS PERMUNINAS ESPANIOS PERMUNINAS PERMUNINAS PERMUNINAS PERMUNINAS PERMUNINAS PERMUNINAS PERMUNINAS PERMUNINAS PERMUNINAS PERMUNINA	
in	inches	25.40	millimeters	mm	mm	millimeters	0.0394	inches	in
ft	feet	0.3048	meters	m	m	meters	3.281	feet	ft
yd	yards	0.9144	meters	m	m	meters	1.094	yards	yds
mi	miles	1.609	kilometers	km	km	kilometers	0.6214	miles	mi
		AREA		1			AREA		
in ²	square inches	645.2	square millimeters	mm²	mm²	square millimeters	0.00155	square inches	in²
ft²	square feet	0.0929	square meters	m ²	m²	square meters	10.764	square feet	ft²
yd²	square yards	0.8361	square meters	m²	m²	square meters	1.196	square yards	yd²
ac	acres	0.4047	hectacres	ha	ha	hectacres	2.471	acres	ac
mi²	square miles	2.590	square kilometers	km²	km²	square kilometers	0.3861	square miles	mi²
		VOLUME					VOLUME		
fl oz	fluid ounces	29.57	milliliters	mL	mL	milliliters	0.0338	fluid ounces	fl oz
gal	gallon	3.785	liters	L	L	liters	0.2642	gallon	gal
ft³	cubic feet	0.0283	cubic meters	m³	m³	cubic meters	35.315	cubic feet	ft³
yd³	cubic yards	0.7645	cubic meters	m³	m³	cubic meters	1.308	cubic yards	yd³
		MASS					MASS		
oz	ounces	28.35	grams	g	g	grams	0.0353	ounces	oz
lb	pounds	0.4536	kilograms	kg	kg	kilograms	2.205	pounds	lb
T	short tons (2000 lb)	0.907	megagrams	Mg	Mg	megagrams	1.1023	short tons (2000 lb)	T
	TEMP	ERATURE	(exact)			TEM	PERATURE	(exact)	
°F	degrees Fahrenheit	(°F-32)/1.8	degrees Celsius	°C	°C	degrees Fahrenheit	9/5(°C)+32	degrees Celsius	°F
	FORCE and	PRESSUR	E or STRESS			FORCE and	PRESSUR	E or STRESS	
lbf	poundforce	4.448	Newtons	N	N	Newtons	0.2248	poundforce	lbf
lbf/in²	poundforce per square inch	6.895	kilopascals	kPa	kPa	kilopascals	0.1450	poundforce per square inch	lbf/in²

EVALUATION OF BONDED OVERLAY AND DOWEL BAR RETROFIT ON I-40

Construction Report

May 1999

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INTRODUCTION

The Oklahoma Department of Transportation (ODOT) has completed a rehabilitation project on a 1,000 foot-long section of existing jointed PCC pavement. The test section was added to work done under Project NHI-40-4(344)137IP, Canadian County (Figure 1). The experimental section is located in the two westbound traffic lanes of I-40, from approximately 500 feet west to 500 feet east of Czech Hall Road (Station 1848 + 10 to 1858 + 10).

As jointed PCC pavements age, they often develop faults. Faults can be removed by grinding, but this often proves to be a short-term solution unless structural factors (formation of voids in the subgrade and lack of load transfer between slabs) are also addressed. Also, surface friction generally decreases as pavements age. Surface friction may be restored by grinding or placing an overlay.

Traditional methods of improving load transfer between slabs generally require saw cutting a section of each slab, removing it, drilling holes into the adjacent slab and fastening Load Transfer Units (dowel bars and expansion caps) into the holes with epoxy. The joint then has to be re-formed and new concrete poured to replace the removed section. This is an expensive, labor-intensive operation.

The rehabilitation done on this project has the potential of improving load transfer between slabs with a faster, less labor-intensive operation. Other benefits are improved ride quality, and improved surface friction characteristics. ODOT has not used this rehabilitation method in the past. The method will be evaluated from a constructability standpoint. The completed overlay will be evaluated with the goal of projecting the long-term performance of the rehabilitated section.

INTERSTATE 40 & CZECH HALL ROAD INTERCHANGE CONTROL SECTION NO. 40–09–05 STATE JOB NO. 11309(04) R-5-W REG. BE. STA, RO+61.73 CZECH MALL ED. A. BE LENGTH - 380° CICH MALL ED. TO GRANDAN CITY COW, LIMIT 9 BRIDGE 7. STA. 1877 + 00 END FEDERAL AID PROJ. NO. NHI-40–4(344) 137IP

Figure 1 Location of Project NHI-40-4(344)137IP

EXISTING PAVEMENT

The rehabilitation was done on a section of I-40 located roughly four miles west of the Oklahoma - Canadian County Line. ADT for this area is 74,500 with 11.28 percent trucks.

The existing roadway was built in 1969, under ODOT Construction Project I-40-4(52)134. Construction consisted of nine inches of PCC on a four inch layer of bituminous base, fine aggregate type. The PCC pavement had joints on 15 foot centers. After 30 years of service, ride quality had deteriorated in some areas due to fault formation. Some slabs had developed cracks which had required repair. Due to short notice prior to construction of the experimental feature, it was not possible to get condition data. ODOT Construction personnel agreed that moderate faulting was present before the overlay was placed, but no cracked slabs were found in the 1,000 foot long section before construction.

REHABILITATION OPERATIONS

Work on the section to be rehabilitated consisted of retrofitting load transfer units (LTU) between the slabs and placing a thin (3 inch), bonded, fiber reinforced overlay. The work is described in additional detail in the following section.

The Contractor, (Duit Construction, Edmond Oklahoma) milled one hundred foot long transitional tie-ins on either end of the one thousand foot-long section to accommodate the overlay. Depth of this cold milling varied from zero to 2 ½ inches. A CMI PR500FL Rotomilling Machine was used to do the milling.

MILLING SLOTS FOR LOAD TRANSFER UNITS

After the tie-ins were milled, the Contractor removed the milling arbor and reconfigured the cutting heads and spacers on the Rotomill (Figure 2). The reconfigured head was used to mill slots into the existing pavement (Figure 3). The milled slots were located spaced at two, three, four, nine, ten, and eleven feet from the centerline of the two westbound lanes (Figure 4). Individual slots were approximately four inches deep and three feet long.

Rotomilling slots into the existing pavement concerned both ODOT and Contractor's personnel. A significant amount of cracking and breakage occurred around the milled slots. Although all broken material was removed, the Construction personnel were concerned that the already fatigued existing PCC pavement may have been further damaged (hairline cracks, too small to be observed, etc.) by milling the slots. The cause of the breakage observed was thought, by the Construction personnel, to stem from either insufficient length of cutter teeth, cutter head oscillation or a combination of both.

Following slot milling, the Contractor sand blasted the surface, removed pavement markers, removed any remaining loose material from the surface with hand tools, cleaned the surface twice by shot blasting, and water blasted the pavement surface.

INSTALLING LOAD TRANSFER UNITS (LTU's)

Load Transfer Units (LTU's) consisted of 1 1/4 inch diameter, epoxy-coated dowel bars with expansion caps. The Contractor used three foot sections of LTU Support Baskets to secure the LTU's in place (Figure 5). Basket sections were cut in half along their length. This was done to ensure that the assembly (baskets and LTU's) were fully isolated on each side of the transverse joint, allowing for free movement during expansion and contraction. The dowel bars were next placed into the baskets and expansion caps were installed on the non-supported side of the LTU. The completed assemblies (Baskets and LTU's) were then placed in the milled slots and the baskets were secured to the pavement (Figure 6).

The Contractor's employees then set grade rails and line for the electronic paver controls. These were carefully checked to insure that overlay smoothness would compare favorably with ODOT requirements. Following this, reference kerfs were sawed (Figure 10) into the shoulders to ensure that joints sawed into the overlay would match exactly with those in the existing pavement.

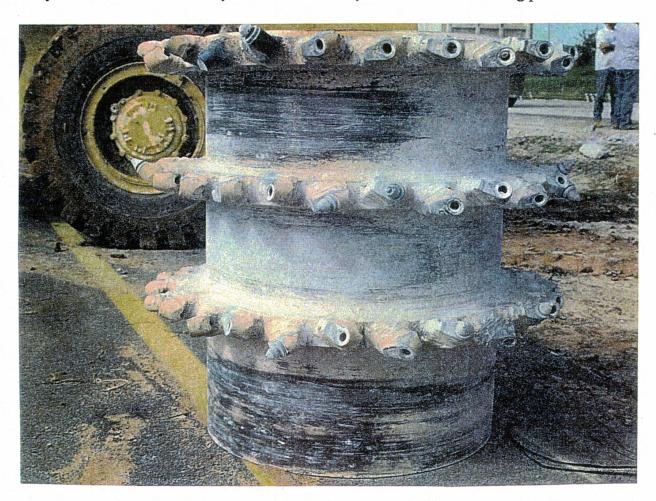


Figure 2 Rotomill Head Configured to Mill Slots for Load Transfer Units



Figure 3 Milled Slots



Figure 4 Roadway after Milling Slots

The Contractor used three foot sections of Load Transfer Unit Support Baskets to secure the LTU's in place (Figure 6). Basket sections were cut in half along their length. This was done to to ensure that the assembly (baskets and LTU's) were fully isolated on each side of the the transverse joint, allowing for free movement during expansion and contraction. The 1 1/4 inch diameter, epoxy-coated LTU's were then placed into the baskets (Figure 7) and expansion caps were installed on the non-supported side of the LTU (Figure 8). The completed assembly (Basket and LTU's) was then placed in the milled slots and the basket was secured to the pavement (Figure 9).

The Contractor's employees next set grade rails and line for the electronic paver controls. These were carefully checked to insure that smoothness of the overlay would compare favorably with ODOT smoothness requirements. Following this, reference kerfs were sawed (Figure 10) into the adjacent pavement to ensure that joints sawed into the overlay would match exactly with those below them in the existing pavement.

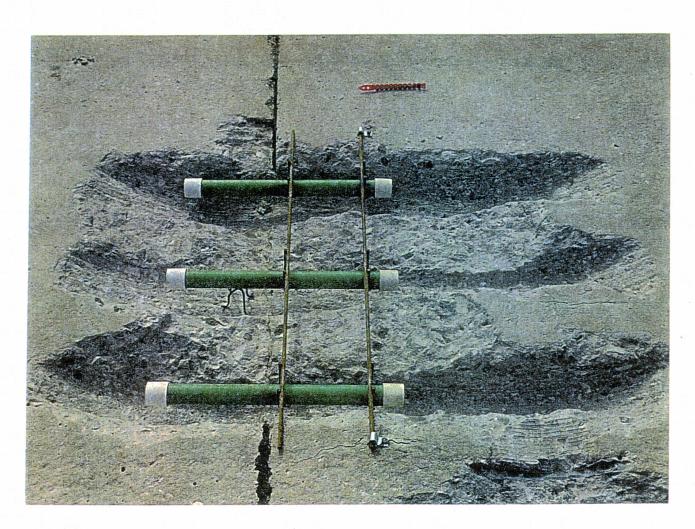


Figure 5 Complete Basket Assembly with LTU's and Expansion Caps Installed in Slot



Figure 6 Securing Basket to Existing Pavement

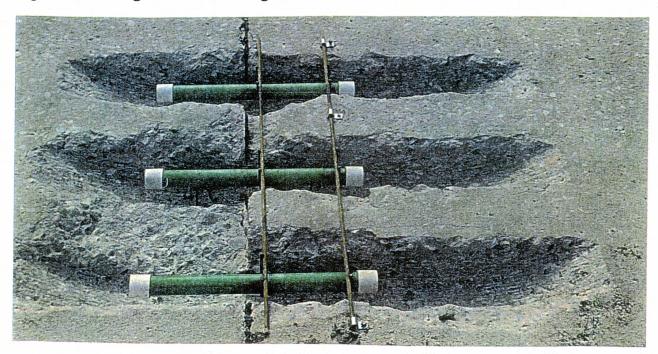


Figure 7 Basket Secured to Existing Pavement



Figure 8 Reference Kerf Marking Joint Location

PLACING THE OVERLAY

A CMI SF450 Twenty-Four paving machine and a mechanical tining machine were set up to be used for the overlay operation. Concrete production began at 11:00 AM, October 23, 1998. The concrete was produced at a portable batch plant set up at I-40 and SH 92, approximately one mile from the jobsite. Concrete mix and placement information is listed in Appendix "B". The concrete was delivered to the site in end dump trucks and dumped immediately in front of the paver (Figure 8). Laborers placed concrete around the LTU's in the milled slots using shovels (Figure 9) just ahead of the paver. This was done to avoid the possibility of LTU's becoming misaligned as they could have been if filled by the larger amount of concrete moved by the paver. The paver placed the overlay and filled any remaining space in the slots monolithically. Tining followed placement, strike off, floating and finishing by approximately 45 minutes. Curing compound was applied (Figure 10), at twice the normal rate, roughly 30 minutes after tining. Joint sawing was done approximately 18 hours after placement had been completed. The reference kerfs sawn earlier were used to locate all joints before sawing the overlay. Joints were sawn to a depth equal to the overlay thickness, plus one-half inch. Normal joint sealing, with self-leveling silicone sealant, was the final step in this operation.



Figure 9 Concrete Delivered to Paver

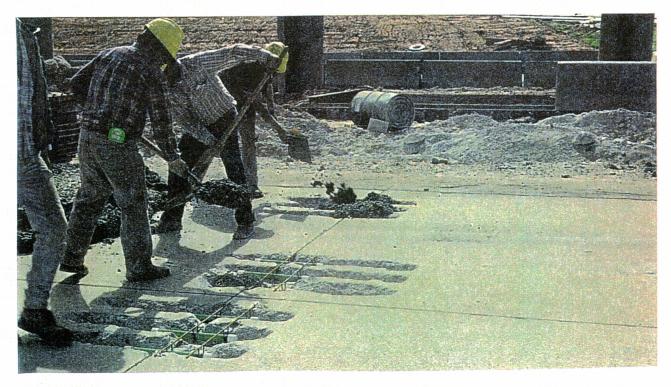


Figure 10 Filling Slots in Front of Paver

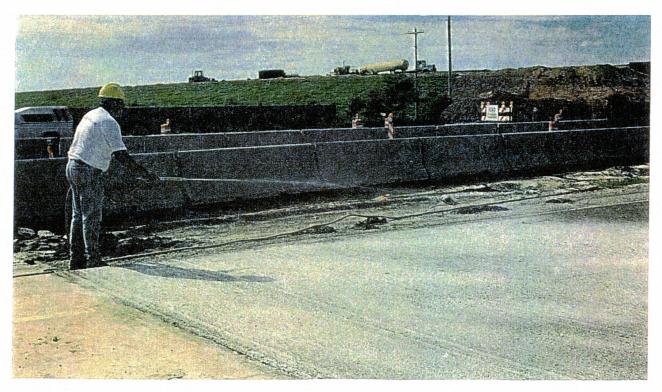


Figure 11 Applying Curing Compound

TEST DATA

Falling Weight Deflectometer (FWD)

Falling Weight Deflectometer (FWD) testing was first done on the rehabilitated section on October 22, 1998, one day before the overlay was placed. At that time, tie-ins on each end of the experimental section had been milled. Also, slots had been milled for LTU's.

The second "round" of FWD tests was done March 3, 1999, roughly four months after completion of work on the overlay.

FWD test data are listed in Appendix "C". Data includes figures for deflections at the FWD plate (designated D_0) and deflections 12 inches from the joint (on the opposite side of the joint from the plate) designated D_{-12} . Also listed are Load Transfer Efficiency (LTE) figures calculated from deflections before (October 22, 1998) and after (March 3, 1999) as shown below.

 $LTE(\%) = D_{-12} / D_0 X 100$

Briefly, LTE's calculated from the October 22, 1998 tests did not differ greatly from those calculated from tests done March 3, 1999. However, both deflections (D_0 and D_{-12}) measured during the March 3, 1999 tests were considerably lower than those measured October 22, 1998. Generally, deflections measured during the second round of FWD tests were more than 40 percent smaller than those measured October 22, 1998.

Smoothness

Smoothness of the overlay was measured using the California Profilograph. Short notice made it impossible to get smoothness data for the existing roadway before the overlay was placed.

The overlay was tested on October 26, 1998, three days after placement. Test results on the overlay showed good smoothness, the outside (north) lane had a Profile Index (PI) of 0.977 Inches per mile of roughness while roughness measured on the inside (south) lane was 0.0 inches per mile. Both PI's would qualify for the maximum bonus under ODOT smoothness requirements.

Profilograph test data are included in Appendix "C".

APPENDIX A ODOT SPECIAL PROVISION FOR FIBER REINFORCED, BONDED PORTLAND CEMENT CONCRETE OVERLAY

435-1(a-f) 91S 6-16-94

OKLAHOMA DEPARTMENT OF TRANSPORTATION SPECIAL PROVISION

FOR

FIBER REINFORCED, BONDED PORTLAND CEMENT CONCRETE OVERLAY PROJECT NO. SAP-11(335), J/P 09244(04) CHEROKEE COUNTY

Special Provisions revise, amend and where These supersede applicable sections of the Standard Specifications for Highway Construction, Edition of 1988, and the

Supplement, thereto, Edition of 1991.

DESCRIPTION. This work consists of placing a fiber 435.01. reinforced, bonded PCC resurfacing overlay of an existing PCC pavement to the thickness as shown on the plans, and may include associated patching and widening work.

shall All materials MATERIALS.

requirements for the items in Section 700, Materials.

Cement. Cement shall conform to Subsection 701.02. use of Type III and Type IIIA cement will not be permitted.

Fly ash may not be used as a cement replacement.

Aggregate shall conform to Subsection Aggregate. (b) 701.05 and 701.06. The gradation of the coarse aggregate in the PCC overlay shall have 100% passing the 1 inch sieve. The type coarse aggregate in the overlay shall be of the same type the original pavement to reduce stresses due to the coefficients of thermal expansion of different materials.

(c) Concrete. Concrete shall conform to Section 701.01 for The bond shear strength between the 'AA' concrete. Class original pavement and overlay shall be a minimum of 200 PSI, tested with an Iowa Shear Strength test fixture or other similar device. A Type 'A' certification, as shown in Subsection 106.12, shall be submitted for each lane-mile (or major portion there of) of the overlay.

No grout or dampening of the slab (d) Grout.

permitted prior to overlaying.

(e) Joint Filler and Sealer. Joint filler and sealer shall be a preformed elastomeric compression joint seal and conform Subsection 701.08(c) or a low modulus silicone joint sealant and shall conform to Subsection 701.08(e)

Admixtures. Admixtures shall conform to Subsection

701.03.

Fiber Reinforcement. The fibers shall be engineered, synthetic fibers that are 100 percent polypropylene, collated, fibrillated fibers which have been specifically manufactured optimum gradation for use as a secondary reinforcement Portland Cement concrete. The fibers shall not contain any reprocessed olefin materials. The following requirements also be met for the fibers:

435-1(b) 91S 6-16-94

Specific Gravity----- 0.90 to 0.95
Modulus of Elasticity-- 500 KSI, minimum
Tensile Strength----- 70 KSI, minimum
Length-------- 1/4 to 1 inch
Absorption Rate----- nil

The fibers shall be added to the concrete overlay PCC at the rate of 3 pounds per cubic yard, or at the manufacturer's suggested rate, which ever is greater. The concrete will be mixed sufficiently to distribute the fiber uniformly. The manufacturer or approved distributor shall provide the services of a qualified technical representative to prepare a trial batch of modified PCC and assist in the initial placement/finishing.

435.03 EQUIPMENT. The equipment used shall be subject to approval of the Engineer and shall comply with the following:

- (a) Shot Blasting Equipment. Equipment, such as the "SKIDABRADER" or its equivalent, shall be power-operated and capable of propelling steel shot against the pavement surface such that the entire concrete surface is uniformly textured at minimum width of six feet per pass. The shot blasting equipment shall include a means to collect used shot, which may be used again, and waste material. Materials removed in the cleaning operation shall be disposed of in a manner approved by the Engineer.
- (b) Proportioning and Mixing Equipment. Proportioning and mixing equipment shall conform to Subsection 414.03(a). Sufficient mixing capacity of mixers shall be provided to permit the intended pour to be placed without interruption.
- (c) Placing and Finishing Equipment. Placing and finishing equipment shall conform to Subsection 414.04. The contractor shall construct the pavement in a manner and with a system that will provide a smooth riding surface. The concrete placing equipment shall be either controlled to the proper elevation by string line or operated on a pad line that is constructed to a true grade line.
- true grade line.

 (d) Sand Patch Test Equipment and ACI Surface Soundness
 Test Equipment. Two sets of each test equipment will be
 purchased by the contractor and turned over to the ODOT. Resident
 Engineer at the completion of the project.
- 435.04 CONSTRUCTION METHODS.

 (a) Full Depth PCC Patches. Full depth PCC patches shall be PCC patches constructed according to the plans with Class 'A' Concrete. Dowels will be required as shown on the plans. The patches will be full lane width, and the minimum length of patch, measured parallel to the centerline, will be 6 feet. Full depth patches shall be completed in an area before resurfacing work is done. Construction of full depth patches shall be according to Construction Standard PCPR-1-1 or

435-1(c) 91S 6-16-94

the most current Standard. However, joint sealing will not be required, calcium chloride shall not be used, traffic shall not be permitted on the patch for 36 hours, and smoothness will be subject to evaluation to 1/4 inch tolerance in 10 feet.

- (b) Pavement Widening. Pavement widening will be required at locations and as detailed on the plans. The pavement widening shall be tied to the existing pavement as shown on the plans. Holes for tiebars shall be drilled 1/8 inch larger in diameter The holes shall be clean and dry, than the tiebar. with particles of dust remaining in the holes. The tiebars shall grouted in place with an epoxy material, which conforms Subsection 701.13 and recommended for horizontal installation. A positive injection and retaining procedure for the epoxy will be required, as approved by the Engineer. Tiebars shall have a minimum cure time of 24 hours before being subjected to any Tiebars shall be located at least 12 inches from existing transverse joints. The pavement widening may not be placed monolithically with the overlay in a single pass.
- (c) Preparation of Surface. Preparation shall include the entire surface to be overlayed. The surface shall be thoroughly cleaned with a shotblasting machine or equivalent equipment. The preparation of the surface shall be of such an extent as to remove all dirt, oil, and other foreign materials, as well as any laitance or loose material from the surface and edges against which new concrete is to be placed. All old joint sealing the material and incompressible materials shall likewise be removed. Materials removed in the preparation operation shall be disposed of in a manner approved by the Engineer.
- (d) Placing and Finishing Resurfacing Concrete. Resurfacing concrete shall be placed in accordance with applicable requirements of Section 414 with the following modifications:
- Surface Cleaning. Prior to applying the overlay to the surface, the entire surface shall be cleaned with a oil-free, compressed airblast. Acceptable cleaning will be determined by increasing the average surface macrotexture at least 0.030 from the unimproved value to an average minimum value of 0.060, as measured with ASTM E 965-87 standard volumetric method. This shall be followed by successful surface soundness tests in accordance with ACI 503R surface adhesion test method. The ACI test results will be based on the average of 3 or more tests, exceeding 250 PSI at a minimum of 4 job locations. that are less than 250 PSI, but fail at a depth of 1/4 inch or more, and over50% or more of the test area, may be considered passing test. If passing tests are not achieved, the surface macrotexture shall be increased until passing test results are After cleaning, no traffic shall be permitted on cleaned surface except the paving machine. The Contractor shall take precautions to prevent contamination of the cleaned pavement surface prior to overlaying.

435-1(d) 91s 6 - 16 - 94

- Joint Identification. The exact location contraction and expansion joint in the existing pavement joints to be sawed at each full depth patch shall be identified on both sides by a reliable method.
- Placing and Finishing Resurfacing Concrete. Contractor shall take every reasonable precaution to secure a smooth riding surface. Prior to the placement operation, shall review his equipment, procedures, personnel, previous results and the inspection procedure with the Engineer to assure a coordinated effort. Precautions shall include the following:

-Assurance that the concrete can be produced and placed to the proper thickness and cross section within the specified limits, continuously and with uniformity,

-The thickness of all new concrete above the prepared surface

shall be as specified on the plans,

-The use of a broom, burlap or astroturf drag or mechanical tining shall be as directed by the fiber manufacturer's field

-At those times when the evaporation rate (see attached nomograph) exceeds 0.20 LBS/SF/HR for a period of time as specified by the Engineer, or greater than 20 minutes, or the difference in the ambient temperature at the time of placement verses the expected low temperature in a 24 hour period exceeds degrees Frameasures shall be taken to control the moisture content of the newly placed concrete overlay. Fogging, wet or other measures as directed by the Engineer, shall be curing, used to control the moisture content. The measures are addition to the membrane curing required. When such conditions occur, the gentire day's placement shall be protected The protection shall remain in place for a minimum of 36 hours, until such a time as directed by the Engineer. The removal the protection shall be as directed by the Engineer, the protection shall be as directed by the Engineer,

-At those times when the difference in the ambient temperature the time of placement verses the expected low temperature is expected to exceed 25 degrees F, the overlay shall be placed later than 12 o'clock noon the preceding day or a minimum of hours prior to the time the maximum temperature difference is Sec. 17.

expected,

-A white pigmented curing compound meeting the requirements Subsection 701.07(d) shall be used and it shall be applied at the rate of one gallon to not more than 100 square feet.

Joints. Joints shall be sawed in the overlay directly over existing stransverse and longitudinal joints. The joints shall be sawed to the full depth of the new overlay concrete plus one-half inch. Joints in the widened section shall be sawed 1/3 of thickness of the pavement in these areas. The joints shall be

435-1(e) 91S 6-16-94

sawed as soon as possible without causing excessive raveling. All joints shall be cleaned and sealed in accordance with Subsection 701.08 or according to the manufacturer's

一种的 人名法沙勒 薩登 美国

- recommendation.

 (e) Limitations of Operations. Limitations of operations shall conform to Subsection 414.04(f), 414.04(p) and 414.04(q) No resurfacing concrete shall be placed when the air or pavement temperature is at or expected to be below 40 degrees F during At the time of the overlay construction, the sum of the free air temperature and temperature of the plastic concrete delivered to the paver shall not exceed 180 degrees F. At no shall the plastic PCC be delivered at a temperature exceeding 80 degrees F. If ice is substituted for the mix water in the concrete to lower the plastic concrete temperature, all ice shall be melted and the water thoroughly mixed in the the plastic PCC prior to placement.
- 435.05. METHOD OF MEASUREMENT. The quantity of the various items of work involved in the construction of Portland Cement concrete resurfacing will be measured in accordance with the following provisions: which is the same of the sam
- Portland Cement Concrete Resurfacing will be measured in cubic yards of Class 'AA' Concrete using a count of batches incorporated. This quantity will include concrete placed in the overlay only. The control of the second of t
- (b) Overlay Placement of Portland Cement Concrete Pavement, will be measured in square yards. This area will be measured in accordance to Section 414.05.

 (c) Full Depth PCC Patches will be measured in square yards
- of the nominal surface area of the patches. The patches the patches of the patches the patches of the patches o
- 435.06. BASIS OF PAYMENT. Accepted quantities, measured as provided above, will be paid for in accordance with following provisions: White the water water was the same water the
- (a) Portland Cement Concrete Resurfacing and Overlay Placement will be paid for at the contract unit price for:
 - (A) CLASS AAA CONCRETE CONTRACTOR OF THE CONTRAC

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THE COURT OF THE PROPERTY OF T which shall be full compensation for furnishing all material, equipment, labor and incidentals necessary for the proportioning, mixing, delivery and placement of the concrete. This includes the placement of bars for widening and sawing, cleaning and sealing the joints, in accordance with the plans and this Special Provision.

- (b) Full Depth Patches will be paid for at the contract unit price for: 1 1 1
 - (C) PORTLAND GEMENT CONCRETE (PATCHING) S.Y.

435-1(f) 91S 6-16-94

which shall be full compensation for furnishing all materials, equipment, labor and incidentals necessary for proportioning, mixing, delivery and placement of the concrete. This includes the cost of bars and bar assemblies used in accordance with the plans.

APPENDIX B CONCRETE MIX AND PLACEMENT INFORMATION

CONCRETE DESIGN

Design: Class "AA" Fiber Reinforced Concrete (ODOT Special Provision 435-1(a-f) 91s, Appendix A).

Proportions: Holnam Type I Portland Cement (Ada)	SG = 3.15	658 lb.
Dolese Fine Aggregate (Dover)	SG = 2.64	1,181 lb.
Dolese # 67 Coarse Aggregate (Davis)	SG = 2.667	1.790 lb.
Batch Water		253 lb.
Masterbuilders Air Entraining Agent		59 fl. oz.
Fibermesh fiber reinforcement (polypropylene)		3 lb.

Other Requirements;

11 1	143.41 lb./ cf
Yeild	143.41 ID./ CI
Twenty eight day Compressive Strength	3,500 psi
Slump	1 - 3 in
Water Cement Ratio	0.385
Air Entrainment	5 - 7 %
Aggregate Proportions	60 % Coarse: 40 % Fine

BATCH PROPORTIONS

Holnam Type I Portland Cement (Ada)	5,250 lb.
Dolese Fine Aggregate (Dover)	9,620 lb.
M.P.I. # 67 Coarse Aggregate (Davis)	14,380 lb.
Batch Water	1,524 lb.
Masterbuilders Air Entraining Agent	175 fl. oz.
Fibermesh Fiber Reinforcement	24 lb.

PLACEMENT CONDITIONS

(October 23, 1998, 11:30 AM)

Slump Ambient Air Temperature Fresh Concrete Temperature Water Cement Ratio Air Entrainment

69 - 70 degrees F 5.9 - 6.0 %

2 - 2.5 in

0.29

65 - 66 degrees F

Other Information

Thirty-three 8 cubic yard loads of Class "AA" Fiber Reinforced Concrete were batched, totaling 264 cubic yards.

APPENDIX C FALLING WEIGHT DEFLECTOMETER AND SMOOTHNESS DATA

PROJECT NO. NHI-40-4(344)137IP (Experimental Overlay Section)

County Canadian

CONTRACTOR Duit Construction

Pavement Type <u>Thin bonded PCC Overlay (Jointed,</u> with retrofitted dowel bars)

Station 1848 + 10 to station 1858 + 10 Traffic Direction Westbound

No. Of Lanes 2 (The two Westbound lanes of a divided four-lane highway)

Date Placed <u>10-23-98</u> Date Tested <u>10-26-98</u> Tested by <u>Gary Williams, ODOT Research</u>
<u>Outside (North) Lane, Westbound Expressway</u>

	<u>Trac</u> Meas			Track 1 (excluding 1st and last 25') Measured				
Segment	Length (Mi)	Roughness (In)	PRI (In/Mi)	Segment	Length (Mi)	Roughness (In)	PRI (In/Mi)	
1	0.100	0.44	4.40	1	0.100	0.11	1.10	
2	0.074	0.06	0.81	2	0.074	0.06	0.81	
Total	0.174	0.50	2.87	Total	0.174	0.17	0.977	. `

Inside (South) Lane, Westbound Expressway

	<u>Track 2</u> Measured				Track 2 (excluding 1st and last 25') Measured				
Segment	Length (Mi)	Roughness (In)	PRI (In/Mi)	Segment	Length (Mi)	Roughness (In)	PRI (In/Mi)		
1	0.100	0.20	2.00	1	0.100	0.00	0.00		
2	0.077	0.38	4.94	2	0.077	0.00	0.00		
Total	0.177	0.58	6.94	Total	0.177	0.00	0.00		

 D_0 = Deflection at plate, D_{12} = Deflection 12 inches from joint, LTE = Load Transfer Efficiency

October 22,1998						March	Deflection Change October to March			
	Distance	Drop	Drop	LTE(%)	Distance	Drop	Drop	LTE(%)	Drop	Drop
		D_0	D- ₁₂	$D_{-12}/D_0 \times 100\%$		D_0	D- ₁₂	$D_{-12}/D_0 \times 100\%$	D_0	D- ₁₂
=										
	150	8.7	8.22	94.5	150	4.56	4.25	93.2	4.14	3.97
•	166	8.35	7.33	87.8	166	4.7	4.31	91.7	3.65	3.02
*	181	7.78	7.24	93.1	181	4.15	3.85	92.8	3.63	3.39
	197	6.81	6.12	89.9	197	4.21	3.91	92.9	2.6	2.21
	469	9.52	9.14	96.0	469	5.33	5.07	95.1	4.19	4.07
	484	9.31	8.64	92.8	484	5.38	5.13	95.4	3.93	3.51
	499	9.62	9.19	95.5	499	5.38	5.14	95.5	4.24	4.05
	515	8.7	8.06	92.6	515	5.19	4.87	93.8	3.51	3.19
	698	9.13	8.54	93.5	698	5.07	4.81	94.9	4.06	3.73
	713	9.11	8.24	90.5	713	5.2	4.79	92.1	3.91	3.45
	728	8.94	8.41	94.1	728	5.05	4.63	91.7	3.89	3.78
	744	7.67	7.1	92.6	744	4.88	4.57	93.6	2.79	2.53
	759	7.95	7.5	94.3	759	4.68	4.54	97.0	3.27	2.96

I 40 / Czech Hall Rd. FWD Joint Load Transfer Tests, Right

D_0 = Deflection at plate, D_{12} = Deflection 12 inches from joint, LTE = Load Transfer Efficiency

1	October	22,199	8		March	Deflection Change October to March			
tance	Drop D₀	Drop D- ₁₂	LTE(%) D ₋₁₂ /D ₀ x100%	Distance	Drop D₀	Drop D- ₁₂	LTE(%) D ₋₁₂ /D ₀ ×100%	Drop D ₀	Drop D- ₁₂
759	10.47	9.78	93.4	759	5.53	5.28	95.5	4.94	4.5
743	8.02	7.5	93.5	743	5.46	5.07	92.9	2.56	2.43
728	8.26	7.43	90.0	728	4.26	3.99	93.7	4	3.44
713	10.24	8.98	87.7	713	4.64	4.35	93.8	5.6	4.63
697	10.33	9.57	92.6	697	5.11	4.78	93.5	5.22	4.79
516	8.61	8.13	94.4	516	5.52	5.02	90.9	3.09	3.11
500	9.68	9.33	96.4	500	5.16	4.84	93.8	4.52	4.49
486	7.47	6.76	90.5	486	4.5	3.88	86.2	2.97	2.88
470	9.89	9.22	93.2	470	4.61	4.24	92.0	5.28	4.98
198	7.46	6.92	92.8	198	4.17	3.81	91.4	3.29	3.11
182	7.74	7.22	93.3	182	4.14	3.98	96.1	3.6	3.24
167	8.22	7.78	94.6	167	4.25	3.97	93.4	3.97	3.81
152	7.63	7	91.7	152	3.88	3.54	91.2	3.75	3.46

I 40 / Czech Hall Rd. FWD Load Tests, Middle of Slab

D_0 = Deflection at plate

,Distance	Drop	Drop	Deflection Change
	D₀	D₀	October to March
. 160	7.18	4.11	3.07
193	7.09	4.52	2.57
482	8.58	4.46	4.12
511	9.08	5.52	3.56
724	8.7	5.13	3.57
753	7.11		2.21

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     794 1.007 1.078
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10/22/98

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- 989	241 221	211 179	148 231 69	15707 9.49	8.71	8.31	7.05	5.83	9.09	2.72
993	243 222	213 181	148 232 70	15787 9.56	8.76	8.37	7.11	5.84	9.15	2.74
9.94	244 224	213 181	150 233 70	15800 9.62	8.80	8.39	7.14	5.89	9.19	2.76
994	244 224	214 182	150 234 70	15803 9.59	8.81	8.42	7.16	5.89	9.20	2.77
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			140 201 68	15490 8.52	7.90	7.60	6.57	5.53	7.90	2.68
				15800 8.70	8.07	7.76	6.70	5.64	8.06	2.76
				15803 8.70	8.07	7.75	6.70	5.63	8.06	2.74
				15771 8.70	8.06	7.77	6.72	5.65	8.06	2.76
S	698sb	-17.8	29 21151500							
991				15747 9.11	8.34	7.94	6.76	5.59	8.48	2.44
				15771 9.12	8.35	7.97	6.77	5.63	8.55	2.43
				15823 9.13	8.36	7.96	6.76	5.60	8.54	2.42
				15787 9.15	8.38	7.97	6.80	5.66	8.57	2.42
	713sb	-17.8	28 21151502		0.50	1.51	0.00	3.00	0.57	2.12
S				15533 8.95	8.21	7.87	6.75	5.59	8.02	2.52
	230 211			15636 9.04	8.31	7.96	6.83	5.65	8.13	2.54
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				15816 9.16	8.44	8.08	6.90	5.69	8.29	2.57
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969			135 208 55		8.08	7.68	6.62		8.37	2.16
				15684 8.89	8.24	7.85	6.67	5.38		
				15747 8.94	8.29	7.89	6.74	5.45	8.41	2.26
				15847 8.98	8.33	7.94	6.81	5.50	8.46	2.33
S	744sb	-17.8	28 20151506	-0 82 69	6 0 5	6 51	- A A	4 40	6 00	1 00
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986				15665 7.63	6.99	6.63	5.57	4.51	7.06	1.94
				15696 7.67		6.64	5.59	4.54	7.10	1.95
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				15871 7.95	7.13	6.75	5.53		7.50	1.93
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		127 114		16152 5.55	5.14	4.98	4.48	3.93	5.22	2.30
		124 111	98 130 58	15843 5.41	5.03	4.89	4.38	3.85	5.11	2.26
997		125 112		15843 5.38		4.92	4.40	3.85	5.13	2.28
2006		125 112		15903 5.40	5.07	4.91	4.41	3.89	5.15	2.30
S.	499sb	-17.8	27 13I51446							1
		124 110		16237 5.55	5.07	4.89	4.31	3.72	5.29	2.02
		121 106		15807 5.32	4.95	4.76	4.17	3.56	5.08	1.93
		121 107		15879 5.38		4.78	4.20		5.14	1.96
		121 106		15795 5.37	4.95	4.77	4.19	3.60	5.12	1.95
S		-17.8	28 13I51447							
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		118 104		15795 5.17		4.64	4.09	3.52	4.88	1.93
		118 104		15874 5.19		4.66	4.10		4.87	1.93
		118 104		15911 5.19	4.84	4.66	4.10	3.52	4.88	1.93
S	696sb	-17.8	28 14I51449	-0 83 56						
1026	133 125	120 107	92 126 49	16311 5.25	4.91	4.72	4.20	3.61	4.96	1.94
996	129 121	116 104	90 122 48	15823 5.08	4.76	4.57	4.08	3.53	4.81	1.89
996	129 121	116 103	89 122 48	15831 5.07	4.76	4.58	4.06	3.50	4.81	1.89
1001	130 122	117 104		15898 5.10	4.79	4.60	4.10	3.55	4.83	1.91
S	712sb	-17.8	28 13I51450	-0 82 55						
1001	133 124	120 107	92 122 49	15906 5.25	4.87	4.72	4.20	3.62	4.81	1.94
996	131 122	119 106	91 121 49	15823 5.16	4.81	4.67	4.16	3.59	4.77	1.94
1002	132 123	119 106		15927 5.20	4.84	4.69	4.19	3.61	4.79	1.95
996	133 123	119 106	91 121 49	15823 5.22	4.84	4.70	4.17	3.60	4.78	1.95
S	727sb	-17.8								
		116 101		16070 5.11	4.73	4.58	4.00	3.38		1.76
~ 4		114 100		15819 5.04	4.67	4.49	3.96	3.37		1.77
		115 100		15882 5.05	4.69	4.52	3.96	3.37	4.63	1.76
1005		115 101		15966 5.06	4.70	4.52	3.98	3.40	4.69	1.81
S		-17.8	28 12I51453							
		109 96		15827 4.89		4.30	3.76	3.18	4.56	1.65
	124 114			15914 4.87		4.30	3.78	3.20	4.57	1.65
996	124 114	109 96	82 116 43	15831 4.88	4.49	4.31	3.78	3.22	4.57	1.68
998				15855 4.87	4.48	4.30	3.78	3.20	4.57	1.66
S	758sb		28 11151455							
	120 110			15974 4.74	4.32	4.14	3.58	3.01		1.58
	119 109			15882 4.69	4.28	4.10	3.56	2.99		1.57
	119 109			15919 4.68	4.31	4.12	3.55		4.54	1.56
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S	516sb	-17.8	28 18I51530							
		187 156		15684 8.45	7.74	7.37	6.15	5.02	7.99	2.29
				15843 8.58	7.85	7.46	6.23	5.09	8.11	2.31
996				15819 8.61	7.86	7.48	6.24	5.10	8.13	2.34
		191 160		15819 8.68	7.91	7.52	6.30	5.14	8.19	2.35
S	500sb	-17.8	28 18151531	-0 82 64	1.51	1.52	0.50	J.11	0.10	2.55
070		204 169		15538 9.45	8.49	8.04	6.66	5.39	9.11	2.41
970	240 210	204 109		15819 9.62	8.67	8.20	6.78	5.48	9.28	2.43
996	244 220	200 172	140 237 62	15895 9.68	8.68	8.22	6.80		9.33	2.43
		209 173		15831 9.67	8.65	8.19	6.79	5.50	9.31	2.44
de la constant de la		-17.8	27 18I51533		0.65	0.19	0.79	5.50	9.31	2.45
S	486sb				C 70	C 17	E 47	4 52	c c2	2 06
		164 139		15688 7.38	6.78	6.47	5.47	4.53	6.63	2.06
				15847 7.48	6.87	6.55	5.54	4.59	6.74	2.07
				15795 7.47	6.83	6.49	5.61	4.54	6.76	2.09
		167 141		15847 7.51	6.89	6.56	5.56	4.59	6.78	2.06
S	470sb	-17.8	27 19151535	-0 81 67	0 75	0 04	6 05		0 00	
		209 174		15556 9.65	8.75	8.24	6.85	5.56	8.98	2.31
		213 176		15819 9.81	8.88	8.39	6.94	5.63	9.16	2.36
		215 177		15827 9.89		8.45	6.99		9.22	2.36
		215 178		15823 9.92	8.96	8.47	7.02	5.68	9.26	2.41
S	198sb	-17.8	27 20151540	-0 81 69	6 00	C 4 F	- 25	4 22	6 06	1 0 5
		164 136		15927 7.50	6.80	6.45	5.37	4.33	6.96	1.87
				15858 7.46	6.80	6.47	5.34	4.28	6.97	1.85
		163 136		15855 7.46	6.76	6.41	5.34	4.28	6.92	1.85
		163 135		15855 7.46	6.76	6.43	5.33	4.28	6.93	1.85
S		-17.8	28 20151541	-0 83 68	6 00	6 50		4 00	- 45	
		166 136		15744 7.65	6.89	6.53	5.36	4.28	7.17	1.75
	196 176	167 137	110 183 45	15823 7.72	6.93	6.56	5.39	4.33	7.21	1.76
		167 137		15866 7.74		6.57	5.41	4.34	the state of the s	1.78
		167 137		15823 7.74	6.97	6.59	5.40	4.30	7.27	1.76
S		-17.8	29 19I51543	-0 83 66						
		173 141		15580 8.06	7.20	6.81	5.55	4.37		1.78
		175 143		15800 8.19	7.31	6.90	5.61	4.42	7.74	1.81
	209 187			15863 8.22	7.35	6.94	5.64	4.44	7.78	1.80
		176 143		15850 8.19	7.30	6.92	5.63	4.45	7.76	1.80
S	152sb	-17.8	28 20151552	-0 82 67						
		162 133		15474 7.43	6.72	6.39	5.23	4.13		1.72
		164 136		15760 7.56	6.81	6.46	5.34	4.32	6.93	1.75
		166 137		15823 7.63	6.87	6.52	5.38	4.35	7.00	1.76
996	194 175	166 137	109 179 45	15827 7.66	6.90	6.55	5.38	4.29	7.04	1.76

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			115	100	85	122	49	15895	5.11	4.69	4.51	3.93	3.35	4.78	1.91
		1	115			121		15863	5.10	4.69	4.52	3.94	3.35	4.78	1.90
A		ر طد	-17			11151			76 53	4.00	1.52	3.54	3.33	4.70	1.50
			123			128		15895	5.54	5.05	4.83	4.17	3.51	5.03	1.80
			122			127		15839	5.50	5.00	4.80	4.15	3.48	5.00	1.81
196	140					127		15831	5.52	5.02	4.82	4.16	3.49	5.02	1.81
Acc.	140					127		15906	5.52	5.03	4.82	4.17	3.50	5.01	1.81
3	5008		-17.			11151			76 52	5.05	4.02	4.17	3.50	3.01	1.01
	132			98		124		15950	5.18	4.68	4.49	3.85	3.26	4.87	1.72
	130			97		122		15807	5.13	4.64	4.45	3.82	3.23	4.87	
	131			98		123		15890	5.16						1.70
	131			98		123		15927	5.15	4.66	4.47	3.84	3.26	4.84	1.73
S 1002	484		-17.			123 12I51			78 53	4.67	4.47	3.85	3.26	4.84	1.72
	113 1		98	84	70	98		15723	4.44	4.06	3.86	3.31	2.77	2 00	1 11
	114 1		99	85	71	98		15723	4.44	4.08				3.86	1.44
	114 1		99	85	71	99		15879			3.88	3.34	2.80	3.87	1.44
	114 1		99	85	71	9.8		15863	4.50	4.08	3.89	3.35	2.80	3.88	1.45
S	4698		-17.						4.47	4.07	3.88	3.33	2.78	3.87	1.44
i i						11I51 108		-0		4 25	4 05	2 40	0 00	4 0 4	
	119 1 118 1			89				15816	4.67	4.25	4.05	3.49	2.98	4.24	1.60
				88		108		15974	4.66	4.22	4.04	3.48	2.93	4.27	1.58
	117 1			88		108		15882	4.61	4.20	4.01	3.46		4.24	1.59
	118 1			88		108		15935	4.64	4.19	4.00	3.45	2.84	4.27	1.53
S	1988		-17.			L3I51		-0 7		2 76	2 60	2 4 4			
	107	95	91	79	67	98		15871	4.19	3.76	3.60	3.11	2.65	3.85	1.41
999		95	91	79	67	97		15871	4.17	3.75	3.59	3.11	2.65	3.83	1.43
999		95	91	79 79	67	97		15874	4.17	3.74	3.59	3.11	2.65	3.81	1.43
1001		95	91		67	97		15903	4.18	3.76	3.59	3.11	2.63	3.83	1.42
S	1825		-17.			13151		-0 7		2	2 56				
1007		95	90	77		101		16006	4.17	3.75	3.56	3.02	2.52	3.98	1.32
7		94	89	76	63	99		15839	4.11	3.69	3.50	2.98	2.48	3.91	1.31
12		95	89	77		101			4.14	3.74	3.51	3.02	2.52	3.98	1.31
996		94	89	76		101		15831	4.11	3.72	3.48	2.99	2.51	3.96	1.32
S	1685		-17.			3151		-0 7							
998		96	91	77		101		15858	4.24	3.77	3.58	3.04	2.52	3.98	1.31
994		95	90	77		100		15803	4.22	3.75	3.56	3.03	2.52	3.95	1.31
999		96	91	77		101		15874	4.25	3.76	3.58	3.02		3.97	1.31
1004		95	91	77		101		15959	4.26	3.76	3.58	3.02	2.52	3.97	1.31
S	152s		-17.			3151			6 55						
1019		92	88	77	65	92		16184	3.96	3.62	3.46	3.02	2.56	3.61	1.38
995	98	90	86	75	64	90		15807	3.87	3.53	3.38	2.95	2.51	3.54	1.37
1001	98	90	86	75	64	90		15903	3.88	3.54	3.37	2.96	2.52	3.54	1.36
999	98	90	86	75	64	90	35	15874	3.88	3.54	3.38	2.95	2.51	3.54	1.36

TOF