

OPTIMIZATION OF THE SIZE AND THE
LOCATION OF STEEL MEMBERS OF
A GRIDWORK BY MATHEMATICAL
PROGRAMMING

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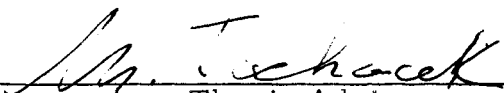
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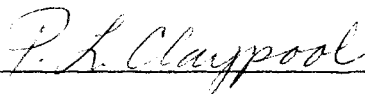
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NOMENCLATURE

a	Spacing of beams oriented in the y-direction, ("y-beams")
A	Dimension in the x-direction of a given rectangular area spanned by beams
$\{ A \}$	Column matrix of the unknown coefficients, (A_{11} A_{12} A_{21} A_{22})
A_f	Flange cross-sectional area
A_{mn}	Parameters associated with the assumed solution of deflection
A_t	Total cross-sectional area
A_w	Web cross-sectional area
b	Spacing of beams oriented in the x-direction, ("x-beams")
b_f	Flange width
b_o	Depth (width) of the narrow rectangular element
b_1	Constant
B	Dimension in the y-direction of a given rectangular area spanned by beams
BL_j	Lower bounds of x_j
B_1	Ratio of flange thickness to web thickness
B_2	Ratio of flange area to web area
C	Parameter changing the constraints from inequality to equality
$[C]$	A square matrix with elements defined by Formula (6.15)
C_1	Moment coefficient for simple supports and uniformly distributed load

C_2	Deflection coefficient for uniformly distributed load
C_3	Coefficient for deflection specified in building codes
C_4	Coefficient for critical stress
C_x, C_y	Unit torsional rigidities of the x- and y-beams
D	Decision function
$\{D\}$	Column matrix with four identical elements of $(p A B)$
D_x, D_y	Unit flexural rigidities of x-and y-beams
D_1, D_{xy}	Parameters associated with unit torsional rigidities
e_1, e_2	The distances from the neutral axis to the top and bottom fibers
E	Young's modulus of elasticity
$f(X)$	Function of X
$f(x)$	Loading function, varying in the x-direction
$F(X)$	Objective function
F_x	Flexural rigidity of each x-beam
F_y	Flexural rigidity of each y-beam
$g(y)$	Loading function, varying in the y-direction
$g_k(X)$	The k^{th} constraint
G	Shear modulus
$\bar{G}(X_1)$	Taylor series expansion of the constraint $G(X)$, evaluated at the first solution X_1
$G_k(X)$	The k^{th} constraint
h	Web depth
H	Parameter associated with unit torsional rigidities
i	Total number of constraints
I	Moment of inertia with respect to the horizontal principal axis

I_x	Moment of inertia of the x-beam with respect to the horizontal principal axis
I_y	Moment of inertia of the y-beam with respect to the horizontal principal axis
j	Total number of design variables
J	Torsional moment of inertia
J_x, J_y	Torsional moments of inertia for the x- and y-beams, respectively
K	Web coefficient
K_w	Web coefficient
K_1	Coefficient depending on the stress distribution
LIN	Number of linear constraints
m	Summation index for the x-direction
M	Maximum bending moment
MN	Number of variables
M_{tot}	The total moment, covering a strip of width b , being centered at M_i
M_x	Bending moment per unit width in x-beam
M_y	Bending moment per unit width in y-beam
M_{xy}	Torsional moment per unit width in x-beam
M_{yx}	Torsional moment per unit width in y-beam
n	Summation index for the y-direction
NNL	Number of nonlinear constraints
p	Uniformly distributed load (UDL)
$q(x, y)$	Loading function per unit area
R	The reaction on the x-beam
R_1	Ratio of the two sides of the grid
s	Ratio of B to b
S	Section modulus

S_x	Section modulus of x-beam
S_y	Section modulus of y-beam
S_1, S_2	Section moduli for the top fibers 1 and bottom fibers 2
t	Thickness of the narrow rectangular element
t_f	Flange thickness
u	Dimension in the x-direction of the local uniform load
U	Strain energy of bending
UB_j	Upper bounds of x_j
v	Dimension in the y-direction of the local uniform load
V	Total volume of all beams used
V_w	Virtual work
w	Deflection
w_{all}	Allowable deflection
x_1, x_2, \dots, x_j	Coordinates
X	A vector, (x_1, \dots, x_j)
X_A	Point A
X_B	Point B

α	Section asymmetry
β	Constant between zero and one
δ	Web thickness
δ_x	Web thickness of x-beam
δ_y	Web thickness of y-beam
λ	Web slenderness, i. e., web depth-thickness ratio
λ_x	Web depth-thickness ratio of the x-beam, (web slenderness)
λ_y	Web depth-thickness ratio of the y-beam, (web slenderness)
ν	Poisson's ratio
π	3.1416
σ	Maximum bending stress
σ_{all}	Allowable bending stress
σ_{cr}	Critical buckling stress
σ_{jt}	Combined stress at a joint
σ_x	Bending stress in x-beam
σ_y	Bending stress in y-beam
σ_{xy}	Maximum shearing stress
Σ	Summation sign
∇	Gradient
∂	Notation for partial derivatives
$[]$	Square matrix
$\{ \}$	Column matrix
$()^T$	Transpose of a matrix
$()_{,x}$ through $()_{,xxxx}$	First through fourth partial derivatives with respect to x

(), y through
(), $yyyy$

First through fourth partial derivatives with respect to y

(), xy and
(), $xxyy$

Second and Fourth mixed partial derivatives with respect to x and y

CHAPTER I

INTRODUCTION

1.1 Statement of the Problem

It is desired to optimize the location and the size of the steel members of a grid. For a particular combination of spacings and sizes of the steel members, the economical design of the grid is to be considered. It is evident that some design variables will originate from the key word "location" and some from the word "size." Although I-shaped steel plate girders are used throughout this investigation, other shapes can be handled using procedures developed herein. The approach to the optimal design of a grid is to use a suitable Structural Analysis method and to cast it into a form of Nonlinear Programming problem, usually referred to as NLP, so that a compatible NLP technique can be applied to solve the grid problem.

1.2 Historical Sketch

Optimization has long been industry's main concern. Contracts of construction or of manufactured products are made usually based on the lowest bid. The Armed Services started using optimization in its strategic decisions. G. B. Dantzig and M. Wood developed and applied the problem of Linear Programming, LP, for the U. S. Department of Air Force.(1). Publications on optimization can be found in Operation Research, Management Science, Economics, Industrial Engineering,

Aeronautical and Aerospace Engineering, Chemical Engineering, Civil Engineering, Mechanical Engineering, Applied Mathematics, and among various other science fields. A good account of optimization development in Civil Engineering can be found in papers by Sheu (2) and Wasiutynski (3). With increasing demands for solving complicated optimization problems, developments in Nonlinear Programming techniques have been flourishing.

Probably one of the earliest investigations in the Civil Engineering field using Mathematical Programming was the three-bar truss problem solved by Schmit (4). Some work was done on individual structural members by Goble (5), Razani (6), Mauch (7), Holt (8), and Seaburg (9). The building frame was first investigated through the use of plastic design by Pearson (10), and Livesley (11), whereby the simplified assumptions yielded linear constraints so that Linear Programming could be applied. As the NLP was becoming more popular, the building frame could be analyzed using elastic design with nonlinear functions; for example, in Ref. (12). Since it has taken some time to get plastic design theory up to a competitive level with elastic design theory, it is expected that this approach to optimization should earn its recognition in the Civil Engineering field in the similarly slow pace. At the present time, it still stays at the academic level, caused by rather slow demands for it. The reaction is mutual; the industry is not getting much stimulation from universities.

A historical survey on Mathematical Programming is included in Section 2.3.

1.3 Object of Investigations

Under the term "conventional design" of a structure, sizing of structural members is usually understood, the geometry of the structure being given or arbitrarily selected. On the other hand, in the present "optimum design" approach, the spacings of beams in the two directions of the grid are treated as design variables rather than given or preselected constants. Similarly, some other parameters (e. g. , section properties) may be treated as design variables to be optimized. The economy achieved by the optimum design is the advantage over the conventional design method, the latter being based rather on experience and intuition. The designer can choose his own analysis method and his available NLP technique to do the job. This point will be discussed further in Chapter II, together with the design procedure.

The theme of this investigation starts off with a "degenerated" grid, with beams in one direction only (Chapter III). Chapter IV considers first a grid with beams in two orthogonal directions, joined by connections without torsional resistance, and also a grid with torsion-carrying connections. Numerical examples for these three systems with simple supports are given in Chapter V. Clamped ends (fixed supports) are treated in Chapter VI. Also in Chapter VI, boundary dimensions and general loading conditions are discussed. Finally, Chapter VII discusses results and summarizes conclusions.

CHAPTER II

METHOD OF DESIGN

2.1 Introduction

The conventional design of a grid proceeds as follows: A preliminary design of members with the choice of their spacings is based on experience and engineering judgment. Proposed sections are checked for effects determined by methods of Structural Analysis and Strength of Materials to satisfy safety and functional requirements of official building codes. If the resulting safety factors are too high, the design might be modified for economy and the sections reduced in size. Sometimes several different beam spacings are considered and economy of different arrangements is studied. As computer usage is becoming popular, the structural engineer is relieved of tedious calculator and slide rule work. Modifications in the design are done by changing parts of the computer program, or even just a few data cards. However, with the use of standard computer programs such as the MIT ICES (13) packages, the conventional procedure of design has remained basically unchanged.

Only in recent few years research scientists have started to apply optimization methods of Mathematical Programming to structural design: A physical model is set up and cast into a mathematical model. The physical model is formulated through Structural Analysis and Strength of Materials, and then transformed into a mathematical

model treatable by Mathematical Programming. The objective function, the goal, is introduced, for instance, to minimize the cost of the structural system, or its volume. (The total cost of the structure varies from company to company and from city to city. Therefore, for the sake of generality, the volume of the material used is selected for the objective function in this study.) The functional and safety requirements define the so-called constraints. Techniques of Mathematical Programming enable one to arrive from a starting point within the feasible region (the preliminary design) at a final point (the optimum design).

2.2 Selection of the Structural Analysis Method

At the very beginning of this investigation, a Structural Analysis method must be chosen such that it will be suitable to be incorporated with the Mathematical Programming model. Various methods are available to treat the grid as

- (a) a discrete system, or
- (b) a continuous system.

(a) In the first group, the displacement method can be used to calculate deflections and moments. If evaluations of functions involved were done just a few times, the selection of Structural Analysis methods would not make too much difference. However, the Mathematical Programming technique requires many function evaluations; hence, the choice of a suitable Structural Analysis method will play an important role, especially in the consumption of computer time. The displacement method also depends on the number of nodes; its matrix operation, especially inversion, requires more execution time besides storage time as the number of nodes increases.

(b) In the second group, another possibility is to treat the grid as an orthotropic plate. Computation of deflections and moments is done by algebraic manipulation with double or single series. As far as computer time is concerned, this approach is preferred because the previous method (with too many redundant quantities) requires more time. Therefore, in this study the grid will be designed using the Theory of Plates.

2.3 Selection of the Mathematical Programming Technique

In the mathematical language, the problem can be formulated as follows:

$$\begin{aligned} \text{Let } X &= (x_1, x_2, \dots, x_j), \\ j &= \text{total number of design variables, and} \\ i &= \text{total number of constraints.} \end{aligned}$$

Minimize¹ the objective function $F(X)$ subject to constraints

$$G_k(X) \geq 0, \quad k = 1, \dots, i \quad (2.1)$$

for the variables $X \geq 0$. Both the objective function $F(X)$ and the constraints $G_k(X)$ are nonlinear in this study; hence, the Linear Programming technique cannot be used directly and Nonlinear Programming must be applied.

Nonlinear Mathematical Programming techniques fall into five main categories:

- (1) Methods to solve a problem with a nonlinear objective function but linear constraints,

¹Minimization of a function $F(X)$ is the same as maximization of the negative function $-F(X)$.

- (2) Sequential Unconstrained Minimization Technique (SUMT),
- (3) Sequence of Linear Programming Solutions (SLP),
- (4) Direct Search,
- (5) Totally Nonlinear Programming.

2.3.1 First category

The objective function is approximated by a quadratic function and the algorithm is based on linear constraints. The method of solution was developed mainly by Zoutendijk (14) in 1960. An up-to-date reference on the method is given by Abadie (15) and Wolfe (16). They are among the pioneers who conducted researches along this line. In their adaptation, the eventual nonlinear constraints are linearized by Taylor series expansion. Successive iterations remove errors resulting from the truncation of terms of higher degrees. The accuracy of the method depends on the degree of nonlinearities of the objective function and the constraints. Computer Codes for this technique for a general case have not been released yet; a code is being developed by RAND Corporation, Santa Monica, California (17). IBM Corporation has a code, called "Separable Programming" (18).

2.3.2 Second category

Some researchers started the idea of Sequential Unconstrained Minimization Technique (SUMT) about 1959 but did not go into depths until Fiacco and McCormick (19), (20), and (21) developed the details and a computer code of the method under an Army research contract between the years 1963 (19) and 1965 (20). The constraints are incorporated with the objective function in the so-called penalty function and

the algorithm treats the problem as an unconstrained one. A sample problem was solved in (20) with a cubic objective function and linear constraints. If a problem is highly nonlinear in both the objective function and the constraints, complications can be expected.

2.3.3 Third category

Kelly (22) suggested in 1960 a sequence of linear programming expressions, SLP, to approximate a nonlinear constraint set by a linear set, while the objective function is linear. He named his technique "the Cutting Plane Method" (22). A brief description follows in Section 2.5.

2.3.4 Fourth category

The Direct Search method does not require any quadratic or linear approximation. The main problem is the choice of the mesh density and of the methods of successively reducing the feasible region during the solution. Massachusetts Institute of Technology developed a package within the ICES, called OPTECH (13), in 1968. It works well for an unconstrained problem; however, no hints are given to handle a constrained problem.

The IBM Corporation has a code, termed the Ricochet Gradient Method (RGM) due to Mertz (23), released in 1967. He used the Steepest Descent Method with some modifications (23). A brief description of the RGM is in Section 2.4.

2.3.5 Fifth category

In recent years, researchers have been trying to improve the Totally Nonlinear Programming; a stimulation has been evident. Three techniques are mentioned here in passing: Geometric Programming by Duffin, Peterson, Zener (24) in 1967; Generalized Reduced Gradient Method by Abadie (15) in 1966; and Dynamic Programming by Bellman (25) and (26) in 1957.

Since Geometric Programming depends on the manipulation with the exponential powers of the variables appearing in the functions involved, an immediate drawback is when the method is applied to a problem with fractional functions.

The Generalized Reduced Gradient Method promises to be superior in both speed and accuracy.

Naturally, after a structural problem has been cast into a Mathematical Programming model, one can choose any suitable technique. From the viewpoint of availability and compatibility with the Structural Analysis method chosen, the Ricochet Gradient Method (RGM) and the Sequence of Linear Programming Solutions (SLP), from the fourth and third categories respectively, seem to be most suitable for this study.

The two methods chosen are explained in more detail in Sections 2.4 and 2.5.

2.4 The Ricochet Gradient Method (RGM)

The Ricochet Gradient Method, RGM Code, by Mertz (23), is essentially a **Direct** Search method using the Gradient Method, also

known as the Steepest Descent¹ Method. It solves a Nonlinear Programming problem (Formula 2.1 in Section 2.3). Consider a two-dimensional case (Fig 1). The routine starts from a feasible point X_0 and follows the path of steepest descent or the steepest gradient of the objective function. If a local minimum is inside the feasible region, then this path will lead to the local minimum. Otherwise, the path will lead to a nearby constraint at point B.

Consider Fig 2 which shows one constraint by itself. It is desired to locate a point B on the constraint function, $G_k = 0$. At point A in Fig 2 the constraint function value is $G_k > 0$; at point C its value is $G_k < 0$. Interpolation locates approximately the point B desired, at which $G_k = 0$.

The path is reflected (ricochets), following the constant value of the objective function, and heads towards another constraint across the feasible region. A point D is obtained in the same manner as point B. The line BD and the two tangents drawn at B and D form a triangle. With two base angles and the base of the triangle known, the point X_1 is determined and used as a starting point for the next cycle of the process. The process is repeated until an "optimum solution" is obtained.

A detailed description of this method and a listing of the Computer Code can be found in (23); therefore, no attempt is made to describe the method any further.

¹Ascent for maximization, descent for minimization.

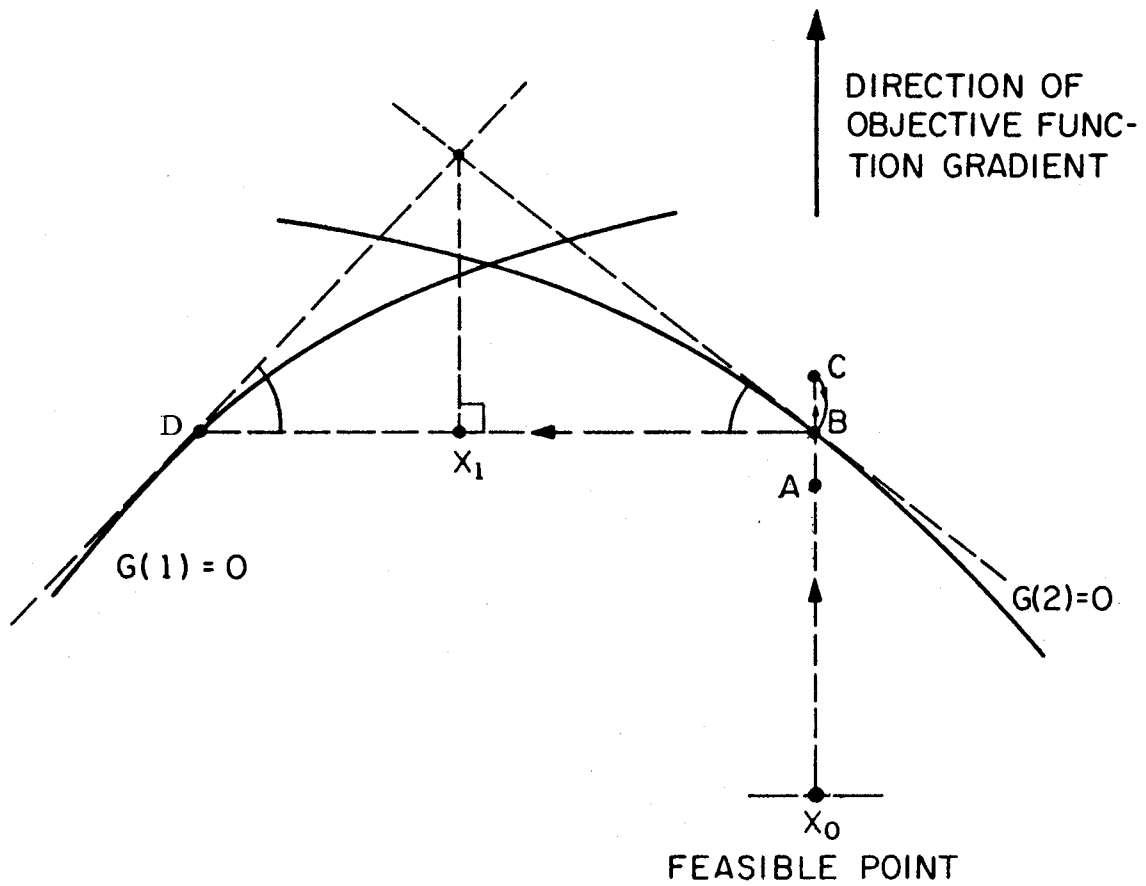


Figure 1. Path of Ricochet Gradient Method

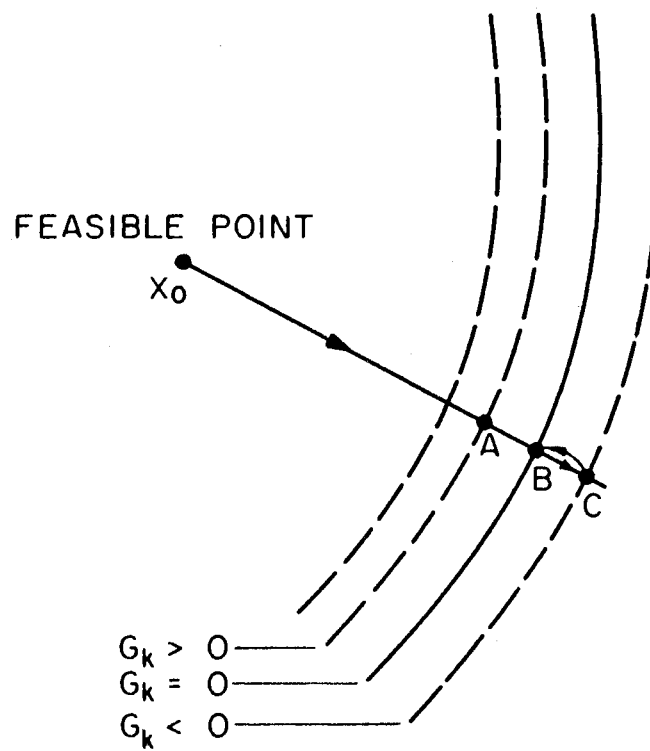


Figure 2. Contours of One Constraint Function

2.5 The Modified Sequence of Linear Programming Solutions Technique (MSLP)

Since the Gradient Method and Linear Programming have been well-documented in the literature, no attempt is made in this section to make a superfluous description of them. The Gradient Method used in Ricochet Gradient Method (RGM) was documented by Mertz (23). The Sequence of Linear Programming Solutions (SLP) was explained in (22); the Linear Programming subroutine used in MSLP was documented by Kuo (27).

Two things are changed in the SLP so that the modified version will be called "the Modified Sequence of Linear Programming Solutions Technique" (MSLP). The first modification is that the objective function is also linearized. The second modification is that the nonlinear constraint set is approximated by a set of lines tangent to the nonlinear constraint set, for example in a two-dimensional case (Fig 3).

A numerical example (22) is given to illustrate the "Cutting Plane Method" (Fig 4):

$$\text{Let } X = (x_1, x_2, \dots, x_j),$$

$$(x_1, x_2, \dots, x_j) = \text{coordinates.}$$

Minimize $F(X) = x_1 - x_2$ subject to the constraint

$$G(X) = 3x_1^2 + 2x_1x_2 + x_2^2 - 1 \leq 0$$

First, a large square, representing the upper and lower bounds of the variables, is placed to enclose the feasible region. Initially we solve the linear problem:

Minimize $F(X) = x_1 - x_2$ subject to the constraints

$$-2 \leq x_1 \leq 2 \text{ and } -2 \leq x_2 \leq 2.$$

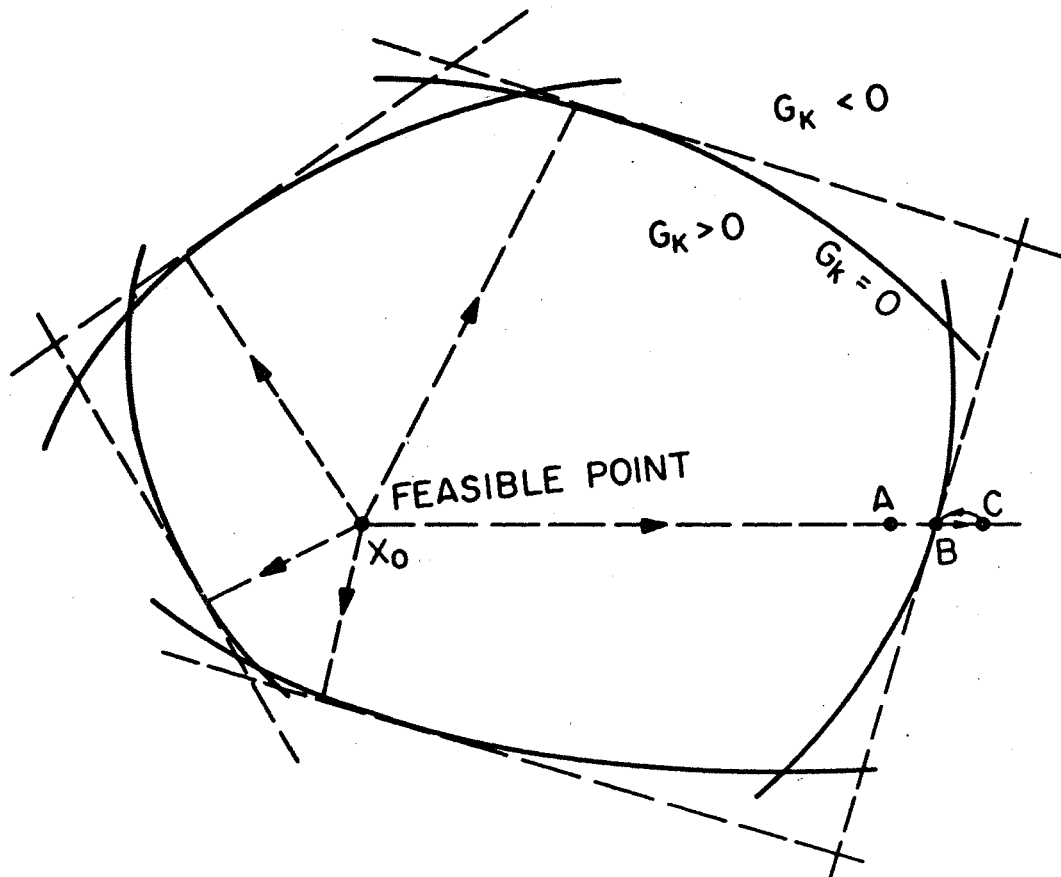


Figure 3. Tangent Lines Enclosing the Nonlinear Constraint Set

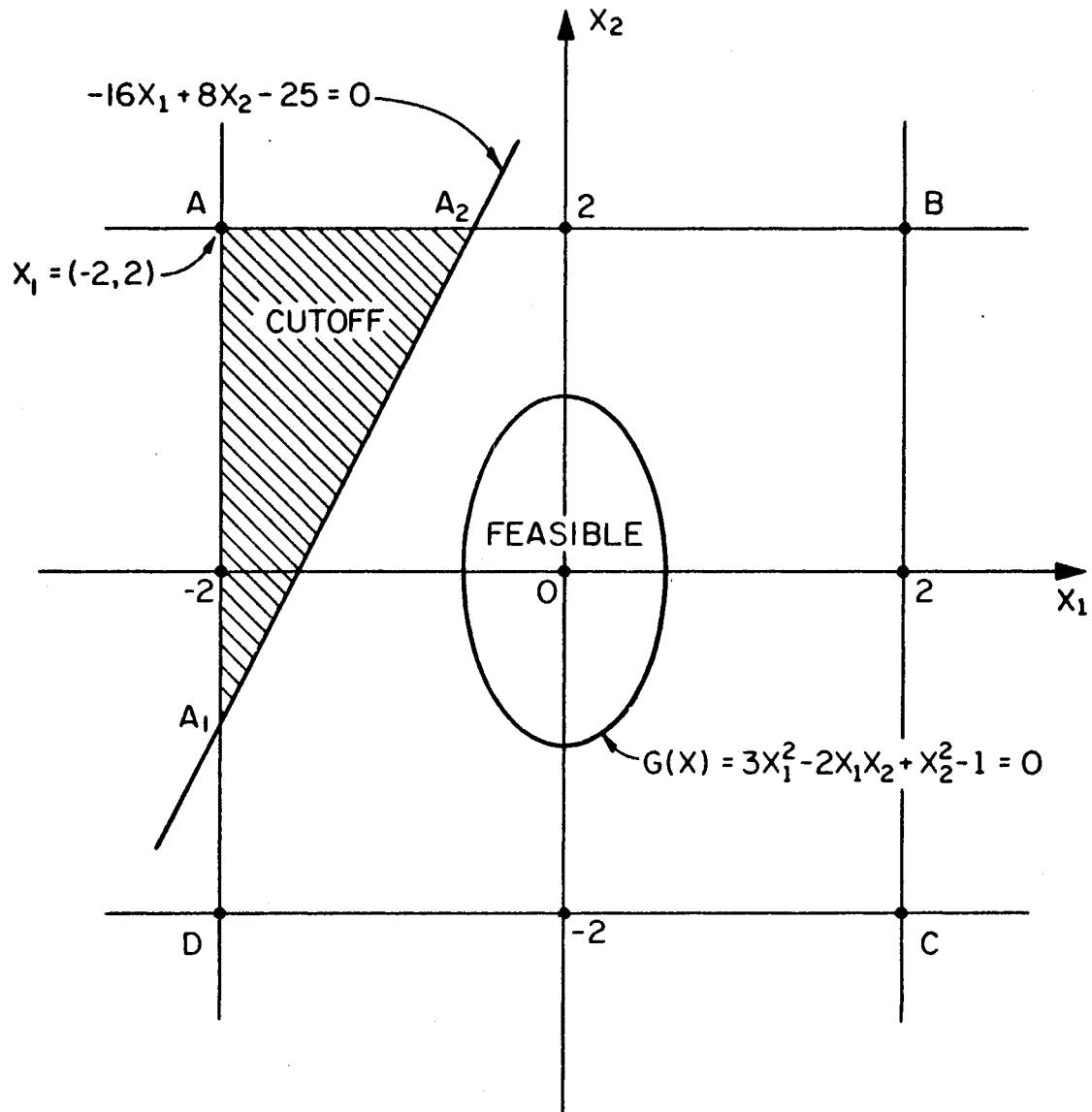


Figure 4. Example Illustrating the Cutting Plane Method

Its solution is $X_1 = (-2, 2)$ and $F(X_1) = -4$.

Evaluate the constraint value at the first solution X_1 , resulting in

$$\bar{G}(X_1) = -16x_1 + 8x_2 - 25 \leq 0$$

where $\bar{G}(X_1) =$ Taylor series expansion of the constraint $G(X)$,
evaluated at the first solution X_1 .

In general,

$$\bar{G}(X) \approx G(X_1) + \nabla G(X_1)^T (X - X_1)$$

where ∇ designates gradient, and T means transpose.

It is observed that the initial domain has been reduced from the square ABCD to the polygon A_1A_2BCD or a portion of the initial domain (triangle AA_1A_2) has been cut off (Fig 4). This process of a sequence of linear programming solutions will finally lead to close "contact" with the nonlinear constraint.

Returning to the second modification, two approaches to form a set of tangent lines are considered:

- (1) an analytical method, and
- (2) a numerical method.

Observing only one constraint (Fig 2) in two-dimensional space (Fig 3), in the analytical approach a normal to the boundary curve is passed through a starting point (feasible point) and at the intersection of this normal with the boundary curve a tangent is considered. (In a multi-dimensional space, curves and tangent lines are replaced by hypersurfaces and tangent hyperplanes.) A parametric representation of the normal in space, the linear representation of the space coordinates, and the equation of the nonlinear constraint constitute a set of simultaneous expressions, a mixture of nonlinear and linear equations. Fortunately, substitutions lead to only one nonlinear equation with only

one unknown, the parameter from the parametric expression of the normal in space. This equation can be solved by methods such as Wegstein's (28), Newton's, Bailey's and Aitken's iterations (29), (30), etc. Once this parameter is computed, the coordinates will be known by back substitution. The solution represents the intersection point B desired (Fig 2). At point B a tangent is drawn.

In the numerical approach there is a subroutine available (31) using the Fletcher and Powell numerical method (32). Consider again any one of the constraints in Fig 3 separately as shown in Fig 2. The routine solves an unconstrained problem. It is desired to locate a point B on the constraint function, $G_k = 0$. The routine starts from a feasible point and follows a path of decreasing values of the constraint function. At point A in Fig 2 the constraint function value is $G_k > 0$; at point C its value is $G_k < 0$. Interpolation locates approximately the point B desired, at which $G_k = 0$.

After point B has been obtained, a tangent line can be expressed at this point, e. g., by expanding the constraint function in Taylor series. This process is to be repeated for all other constraints. A closed set of tangent lines is so formed (Fig 3). With this polygon of linear constraints as an approximation of the nonlinear constraint set, Linear Programming can now be applied.

2.6 Computer Code MSLP

The Computer Code MSLP (Modified Sequence of Linear Programming Solutions) is used to solve the optimum design of a gridwork, which mathematically is a Nonlinear Programming problem of the form as Formula (2.1). The objective function is nonlinear, and the

constraint set is a mixture of nonlinear and linear functions. The MSLP Code is listed in Appendix A.

The Code is based on Kelly's method (22) with modifications; a sequence of linear programming problems is used as an approximation. In the beginning, a set of hyperplanes tangent to the set of nonlinear constraints is formed. The limitation on the Code is that when a variable has a small range between its upper bound and lower bound it must be made constant, being a close approximation anyway, and that the Code works for convex programming only.

Outline of the MSLP (Fig 5)

MAIN PROGRAM

(1) MAIN

SUBROUTINE	SUBROUTINE	SUBROUTINE
(2) PTCON	(7) BNEG1	(10) FCNGR
(3) FPINTP	(8) CALA2	(11) LFCN
(4) FPM	(9) LP2	
(5) FUNCT		
(6) INTP		

In the Computer Code MSLP, the MAIN program controls the iterations with the aim to generate a sequence of linear programming solutions until convergence. In addition, three groups of subroutines are employed. The first group of subroutines is used to form a closed set of hyperplanes tangent to the nonlinear constraint set. The second group sets up the Mathematical Programming problem so that the Linear Programming technique can be applied in subroutine LP2. The

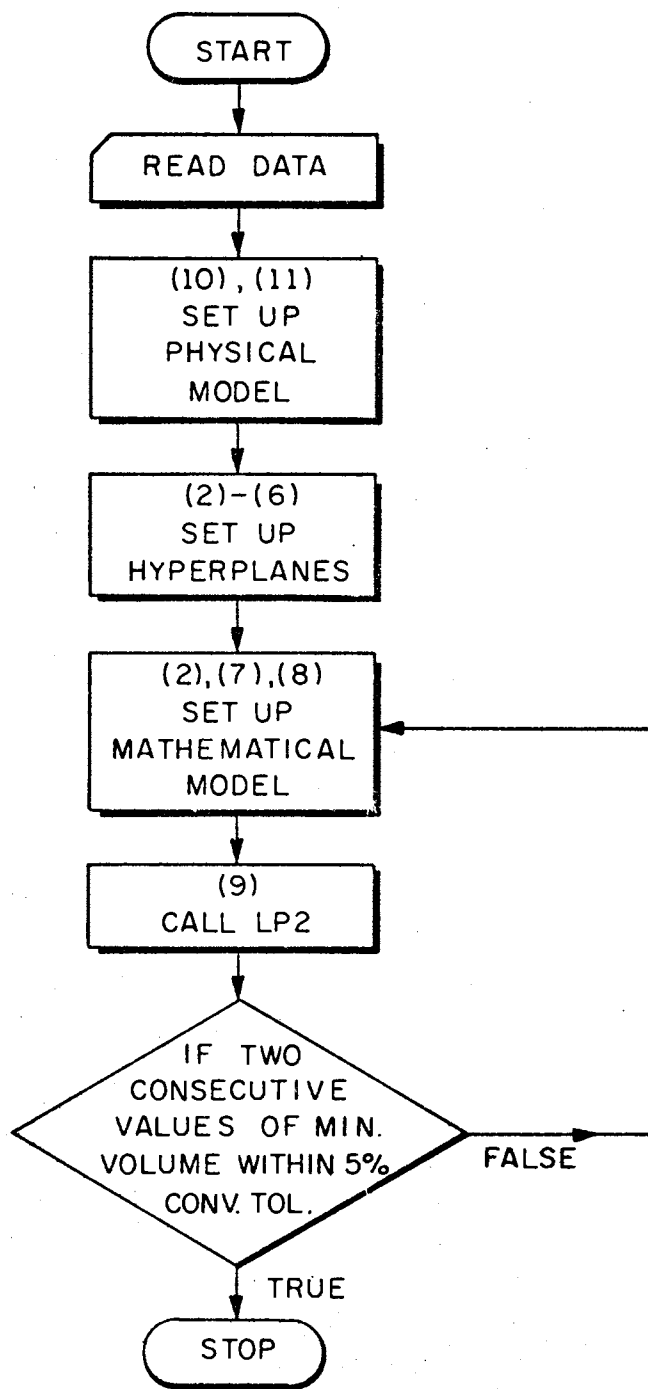


Figure 5. Block Diagram,
"Modified Sequence
of Linear Program-
ming Solutions
Technique"

third group is supplied by the user to calculate values of the functions involved and their first partial derivatives.

The distinct parts of the program are now explained in more detail:

MAIN Program It reads in data: the initial point, number of variables and number of linear and nonlinear constraints. A closed set of hyperplanes tangent to the set of nonlinear constraints is formed. It goes through a sequence of Linear Programming problems. Upon convergence, the values of minimum objective function F of two consecutive iterations differing by less than five percent, the process is stopped with final solution, final point, and minimum value of objective function F printed out.

First group of subroutines

PTCON It controls the formulation of Linear Programming, starting with a set of hyperplanes as a constraint set by calling FPINTP.

FPINTP It furnishes a set of hyperplanes tangent to the nonlinear constraint set.

FPM This is a modified version of the subroutine FMFP (31) which is an unconstrained function minimization routine due to Fletcher and Power (32).

FUNCT This is required by FPM to calculate function values and their first partial derivatives.

INTP As soon as the constraint value steps over zero (Fig 2) to the negative values, this point C and the

previous point A (for which the constraint was positive) are interpolated; thus point B is obtained for which the constraint value is approximately equal to zero.

Second group of subroutines

BNEG1 Owing to the nature of the problem, the constant term b_1 could be negative. The constraints,

$$G_k(X) \geq 0, \quad k = 1, \dots, i, \quad (2.2)$$

can be rewritten as

$$g_k(X) \geq b_1, \quad k = 1, \dots, i. \quad (2.3)$$

This subroutine will keep them in this standard form.

CALA2 It transforms the problem in BNEG1 to the form required by LP2.

LP2 It is the Linear Programming problem solved by Kuo (27), with modifications.

Third group of subroutines (user-supplied)

FCNGR Given an initial point, the number of variables and the number of nonlinear constraints, it calculates the values of the objective function, of the nonlinear constraints, and of their first partial derivatives. It is the formulation of the structural design problem.

LFCN Given the number of variables, the number of linear constraints and the upper and lower bounds, it forms a set of linear constraints.

See Appendix B. The listing of PROBLEM 2 is omitted because it is like PROBLEM 3 with the parameter H and M_{xy} being set equal to zero.

2.7 The Design Procedure

The design procedure can be summarized in the following steps:

Step 1--- Formulate the structural problem by the Theory of Plates, or by other Structural Analysis methods.

Step 2--- Cast it into the form of a Mathematical Programming problem.

Step 3--- Use the gradient technique or other techniques to solve the Mathematical Programming problem.

Step 4--- After the local minimum has been found, one can either be satisfied with the local minimum or try to search for more local minima by using several starting points. The comparison of the local minima can lead to a smaller value of the objective function and in many practical problems even to the global minimum.

The feasibility of this design procedure is evident. After the minimum number of design variables and that of constraints have been determined, one can still apply engineering judgment and reasonable assumptions to reduce that number. This choice has to be made by the designer to see whether the extra computer time could be compensated by the expenses due to extra preparation time and also sacrifice of fully automated procedure. Up to now, Step 4 is probably the best one available for finding a minimum.

2.8 Convexity

Convex programming (33) is defined as one where both the objective function and constraint functions are convex. A function $f(X) = f(x_1, x_2, \dots, x_j)$, where $X = (x_1, x_2, \dots, x_j)$ is a vector and (x_1, x_2, \dots, x_j) are coordinates is a convex function if, for any

two points X_A and X_B and for $0 \leq \beta \leq 1$,

$$\beta f(X_A) + (1 - \beta)f(X_B) \geq f[\beta X_A + (1 - \beta)X_B]. \quad (2.4)$$

Alternately, $f(X)$ is convex if

$$f(X_A) - f(X_B) \geq (X_A - X_B)^T \nabla f(X_B) \quad (2.5)$$

where T means transpose, and ∇ designates gradient.

Figure 6 shows the constraint G_1 being a convex function and the other constraint G_2 being a nonconvex function.

The Sequential Unconstrained Minimization Technique (SUMT) was originally developed for convex programming but it was reported that it also worked for some nonconvex programming problems.

The Ricochet Gradient Method (RGM) always works for convex programming problems but for non-convex programming it might run into difficulties. In the latter case, the Modified Sequence of Linear Programming Solutions (MSLP) might not even work.

The numerical examples in Chapter V will be tested for convexity using the second definition, Formula (2.5).

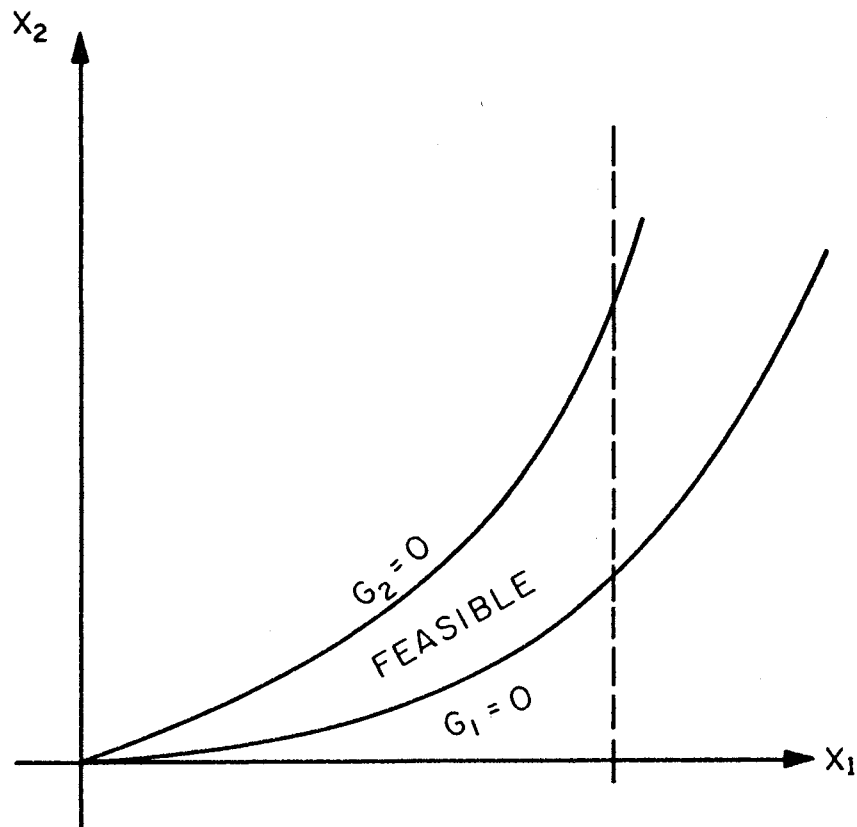


Figure 6. Illustration of Convex (G_1) and Nonconvex (G_2) Functions

CHAPTER III

BEAMS SPANNING IN ONE DIRECTION

PROBLEM 1

3.1 Section Properties

Section properties described in this section are used as design variables. Brown and Ang (12) plotted the section modulus S vs. the moment of inertia I , using the WF rolled sections in the American Institute of Steel Construction Manual. By fitting a curve over the plot, the discrete relationship was changed to a continuous function, $S = S(I)$.

However, a more general approach can be used, appropriate for both welded I-sections and rolled beams. Tochacek (34) expressed all section properties for an unsymmetric I-section in terms of three parameters: Beam asymmetry α , web slenderness λ , and web coefficient K .

$$S_1 = \frac{6\alpha - K(\alpha + 1)^2}{6(\alpha + 1)} (\lambda K A_t^3)^{0.5} \quad (3.1)$$

$$S_2 = \frac{S_1}{\alpha} \quad (3.2)$$

$$A_w = K A_t \quad (3.3)$$

$$I = \frac{6\alpha - K(\alpha + 1)^2}{6(\alpha + 1)^2} \lambda K A_t^2 \quad (3.4)$$

where

$$S_1 = \frac{I}{e_1}, \quad S_2 = \frac{I}{e_2}, \quad (3.5)$$

S_1, S_2 = section moduli for the top fibers 1 and bottom fibers 2,

I = moment of inertia with respect to the horizontal principal axis,

e_1, e_2 = the distances from the neutral axis to the top and bottom fibers,

$$\alpha = \frac{S_1}{S_2} = \frac{e_2}{e_1} = \text{section asymmetry,}$$

$$\lambda = \frac{h}{\delta}, \quad (3.6)$$

λ = web slenderness, i. e., web depth-thickness ratio,

h = web depth,

δ = web thickness,

A_t = total cross-sectional area,

$$A_w = h\delta, \quad (3.7)$$

A_w = web cross-sectional area,

$$K = \frac{A_w}{A_t} = \text{web coefficient.}$$

Assuming equal allowable stress σ_{all} for the bottom as well as for the top fibers, the I-section becomes symmetrical and $\alpha = 1$ or

$S_1 = S_2 = S$. Formula (3.1) reduces to

$$S = (0.5 - 0.333K)(\lambda K A_t^3)^{0.5}. \quad (3.8)$$

The strength condition for flexure reads:

$$\sigma = \frac{M}{S} \leq \sigma_{all} \quad (3.9)$$

where

- σ = maximum bending stress,
 σ_{all} = allowable bending stress,
 M = maximum bending moment,
 S = section modulus.

For the fully stressed, most economical design,

$$M = \sigma_{\text{all}} S = \sigma_{\text{all}} (0.5 - 0.333K) (\lambda K A_t^3)^{0.5}. \quad (3.10)$$

A constant cross-sectional area A_t is assumed throughout the beam span and for the beams used. For the given amount of material

($A_t = \text{constant}$), the maximum resisting moment M is achieved if

$$K = 0.5,$$

$$\lambda = \text{maximum},$$

the latter value to be discussed later.

Substituting $\alpha = 1$, $K = 0.5$ into Formulas (3.4) and (3.8),

$$I = \frac{0.5 \lambda A_t^2}{6}, \quad (3.11)$$

or

$$A_t = 3.46 \frac{I^{0.5}}{\lambda^{0.5}}, \quad (3.12)$$

$$S = 0.235 \lambda^{0.5} A_t^{1.5}, \quad (3.13)$$

or from Formula (3.12)

$$S = 1.51 \frac{I^{0.75}}{\lambda^{0.25}}. \quad (3.14)$$

As obvious from Formulas (3.12) and (3.14) two independent section properties, i. e., I and λ , will be decisive for the sizing of beams, besides other parameters described later, e. g., b , $\frac{A}{B}$, etc.

3.2 Design Variables

It is desired to span beams in one direction over a rectangular area of specified dimensions $A \times B$ (Fig 7) in such a manner that the total volume of beams will be minimum. Such a design is known as an optimum design. The design procedure starts out with specifications to satisfy safety and functional requirements, goes through a physical model which is transformed into a mathematical model, and ends up with a solution.

Given: A = dimension in the x-direction of a given rectangular area spanned by beams,

B = dimension in the y-direction of a given rectangular area spanned by beams,

p = UDL = uniformly distributed load,

Simple supports,

Symmetrical I-shaped welded plate girders (or possibly WF-section or another rolled section).

Required: to optimize the location and the size of beams using the following variables:

b = spacing of beams oriented in the x-direction ("x-beams"),

I_x = moment of inertia of the x-beam with respect to its horizontal principal axis,

λ_x = web depth-thickness ratio of the x-beam (web slenderness).

3.3 Design Criteria

Although other secondary criteria such as shear and lateral buckling could be included as constraints, only three primary criteria for a safe design would be considered:

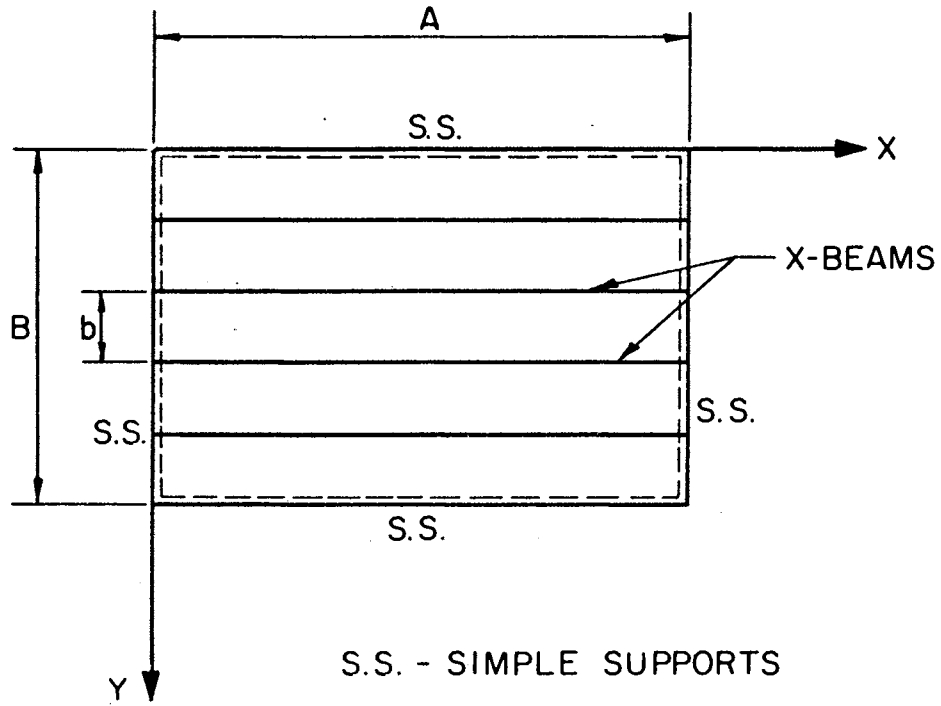


Figure 7. Beams Spanning in One Direction

- (1) flexural strength,
- (2) deflection,
- (3) web buckling.

Shear can be checked by supplemental calculations, and lateral buckling is frequently insignificant in gridworks with compressive flanges supported laterally by slabs and the like.

3.3.1 Flexural Strength

The first criterion as a constraint implies that

$$\sigma_{\text{all}} - \sigma \geq 0 \quad (3.15)$$

rewritten from Formula (3.9). Maximum moment

$$M = C_1 p b A^2 \quad (3.16)$$

where for simple supports and for uniformly distributed load $UDL = p$, $C_1 = \frac{1}{8}$.

Symbols b and A have been explained in Section 3.2. Loadings other than a uniform load will be discussed in Chapter VI. Substitution of Formulas (3.14) and (3.16) in Formula (3.9) yields the first constraint,

$$G_1 = 1.51 \frac{I_x^{0.75}}{\lambda_x^{0.25}} - \frac{C_1 p b A^2}{\sigma_{\text{all}}} \geq 0. \quad (3.17)$$

3.3.2 Deflection

The maximum deflection of a beam should not exceed the allowable deflection; i. e.,

$$w_{\text{all}} - w \geq 0. \quad (3.18)$$

The maximum deflection,

$$w = C_2 \frac{pbA^4}{EI_x} \quad (3.19)$$

where $C_2 = \frac{5}{384}$ for uniformly distributed load UDL = p
and simple supports,

E = Young's modulus of elasticity.

The allowable deflection,

$$w_{all} = C_3 A \quad (3.20)$$

with C_3 = coefficient for deflection specified in building codes. In the forthcoming numerical calculations, mainly the value of 1/360 will be considered as a characteristic one.

Substitution of Formulas (3.19) and (3.20) in Formula (3.18) leads to the second constraint,

$$G_2 = C_3 A - \frac{C_2 pbA^4}{EI_x} \geq 0. \quad (3.21)$$

3.3.3 Web Buckling

The third criterion considered is web buckling. Buckling problems have been investigated by numerous authors. Some results are accumulated, e. g., in the book by Beedle, et al (35), or in the USS Steel Design Manual (36). The girder web can be treated as a plate with the in-plane loading as in (37). The main result of the solution is the critical stress σ_{cr} , under which an ideal plate would buckle. Because of the post-critical reserve in the load-carrying capacity of a compressed plate, the safety factor 1.0 for σ_{cr} is sufficient in the elastic range. Then the buckling condition reads

$$\sigma_{cr} - \sigma \geq 0 \quad (3.22)$$

where the critical buckling stress

$$\sigma_{cr} = K_1 \frac{\pi^2 E}{12(1 - \nu^2)\lambda^2} \quad (3.23)$$

with

$$\pi = 3.1416,$$

$$\nu = \text{Poisson's ratio,}$$

$$K_1 = \text{coefficient depending on the stress distribution and support condition of unstiffened web: the value of 23.9 (36) is considered in the forthcoming numerical examples, corresponding to the unstiffened, simply supported web with the typical linear bending stress distribution.}$$

Let

$$C_4 = K_1 \frac{\pi^2 E}{12(1 - \nu^2)} \quad (3.24)$$

then the critical stress in Formula (3.23) is

$$\sigma_{cr} = C_4 \frac{1}{\lambda^2} \quad (3.25)$$

and Formula (3.22) reads

$$\frac{C_4}{\lambda^2} - \frac{M}{S} \geq 0. \quad (3.26)$$

From Formulas (3.14) and (3.16) and rearranging, the third constraint results in

$$G_3 = \frac{C_4}{\lambda^2} 1.51 \frac{I_x^{0.75}}{\lambda_x^{0.25}} - C_1 p b A^2 \geq 0. \quad (3.27)$$

3.3.4 Objective Function

The objective of this chapter is to minimize the total volume V of all beams used over the area $A \times B$. Hence, it is the volume V that has the character of the objective function F as explained in Section 2.3.

If two beams are located on the two side boundaries (e. g., supported by corner columns), then the total number of beams is equal to $(\frac{B}{b} + 1)$. If the beams are placed only inside the area but not on the sides, then the total number of beams is equal to $(\frac{B}{b} - 1)$. As the side beams are usually not employed, frequently being replaced by walls, only the latter case will be considered. With the girder area A_t from Formula (3.12), the total volume V of all beams used is

$$V = (\frac{B}{b} - 1) (3.46 \frac{I_x^{0.5}}{\lambda_x^{0.5}}) A . \quad (3.28)$$

3.4 Mathematical Model

From Formulas (3.28), (3.17), (3.21), and (3.27), the problem can be formulated in a mathematical model as follows:

PROBLEM 1

Minimize the objective function

$$F(X) = V = (\frac{B}{b} - 1) (3.46 \frac{I_x^{0.5}}{\lambda_x^{0.5}}) A \quad (3.28)$$

subject to the constraints

$$G_1 = 1.51 \frac{I_x^{0.75}}{\lambda_x^{0.25}} - \frac{C_1 p b A^2}{\sigma_{all}} \geq 0 \quad (3.17)$$

$$G_2 = C_3 A - \frac{C_2 p b A^4}{EI_x} \geq 0 \quad (3.21)$$

$$G_3 = \frac{C_4}{\lambda^2} 1.51 \frac{I_x^{0.75}}{\lambda_x^{0.25}} - C_1 p b A^2 \geq 0 \quad (3.27)$$

with the variables $x_i = b, I_x, \lambda_x$ for $i = 1, 2, 3$ satisfying the

non-negativity constraints,

$$\frac{B}{2} \geq b \geq 0, \quad (3.29)$$

$$I_x \geq 0, \quad (3.30)$$

$$\lambda_x \geq 0. \quad (3.31)$$

It is observed that this is a Nonlinear Programming problem with both the objective function and the constraints nonlinear. First the Modified Sequence of Linear Programming Solutions (MSLP) computer program was applied to the problem described in Ex (5.6) without the upper bound of Formula (3.29), but failed, thus indicating an unbounded solution. It was evident that an open set caused the trouble. Also, the lower bounds equalling zero in Formulas (3.29) to (3.31) are not acceptable for practical reasons; e.g., $b = 0$ (zero spacing between two beams) represents a continuous structural system (a slab), for which some applied expressions do not hold true anymore.

The upper and lower bounds on the variables are further considered in Section 3.5. Being subject to removal if necessary, constraints of Formulas (3.29) to (3.31) can generally be reformulated as

$$BL(1) \leq b \leq UB(1), \quad (3.32)$$

$$BL(2) \leq I_x \leq UB(2), \quad (3.33)$$

$$BL(3) \leq \lambda_x \leq UB(3). \quad (3.34)$$

These lower and upper bounds form a large envelope as the starting basis of the Cutting Plane Method (Fig 4). Although a more rigorous investigation of the need of all these bounds in Section 3.5 reveals that only an upper bound on b (or on another variable as in Fig 6) is absolutely necessary, together with one lower bound on any

of the three variables (b , I_x , λ_x), the other bounds turn out to be helpful rather than obstructive, enabling one to observe some practical requirements.

3.5 Analytical Approach

3.5.1 Parameters in the Objective Function

The relative simplicity of PROBLEM 1 enables an analytic solution, permitting one to derive some quantitative results useful for checking numerical solutions of more involved problems. The section properties, derived from Formulas (3.1) to (3.4), originated from three parameters: α , λ , and K . For symmetry, $\alpha = 1$, Formula (3.8) holds true. The strength condition reads

$$\sigma = \frac{M}{S} = C\sigma_{\text{all}} \quad (3.35)$$

where

C is the parameter changing the constraints from inequality to equality, and

$$C \leq 1.$$

Here the inequality sign indicates that the design is not fully stressed (e.g., when deflection or buckling is decisive rather than flexural strength). Substituting Formula (3.8) into Formula (3.35), the resisting moment

$$M = C\sigma_{\text{all}}(0.5 - 0.333K)(\lambda K A_t^3)^{0.5}. \quad (3.36)$$

Letting $s = \frac{B}{b}$, and rewriting Formula (3.16), the maximum bending moment

$$M = C_1 p A^2 \frac{B}{s}. \quad (3.37)$$

Equating Formula (3.36) and Formula (3.37) and rearranging,

$$A_t^3 = \left(\frac{C_1 p A^2 B}{\sigma_{all}} \right)^2 \frac{1}{C^2 \lambda s^2} \frac{1}{K(0.5 - 0.333K)^2} \quad (3.38)$$

Rewriting Formula (3.28),

$$V = (s - 1) A_t A, \quad (3.39)$$

or

$$\left(\frac{V}{A} \right)^3 = (s - 1)^3 A_t^3. \quad (3.40)$$

Multiplying both sides by a constant,

$$\left(\frac{\sigma_{all}}{C_1 p A^2 B} \right)^2 \left(\frac{V}{A} \right)^3 = (s - 1)^3 A_t^3 \left(\frac{\sigma_{all}}{C_1 p A^2 B} \right)^2. \quad (3.41)$$

Since the term on the left-hand side of Formula (3.41) is the cube of **the objective** function V^3 premultiplied by a constant, it can be called a decision function D , and

$$D = (s - 1)^3 A_t^3 \left(\frac{\sigma_{all}}{C_1 p A^2 B} \right)^2. \quad (3.42)$$

Multiplying both sides of Formula (3.38) by $(s - 1)^3 \left(\frac{\sigma_{all}}{C_1 p A^2 B} \right)^2$,

$$(s - 1)^3 A_t^3 \left(\frac{\sigma_{all}}{C_1 p A^2 B} \right)^2 = \frac{(s - 1)^3}{C^2 \lambda s^2} \frac{1}{K(0.5 - 0.333K)^2}. \quad (3.43)$$

Comparing Formula (3.42) with (3.43), the decision function finally reads

$$D = \frac{(s - 1)^3}{C^2 \lambda s^2} \frac{1}{K(0.5 - 0.333K)^2}. \quad (3.44)$$

The decision function D and, hence, also the volume of material decreases with the increasing values of C , λ , $K(0.5 - 0.333K)^2$ and $\frac{s^2}{(s - 1)^3}$. The hints for proportioning the section then are as follows:

- (1) Use fully stressed design ($C = 1$) wherever possible. In other cases, use the highest value of $C < 1$, as permitted by deflection of Formula (3.51).
- (2) Use webs with maximum possible slenderness λ , as permitted by web buckling.
- (3) The expression $K(0.5 - 0.333K)^2$ assumes its maximum for $K = 0.5$.
- (4) The value of $s = \frac{B}{b}$ for $s = 2, 3, 4, \dots$ should be as low as possible or b as great as possible. The limit is $s = 2$ or $b = \frac{B}{2}$. This is based on the arrangement without the side beams (Section 3.3.4). Similarly, this also holds true for the other arrangement. The value of $b < \frac{B}{2}$ is determined usually by practical considerations, e. g., by the length of available floor slabs.

3.5.2 Reduction in Constraints

The decision function D (Formula 3.44) has depended on four variables ($C, \lambda, b = \frac{B}{s}, K$). As the values of b and K have been determined ($K = 0.5, b$ as great as practical considerations permit) only two unknowns, C and λ , should be found to satisfy the constraints

$$G_1(I, \lambda) = G_1(C, \lambda) \geq 0 \quad \text{flexural strength} \quad (3.45)$$

$$G_2(I) = G_2(C) \geq 0 \quad \text{deflection} \quad (3.46)$$

$$G_3(I, \lambda) = G_3(C, \lambda) \geq 0 \quad \text{web buckling.} \quad (3.47)$$

Variables I and C are correlated through Formulas (3.14) and (3.35).

As three expressions (3.45) to (3.47) for only two unknowns, C and λ , are available, just two of them can be equalities.

	<u>Case (1)</u>	<u>Case (2)</u>	<u>Case (3)</u>	
G_1	=	>	=	0
G_2	>	=	=	0
G_3	=	=	>	0

It depends on the given data, which pair would be chosen. Instead of using a method of Mathematical Programming to solve the problem, one can employ also another procedure (analytical approach): Eliminate one variable, e. g., λ , and use the constraints to find the remaining variable C . Then, determine λ by back substitution.

Case (1)

If the equality sign is considered in Formula (3.47), (Formula 3.22),

$$\sigma_{cr} = C\sigma_{all} \quad (3.48)$$

then the constraint G_3 can be eliminated. From Formulas (3.26) and (3.35), the expression for the web slenderness follows:

$$\lambda = \left(\frac{C_4}{C\sigma_{all}} \right)^{0.5}. \quad (3.49)$$

The problem has been reduced to the determination of C . For a fully stressed design, with the equality sign in Formula (3.45),

$$C = 1.0. \quad (3.50)$$

Case (2)

$$C_{11} \leq C \leq 1.0 \quad (3.51)$$

as much as the constraint G_2 (Formula 3.46) for deflection allows.

The limit C_{11} follows from G_2 (with equality sign in Formula 3.46).

See Appendix C, Formula (C 23), where

$$C_{11} = \left(\frac{C_1 \frac{13}{18} C_4 \frac{1}{9}}{A \frac{5}{9} \sigma_{all}} \right) \left(\frac{0.354 E C_3}{C_2} \right)^{\frac{2}{3}} \left(\frac{pb}{0.00305} \right)^{\frac{1}{18}}. \quad (C 23)$$

Case (3)

This case, with equality signs for G_1 and G_2 and an inequality sign for G_3 , yields a constant C_{12} and hence I , where in Appendix D

$$C_{12} = \frac{2 C_2 \sigma_{all}}{C_1 C_3 E}, \quad (D 4)$$

$$I = \frac{C_1 pb A^3 C_{12}}{2 \sigma_{all}}. \quad (D 6)$$

For a chosen feasible value of λ , there is a corresponding value of σ_{cr} (Formula 3.23) such that $G_3 > 0$; and there is a corresponding value of volume since $A_t = f(I, \lambda)$ from Formula (3.12).

The result obtained (three cases) demonstrates that it is not always the fully stressed design which is the optimum, the fully stressed design being not applicable for other conditions.

3.5.3 Steps of Computation

The computation can now be summarized in the following steps:

Step 1 The spacing b should be as great as the design allows but less than or equal to $\frac{B}{2}$. The web coefficient $K = 0.5$.

Step 2 For the given data, calculate C where $C = \text{Min}(1.0, C_{11})$ from Formula (3.51). The three cases in Section 3.5.2 are:

Case (1)

If $C = 1.0$, flexural strength is critical. Go to Step 3 to calculate λ and I .

Case (2)

If $C = C_{11}$, deflection is critical. Go to Step 3.

Case (3)

If web buckling is not critical, go to Step 4.

Step 3 $C = 1.0$ or $C = C_{11}$: Calculate λ from Formula (3.49) and I from Formula (3.17) for Case (1) or from Formula (3.21) for Case (2).

Step 4 $C = C_{12}$: Calculate I from

$$I = \frac{C_1 p b A^3 C_{12}}{2\sigma_{\text{all}}} . \quad (\text{D } 6)$$

For a chosen feasible value of λ , calculate σ_{cr} (Formula 3.23), A_t (Formula 3.12), and volume (Formula 3.28).

This analytical approach is tested numerically by considering five examples, Exs (5.1) to (5.5), in Chapter V. One of these examples (Ex 5.3) is checked by the Code MSLP (Modified Sequence of Linear Programming Solutions Technique) as Ex (5.6).

CHAPTER IV

A GRID WITH BEAMS SPANNING IN TWO ORTHOGONAL DIRECTIONS--SIMPLE-CONNECTION (PROBLEM 2), RIGID-CONNECTION (PROBLEM 3)

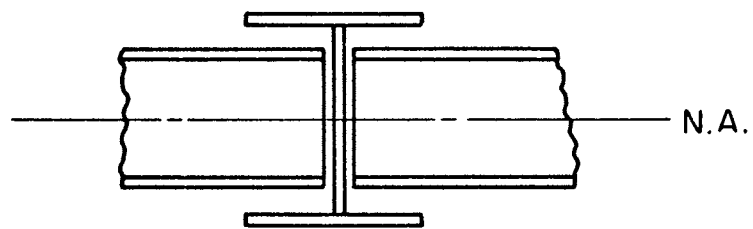
4.1 Introduction

The analysis of a grid requires more than the Theory of Beams as in Chapter III. Since the Theory of Plates (41) is adopted, the governing equation and its solution will lead to the formulation of a mathematical model.

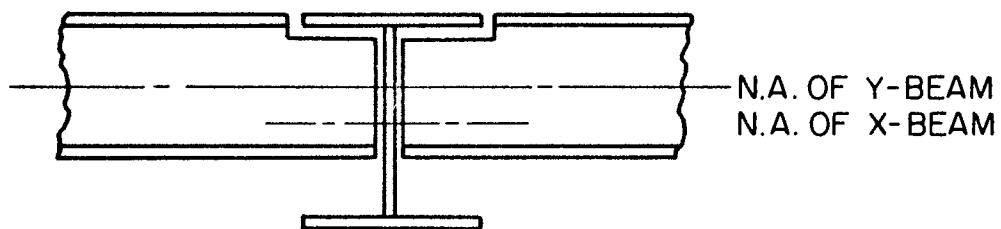
General expressions are derived to include torsional rigidities at the joints (Rigid-Connections). A simpler case is investigated first, namely the Simple-Connection. By Simple-Connection it is meant that girders are connected in such a way that the joint does not offer any torsional resistance. In such a case, hereafter referred to as PROBLEM 2, torsion in the expressions is excluded. On the other hand, the Rigid-Connection is supposed to provide torsional resistance and is hereafter referred to as PROBLEM 3.

With respect to the mutual position of the neutral axes of the intersecting beams, there are generally three types of arrangements at the joint, as shown in Figs 8(a) to (c) which are self-explanatory.

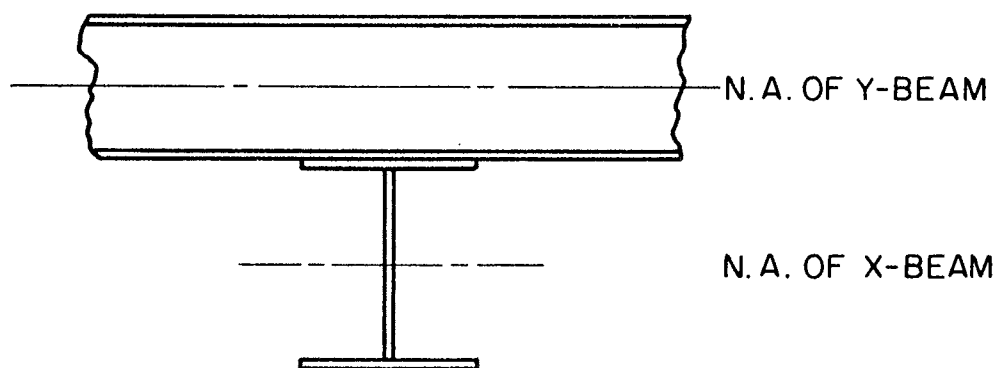
The Theory of Plates assumes the first type (Fig 8a) although it takes on the slight eccentricity in the second type (Fig 8b) without much error. As a matter of fact, most orthotropic-plate bridge decks are



(a)



(b)



(c)

Figure 8. (a), (b), and (c): Types 1, 2, and 3

built with the second type. The most drastic eccentricity is the third type (Fig 8c). However, it is an approximation good enough to warrant the application of the theory derived for the first type (Fig 8a) to all three types. It is important to distinguish the various arrangements when strength conditions (bending stresses) are applied. The second type is used throughout this study.

In general, the subscripts "x, y" denote the x- and y-direction, respectively. The parameters, with or without the subscript "x," used in Chapter III can be used here in Chapter IV for the x-direction and also for the y-direction with the subscripts changed from "x" to "y."

4.2 Geometry of the Grid

Let σ_{jt} = combined stress at a joint,
 σ_x = bending stress in x-beam, and
 σ_y = bending stress in y-beam.

Since the grid is assumed to be a continuous system, it will be conservative enough (more than safe) to consider σ_{jt} , σ_x , and σ_y at the center for this case as the strength criteria. These criteria will appear as constraints G(7), G(1), and G(2), respectively. In order to clarify this statement of "conservative enough," consider for a moment the grid as a discrete system. The four cases in Figs (9) to (12), which are self-explanatory, among in-between situations will occur in the process of Nonlinear Programming (NLP) being applied.

From a simple speculation, it is observed that the maximum of σ_{jt} , σ_x , and σ_y evaluated at the center (for this symmetric case, PROBLEM 3) is greater than any flexural stress evaluated at any critical points in all the four cases.

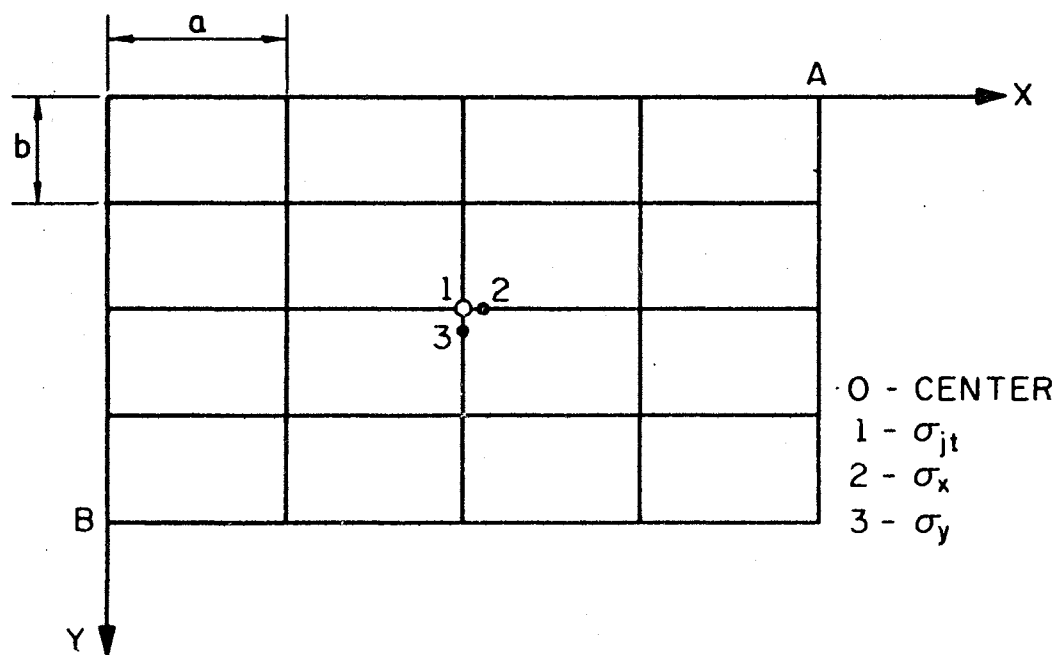


Figure 9. Case 1

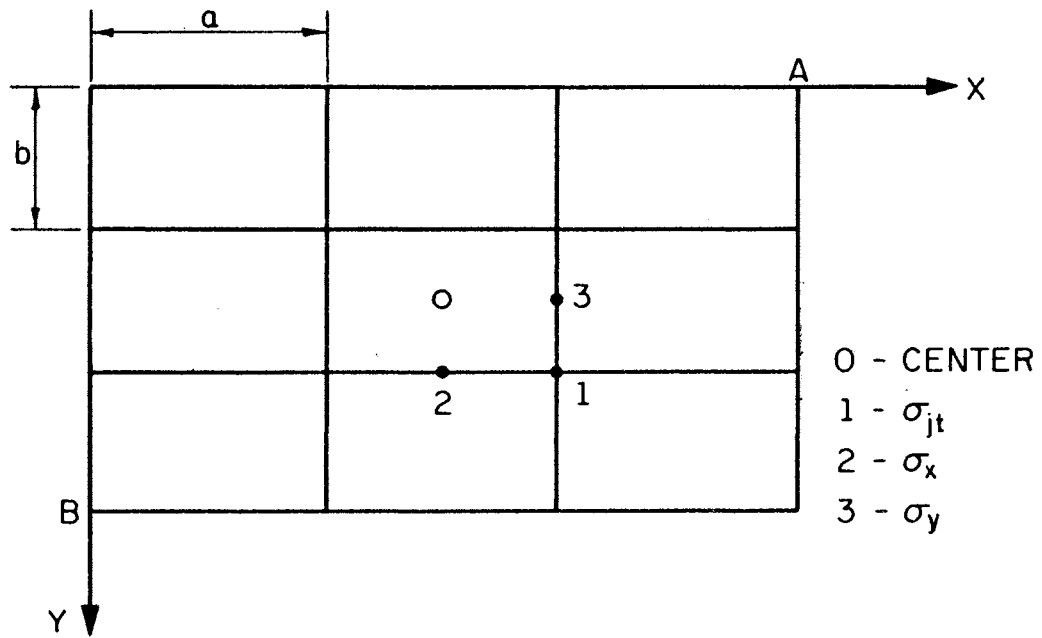


Figure 10. Case 2

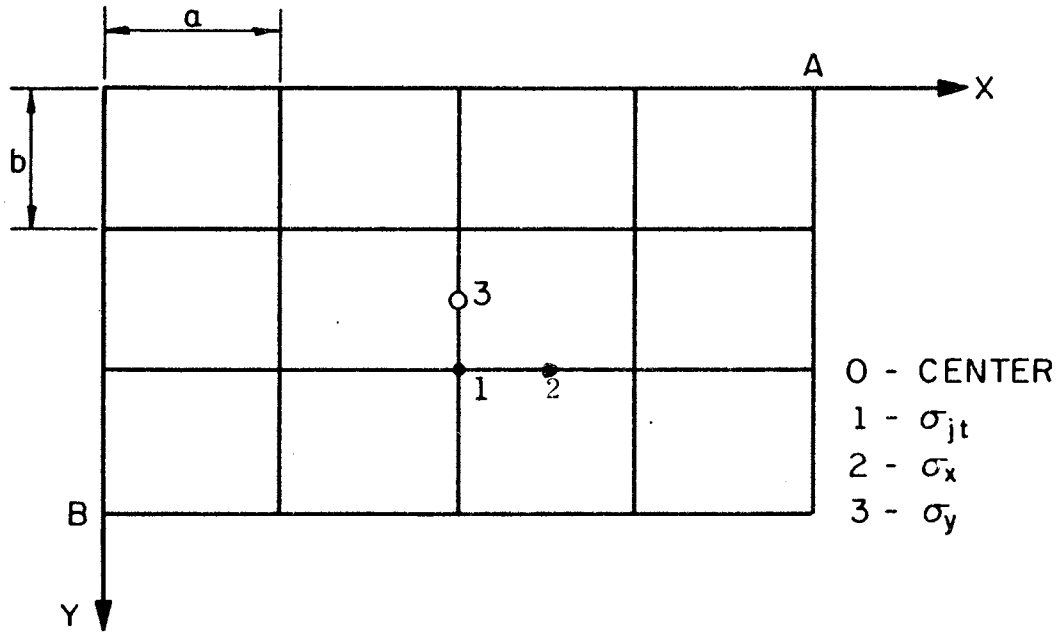


Figure 11. Case 3

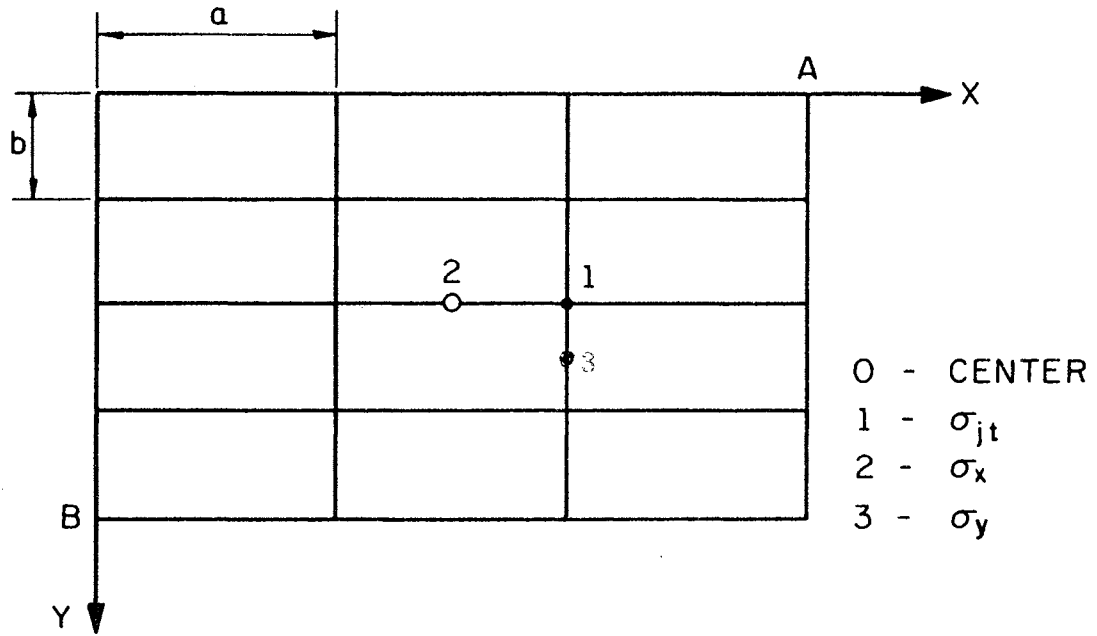


Figure 12, Case 4

As this conservative approach is safe as well as in accordance with the assumption of a continuous system, it will be adopted, although a rigorous analysis can be used if one chooses. A similar argument goes for the deflection and web buckling criteria. However, in the web buckling criterion, since web buckling unlikely occurs at the joint because of mutual reinforcement, the σ_{jt} will be omitted.

4.3 Governing Equation

From the analysis of an orthotropic plate (38), the bending moments per unit width are

$$M_x = -(D_x w,_{xx} + D_1 w,_{yy}) \quad (4.1)$$

$$M_y = -(D_y w,_{yy} + D_1 w,_{xx}) \quad (4.2)$$

and the torsional moment per unit width

$$M_{xy} = 2D_{xy} w,_{xy} = -M_{yx}. \quad (4.3)$$

The governing equation results in the following form:

$$D_x w,_{xxxx} + 2Hw,_{xxyy} + D_y w,_{yyyy} = q \quad (4.4)$$

where the subscript designates the direction and the comma its derivative. Let

$$(\quad),_x \text{ through } (\quad),_{xxxx} = \text{1st through 4th partial derivatives with respect to } x,$$

$$(\quad),_y \text{ through } (\quad),_{yyyy} = \text{1st through 4th partial derivatives with respect to } y,$$

$$(\quad),_{xy} \text{ and } (\quad),_{xxyy} = \text{2nd and 4th mixed partial derivatives with respect to } x \text{ and } y,$$

$$M_x = \text{bending moment per unit width in } x\text{-beam,}$$

$$M_y = \text{bending moment per unit width in } y\text{-beam,}$$

$$M_{xy} = \text{torsional moment per unit width in } x\text{-beam,}$$

$$M_{yx} = \text{torsional moment per unit width in } y\text{-beam,}$$

- $q(x, y)$ = loading function per unit area,
 w = deflection,
 D_x, D_y = unit flexural rigidities,
 H, D_1, D_{xy} = parameters associated with unit torsional rigidities.

4.4 Deflections and Moments

The rectangular grid (Fig 13), with sides A by B, is under consideration. If

$F_x = EI_x$ = flexural rigidity of each x-beam, and

$F_y = EI_y$ = flexural rigidity of each y-beam,

then their unit flexural rigidities are

$$D_x = \frac{F_x}{b} \quad (4.5)$$

$$D_y = \frac{F_y}{a} \quad (4.6)$$

where

a = spacing of beams oriented in the y-direction ("y-beams"),

b = spacing of beams oriented in the x-direction ("x-beams").

The parameter H is defined as

$$2H = \frac{C_x}{b} + \frac{C_y}{a}. \quad (4.7)$$

C_x and C_y will be explained in Section 4.5.2.

The Navier solution (38) for the plate, uniformly loaded and simply supported on all sides (Fig 13), is the deflection

$$w(x, y) = \sum_{m=1,3,\dots}^{\infty} \sum_{n=1,3,\dots}^{\infty} A_{mn} \sin\left(\frac{m\pi x}{A}\right) \sin\left(\frac{n\pi y}{B}\right) \quad (4.8)$$

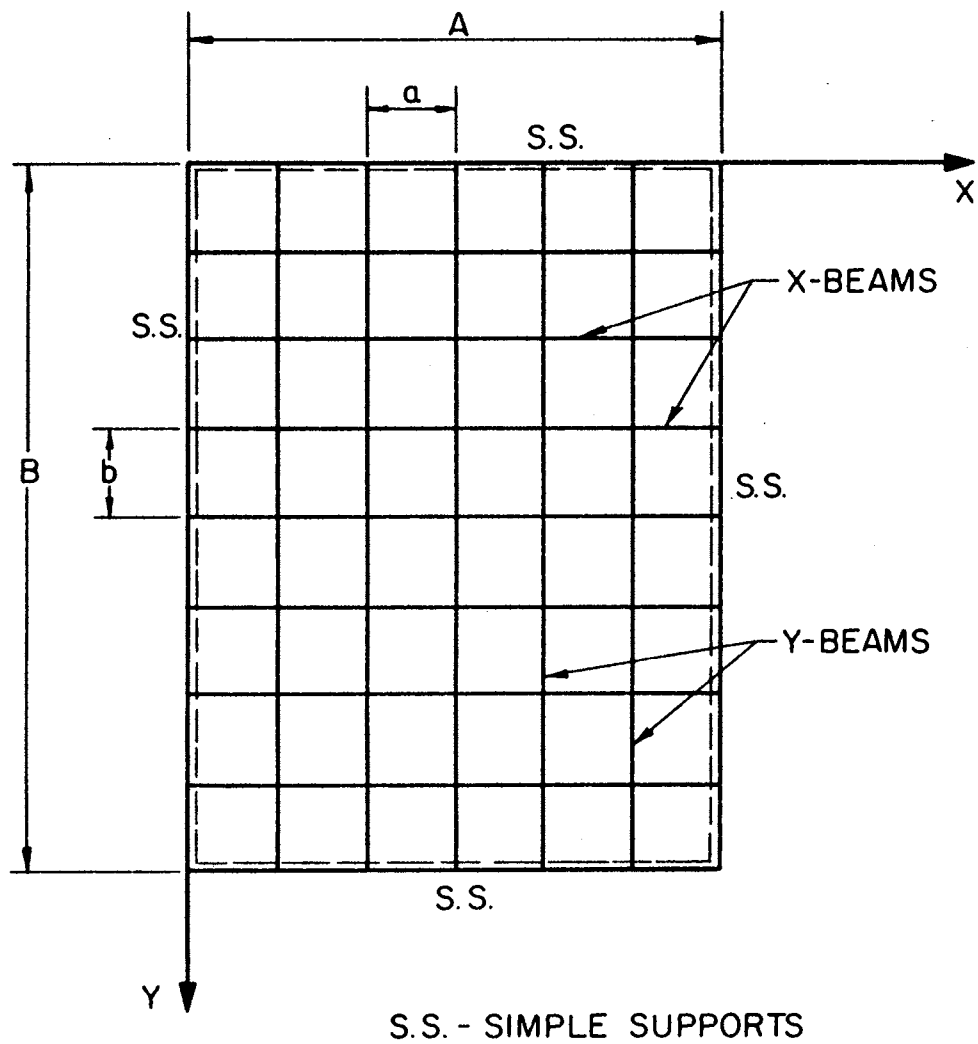


Figure 13. A Grid Simply-Supported on All Four Sides

where A_{mn} = parameters associated with the assumed solution of deflection, and

$$A_{mn} = \frac{16p}{\pi^6} \frac{1}{mn \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \quad (4.9)$$

or

$$w = \frac{16p}{\pi^6} \sum_{m=1,3,\dots}^{\infty} \sum_{n=1,3,\dots}^{\infty} \frac{\sin\left(\frac{m\pi x}{A}\right) \sin\left(\frac{n\pi y}{B}\right)}{mn \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \quad (4.10)$$

Derivatives needed for calculations of moments are:

$$w_{,xx} = \frac{16p}{\pi^6} \sum_{m=1,3,\dots}^{\infty} \sum_{n=1,3,\dots}^{\infty} \frac{-\left(\frac{m\pi}{A}\right)^2 \sin\left(\frac{m\pi x}{A}\right) \sin\left(\frac{n\pi y}{B}\right)}{mn \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \quad (4.11)$$

$$w_{,yy} = \frac{16p}{\pi^6} \sum_{m=1,3,\dots}^{\infty} \sum_{n=1,3,\dots}^{\infty} \frac{-\left(\frac{n\pi}{B}\right)^2 \sin\left(\frac{m\pi x}{A}\right) \sin\left(\frac{n\pi y}{B}\right)}{mn \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \quad (4.12)$$

$$w_{,xy} = \frac{16p}{\pi^6} \sum_{m=1,3,\dots}^{\infty} \sum_{n=1,3,\dots}^{\infty} \frac{+\left(\frac{m\pi}{A}\right) \left(\frac{n\pi}{B}\right) \cos\left(\frac{m\pi x}{A}\right) \cos\left(\frac{n\pi y}{B}\right)}{mn \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \quad (4.13)$$

As in Chapter III, the same three criteria are considered here but twofold, once for x-beams and once for y-beams. With regard to moments, Timoshenko (38) suggested for a fine-meshed grid to assume a parabolic moment variation among three adjacent x-beams (Fig 14)

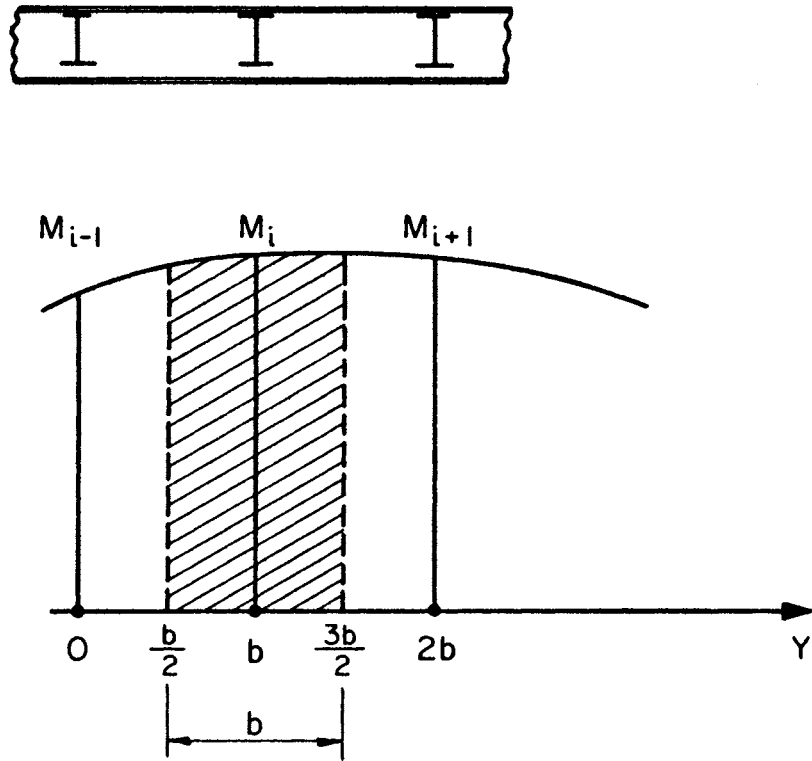


Figure 14. Locations of Three Typical x-Beams

with three ordinates M_{i-1} , M_i , and M_{i+1} at $y = 0, b, 2b$, respectively.

A similar assumption can be made for the x-direction. The total moment, covering a strip of width b , being centered at M_i , is then

$$M_{\text{tot}} = \frac{b}{24} (M_{i-1} + 22M_i + M_{i+1}) \quad (4.14)$$

$$M_i = -D_x (w,_{xx})_i = \frac{-F_x}{b} (w,_{xx})_i \quad (4.15)$$

$$M_x = \frac{-F_x}{24} \left[(w,_{xx})_{i-1} + 22(w,_{xx})_i + (w,_{xx})_{i+1} \right]. \quad (4.16)$$

Similarly,

$$M_y = \frac{-F_y}{24} \left[(w,_{yy})_{i-1} + 22(w,_{yy})_i + (w,_{yy})_{i+1} \right] \quad (4.17)$$

$$M_{xy} = \frac{C_x}{24} \left[(w,_{xy})_{i-1} + 22(w,_{xy})_i + (w,_{xy})_{i+1} \right] \quad (4.18)$$

$$M_{yx} = \frac{-C_y}{24} \left[(w,_{xy})_{i-1} + 22(w,_{xy})_i + (w,_{xy})_{i+1} \right]. \quad (4.19)$$

At the center $(\frac{A}{2}, \frac{B}{2})$, Formula (4, 16) becomes

$$\begin{aligned} \max M_x &= \frac{+F_x}{24} \frac{16p}{\pi^6} \sum_{m=1,3,\dots}^{\infty} \sum_{n=1,3,\dots}^{\infty} \\ &\frac{(\frac{m\pi}{A})^2}{mn \left[(\frac{m}{A})^4 D_x + (\frac{mn}{AB})^2 2H + (\frac{n}{B})^4 D_y \right]} \\ &\sin(\frac{m\pi}{2}) \left[\sin \frac{n\pi}{B} (\frac{B}{2} - b) + 22 \sin(\frac{n\pi}{2}) \right. \\ &\left. + \sin \frac{n\pi}{B} (\frac{B}{2} + b) \right]. \end{aligned} \quad (4.20)$$

Similarly,

$$\max M_y = \frac{+F_y}{24} \frac{16p}{\pi} \sum_{m=1,3,\dots}^{\infty} \sum_{n=1,3,\dots}^{\infty} \frac{\left(\frac{n\pi}{B}\right)^2}{mn \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \sin\left(\frac{n\pi}{2}\right) \left[\sin\left(\frac{m\pi}{A}\right) \left(\frac{A}{2} - a\right) + 22 \sin\left(\frac{m\pi}{2}\right) + \sin\left(\frac{m\pi}{A}\right) \left(\frac{A}{2} + a\right) \right]. \quad (4.21)$$

Torsional moment M_{xy} is usually negligible or equal to zero at the sections where bending moments assume their maxima.

4.5 Design Criteria

4.5.1 First Criterion (Bending)

There exists a spacial state of stress in the gridwork; stresses in the vertical direction being negligible, the two-dimensional state of stress is observed. This biaxial action calls for the selection of a failure theory. The theory of maximum distortion strain energy has been chosen, leading to the strength condition at a joint. Let

σ_{jt} = combined stress at a joint, and

$$\sigma_{jt} = (\sigma_1^2 - \sigma_1\sigma_2 + \sigma_2^2)^{0.5} \quad (4.22)$$

where

$$\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \left[\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \sigma_{xy}^2 \right]^{0.5} \quad (4.23)$$

are principal stresses at a joint. The maximum bending stresses used in the constraints G(1) and G(2) as Formula (3.15) are

$$\sigma_x = \frac{M_x}{S_x}, \text{ and} \quad (4.24)$$

$$\sigma_y = \frac{M_y}{S_y}, \quad (4.25)$$

where S_x = section modulus of the x-beam, and
 S_y = section modulus of the y-beam.

The maximum shearing stress is

$$\sigma_{xy} = \frac{M_{xy}}{J} \delta. \quad (4.26)$$

The torsional moment of inertia J will be discussed in Section 4.5.2. The torsional moments M_{xy} , which are absent in PROBLEM 2 due to zero torsional rigidities, have now come into play.

Recalling the joint of Type 2 (Fig 8b), it is observed that the maximum combined stress occurs at the top fiber as shown in Fig 15. The constraint at the joint becomes

$$G(7) = \sigma_{all} - (\sigma_1^2 - \sigma_1\sigma_2 + \sigma_2^2)^{0.5} \geq 0. \quad (4.27)$$

4.5.2 Second Criterion (Deflection)

Deflection is expressed by Formula (4.10), where the unit torsional rigidities in Formula (4.7) are

$$C_x = GJ_x, \text{ and} \quad (4.28)$$

$$C_y = GJ_y \quad (4.29)$$

with G = shear modulus, and

J_x, J_y = torsional moments of inertia for the x- and y-beams, respectively.

Since an I-section is assumed which is made up of narrow rectangular elements, the torsional moment of inertia is

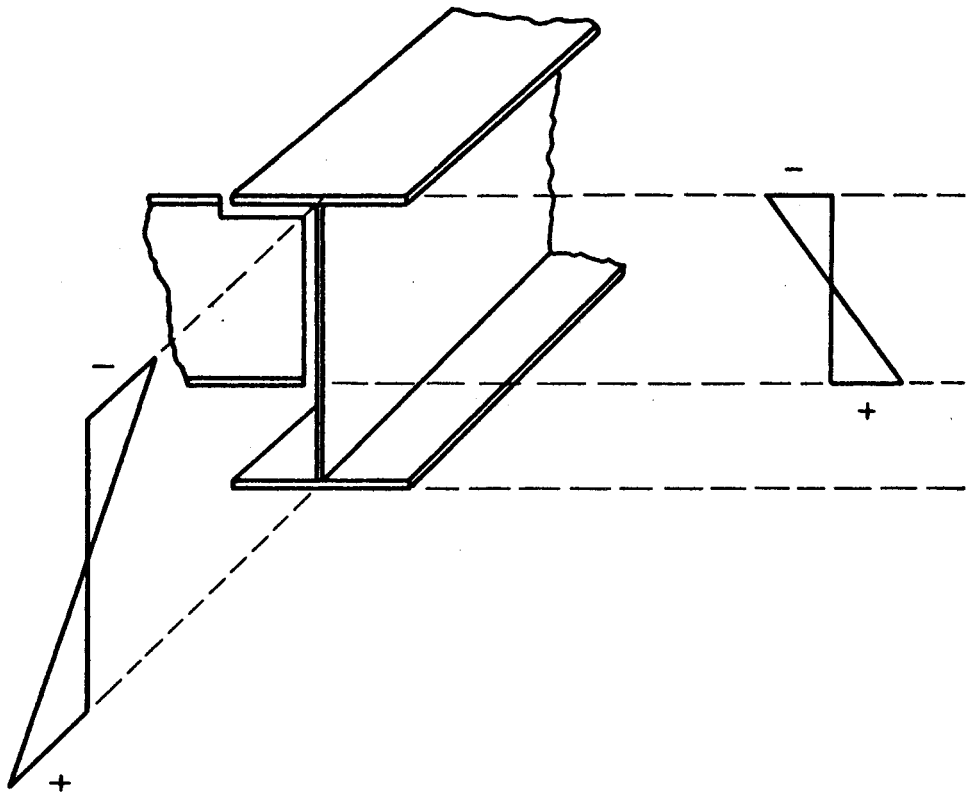


Figure 15. Bending Stresses at a Joint

$$J = \frac{1}{3} \sum_{i=1}^3 b_o t^3 \quad (4.30)$$

or

$$J = \frac{1}{3} (2A_f t_f^2 + A_w \delta^2) \quad (4.31)$$

where b_o = depth (width) of the narrow rectangular element,

t = thickness of the narrow rectangular element,

A_f = flange cross-sectional area,

A_w = web cross-sectional area,

t_f = flange thickness,

δ = web thickness.

Letting

$$B_1 = \frac{t_f}{\delta} \quad (4.32)$$

$$B_2 = \frac{A_f}{A_w}, \quad (4.33)$$

Formula (4.31) becomes

$$J = \frac{1}{3} (2B_2 B_1^2 + 1) (A_w \delta^2). \quad (4.34)$$

Substituting Formulas (3.6) and (3.7) into Formula (4.34), for the respective x- and y-directions, Formula (4.34) reduces to

$$J_x = \frac{1}{3} (2B_2 B_1^2 + 1) (\lambda_x \delta_x^4) \quad (4.35)$$

and

$$J_y = \frac{1}{3} (2B_2 B_1^2 + 1) (\lambda_y \delta_y^4), \quad (4.36)$$

where δ_x = web thickness of the x-beam, and

δ_y = web thickness of the y-beam.

Efforts were made to suppress the addition of the two variables

B_1 and B_2 . The parameter B_2 is dealt with as follows:

$$A_t = 2A_f + A_w . \quad (4.37)$$

Recalling Formula (3.3) and $K_w = K = 0.5$ (the optimal value for bending when bending prevails), where

$$A_w = K_w A_t, \quad (4.38)$$

Formula (4.37) becomes

$$A_f = 0.5A_w \quad (4.39)$$

or by Formula (4.33)

$$B_2 = \frac{A_f}{A_w} = 0.5 \quad (4.40)$$

when torsion is negligible. When torsion is taken into consideration, B_2 may vary within a narrow range, similarly as K_w does.

Treating B_2 as K_w , it is desired to estimate the range of B_1 .

Rewriting Formula (4.34),

$$J = \frac{\lambda \delta^4}{3} \left(1 + 2 \frac{A_f}{A_w} \frac{t_f^2}{\delta^2} \right) \quad (4.41)$$

and noting

$$K_w = \frac{A_w}{A_t} \quad (4.42)$$

the two ratios can be expressed as

$$\frac{2A_f}{A_w} = \frac{1 - K_w}{K_w}, \quad (4.43)$$

$$\frac{t_f}{\delta} = \frac{1 - K_w}{2K_w} \frac{h}{b_f} \quad (4.44)$$

where b_f = flange width. Substituting into Formula (4.41),

$$J = \frac{\lambda \delta^4}{3} \left[1 + \frac{(1 - K_w)^3}{4K_w^3} \frac{h^2}{b_f^2} \right]. \quad (4.45)$$

A practical range of $\frac{h}{b_f}$ is between 2 and 5 for shallow and deep girders, respectively, although the ratio of 3 or 4 is generally preferred. Assuming constant cross-sectional area, a shallow girder will have smaller K_w , say $K_w = 0.4$, whereas a deep girder will have $K_w = 0.6$. Substituting these values of K_w , the factor in the brackets of J in Formula (4.45) varies between 2.85 and 4.38; and B_1 from Formulas (4.44) and (4.32) varies between 1.5 and 1.67. These small ranges of B_1 and J, and the fact that J is usually small compared to I (moment of inertia in bending) warrant the use of B_1 as a constant rather than as a variable. From Formulas (4.43) and (4.33), B_2 is found to be between 0.75 and 0.33 for K_w between 0.4 and 0.6, respectively.

Taking the averages of the narrow ranges of B_1 and B_2 , consider

$$B_1 = 1.59, \text{ instead of } B_1 = (1.5--1.67), \text{ and}$$

$$B_2 = 0.54, \text{ instead of } B_2 = (0.75--0.33).$$

By the speculations above, the necessity of introducing two new variables has been excluded. Because torsional resistance is of secondary importance, this simplification of the problem cannot affect the results. However, if desired, programs can be run for the limiting values of B_1 and B_2 to show the insignificant difference in results.

With B_1 and B_2 being set constant, another advantage is that Formulas (4.37), (3.7), and (3.12) lead to the elimination of the two variables δ_x and δ_y because δ_x can now be expressed in terms of I_x and λ_x , and similarly δ_y in terms of I_y and λ_y . PROBLEM 3 has become a problem of at least six variables.

4.5.3 Third Criterion (Web Buckling)

As the beam-web is stiffened by the crossing beam, web buckling will likely occur in the span rather than at the joint. Since the difference between PROBLEM 3 and PROBLEM 2 is only in the joint condition, the web buckling criteria are the same for both problems. In Formula (3.26), λ , M , and S are changed to λ_x , M_x , and S_x for the x-beam and to λ_y , M_y , and S_y for the y-beam.

4.5.4 Objective function

The objective function is similar to Formula (3.28) except that now both for PROBLEMS 2 and 3 it has two terms, one for x-beams and the other for y-beams.

$$V = 3.46\left(\frac{B}{b} - 1\right) \frac{AI_x^{0.5}}{\lambda_x^{0.5}} + 3.46\left(\frac{A}{a} - 1\right) \frac{BI_y^{0.5}}{\lambda_y^{0.5}}. \quad (4.46)$$

4.6 Mathematical Model

The necessity of an upper bound on the spacing, as discussed in Chapter III, again leads to two constraints for the two upper bounds on x-spacings, a , and y-spacings, b . The other upper bounds and lower bounds could be included. For example, the upper bounds on a and b can be replaced by the upper bounds on I_x and I_y . It should be noted that the bounds are placed here in general as discussed in Section 3.4. They are subject to removal if they are not needed physically or for the sake of programming.

PROBLEM 2 bears the form similar to PROBLEM 3, with the exclusion of the torsional rigidity expressed by the parameter H from

the formulas where H appears. Instead of repeating the pertinent formulas, with V from Formula (4.46) and σ_x , σ_y , w_x , w_y , σ_{cr} , and σ_{jt} given by Formulas (4.24), (4.25), (4.10), (4.10), (3.23), and (4.22) respectively, PROBLEM 3 is formulated in a simplified form:

PROBLEM 3

Minimize $V(x_j)$, $j = 1, \dots, 6$,

subject to the constraints

$$G(1) = \sigma_{all} - \sigma_x(x_j) \geq 0 \quad (4.47)$$

$$G(2) = \sigma_{all} - \sigma_y(x_j) \geq 0 \quad (4.48)$$

$$G(3) = w_{all} - w_x(x_j) \geq 0 \quad (4.49)$$

$$G(4) = w_{all} - w_y(x_j) \geq 0 \quad (4.50)$$

$$G(5) = \sigma_{cr}(x_3) - \sigma_x(x_j) \geq 0 \quad (4.51)$$

$$G(6) = \sigma_{cr}(x_6) - \sigma_y(x_j) \geq 0 \quad (4.52)$$

$$G(7) = \sigma_{all} - \sigma_{jt}(x_j) \geq 0 \quad (4.53)$$

$$BL_j \leq x_j \leq UB_j \quad (4.54)$$

where

$j = 1, \dots, 6$ in Formulas (4.47) to (4.54),

$x_j = (a, I_x, \lambda_x, b, I_y, \lambda_y)$,

BL_j = lower bounds of x_j , and

UB_j = upper bounds of x_j .

CHAPTER V

NUMERICAL EXAMPLES

5.1 Problem 1

Six numerical examples were chosen to illustrate the procedure of design as discussed in Chapter III. In Ex (5.1) the theoretical upper bound of spacing b was used. Since it was a fully stressed design, the next example (Ex 5.2) made use of a higher allowable stress σ_{all} and lower allowable deflection coefficient C_3 to force the problem to be one that was not fully stressed design (or Case 2 in Section 3.5.2). The immediate question was that of a practical upper bound on b , (affected, e.g., by the available width of floor slabs), which was answered by Exs (5.3) and (5.4) similar to Exs (5.1) and (5.2), respectively, with the exception of the upper bound of b . Similarly, certain data in Ex (5.5) were chosen to make up a situation like Case (3) in Section 3.5.2. These five examples were done by the procedure described in Section 3.5, and their data and solutions are shown in Tables I and II, respectively. Table III shows three constraint values for Exs (5.1) to (5.5) which illustrate the three cases in Section 3.5.2. Finally, Ex (5.6) was the same as Ex (5.3) but was done by the Computer Code MSLP (Modified Sequence of Linear Programming Solutions) instead; its result is shown in Table IV. PROBLEM 1, with the data given in Ex (5.6), was found to be a nonconvex programming problem as defined in Section 2.8, using the definition of Formula (2.5).

TABLE I
DATA FOR EXAMPLE (5.1)

A	720	in
B	720	in
b	$\frac{B}{2} = 360$	in
σ_{all}	22	ksi
E	30,000	ksi
ν	0.3	
p	0.001	ksi
K_1	23.9	
C_1	$\frac{1}{8}$	
C_2	$\frac{5}{384}$	
C_3	$\frac{1}{360}$	

Example (5.1)

Simple supports,

$p = \text{UDL} = \text{uniformly distributed load,}$

Symmetrical I-section welded plate girders, and

Data given in Table I.

Example (5.2)

The data are the same as in Ex (5.1) except that

$$b = \frac{B}{2} = 360 \text{ in. ,}$$

$$\sigma_{\text{all}} = 42 \text{ ksi, and}$$

$$C_3 = \frac{1}{1000} \cdot$$

Example (5.3)

The data are the same as in Ex (5.1) except that

$$b = \frac{B}{6} = 120 \text{ in. ,}$$

$$\sigma_{\text{all}} = 22 \text{ ksi, and}$$

$$C_3 = \frac{1}{360} \cdot$$

Example (5.4)

The data are the same as in Ex (5.1) except that

$$b = \frac{B}{6} = 120 \text{ in. ,}$$

$$\sigma_{\text{all}} = 42 \text{ ksi, and}$$

$$C_3 = \frac{1}{1000} \cdot$$

Example (5.5)

The data are the same as in Ex (5.1) except that

$$b = \frac{B}{6} = 120 \text{ in. ,}$$

$$\sigma_{\text{all}} = 42 \text{ ksi, and}$$

$$C_3 = \frac{1}{360} \cdot$$

TABLE II
SOLUTIONS OF EXAMPLES (5.1) TO (5.5)

	(5.1)	(5.2)	(5.3)	(5.4)	(5.5)
CASE	(1)	(2)	(1)	(2)	(3)
C_{11}	1.39	0.369	1.09	0.289	---
C	1.0	0.369	1.0	0.289	---
h (in)	---	---	---	---	75.6
C_{14}	---	---	---	---	0.105
σ (ksi)	22.0 ^(A)	15.6	22.0 ^(A)	12.2	42.0 ^(A)
b (in)	360.0	360.0	120.0	120.0	120.0
λ	170.0	205.0	170.0	232.0	100.0
I (in ⁴)	34,600.0	58,300.0	8,004.0	19,400.0	6,998.0
Min V (in ³)	35,345.0	42,018.0	84,976.0	114,120.0	104,200.0

(A) fully stressed

TABLE III
THREE CONSTRAINT VALUES OF
EXAMPLES (5.1) TO (5.5)

	(5.1)	(5.2)	(5.3)	(5.4)	(5.5)
CASE	(1)	(2)	(1)	(2)	(3)
σ_{all}	22.0	42.0	22.0	42.0	42.0
σ	22.0 ^(A)	15.6	22.0 ^(A)	12.2	42.0 ^(A)
w_{all}	2.0	0.72	2.0	0.72	2.0
w	1.99	0.719	2.0	0.72	2.0
σ_{cr}	22.0	15.5	22.0	12.1	65.1
σ	22.0	15.6	22.0	12.2	42.0
$G_1 \geq 0$	0 (=)	26.4 (>)	0 (=)	29.8 (>)	0 (=)
$G_2 \geq 0$	0.01 (>)	0 (=)	0 (=)	0 (=)	0 (=)
$G_3 \geq 0$	0 (=)	0 (=)	0 (=)	0 (=)	23.1 (>)

(A) fully stressed

Example (5.6)

The data are the same as in Ex (5.3) and the solution is shown in Table IV. In addition to the data given in Table I, more data pertinent to this example are:

$$\begin{aligned}
 MN &= \text{No. of variables} = 3, \\
 NNL &= \text{No. of nonlinear constraints} = 3, \\
 LIN &= \text{No. of linear constraints} = 6, \\
 X(1) &= b \text{ (in)}, \\
 X(2) &= I_x \text{ (in}^4\text{)}, \\
 X(3) &= \lambda_x, \\
 BL(1) &= 24 \text{ (in)}, \\
 BL(2) &= 11.3 \text{ (in}^4\text{)}, \\
 BL(3) &= 100, \\
 UB(1) &= 120 \text{ (in)}, \\
 UB(2) &= 349,000 \text{ (in}^4\text{)}, \text{ and} \\
 UB(3) &= 350.
 \end{aligned}$$

5.2 Problem 2

In Chapter IV general expressions were derived for PROBLEM 3, with the mathematical model given in Section (4.6). It was realized that PROBLEM 2 was actually PROBLEM 3 with the exclusion of the parameter H. The result of PROBLEM 2 as Ex (5.7) is shown in Table V. PROBLEM 2 was found to be a nonconvex programming problem, using the definition of Formula (2.5).

From the last column of Table V, the values of a and b are 106.5 inches, indicating the number of spacings, e. g., $\frac{A}{a}$, a non-integer. The two adjacent integers are 6 and 7, which top the second and fourth

TABLE IV
SOLUTION OF EXAMPLE (5.6), (PROBLEM 1)

	INITIAL POINT	MSLP FINAL POINT
X(1) = b (in)	103	120
X(2) = I_x (in ⁴)	9,000	8,022
X(3) = λ_x	100	172
Min V (in ³)	141,600	85,070
		<u>SOLUTION</u>
IBM 360/65		
Execution time	3.98 seconds	
Total time	20.82 seconds	

columns of Table VI. The last column of Table V (solution) is inserted in Table VI for comparison. The last line of Table VI shows whether the three prospective solutions satisfy all the constraints or not. As the solution point in the last column of Table VI satisfies all the constraints, it is the final design.

Example (5.7)

Simple supports,

p = UDL = uniformly distributed load, and

Symmetrical I-section welded plate girders.

In addition to the data given in Table I, more data pertinent to this example are:

MN = No. of variables = 6,

NNL = No. of nonlinear constraints = 7,

LIN = No. of linear constraints = 12,

X(1) = a (in),

X(2) = I_x (in⁴),

X(3) = λ_x ,

X(4) = b (in),

X(5) = I_y (in⁴),

X(6) = λ_y ,

BL(1), (4) = 24 (in),

BL(2), (5) = 11.3 (in⁴),

BL(3), (6) = 100,

UB(1), (4) = 120 (in),

UB(2), (5) = 349,000 (in⁴), and

UB(3), (6) = 350.

TABLE V
SOLUTION OF EXAMPLE (5.7), (PROBLEM 2)

	Initial point	Intermediate point	RGM Final point
X(1) = a (in)	103	120	106.5
X(2) = $I_x(\text{in}^4)$	9,000	8,999	9,000
X(3) = λ_x	100	108	172
X(4) = b (in)	103	120	106.5
X(5) = $I_y(\text{in}^4)$	9,000	8,999	9,000
X(6) = λ_y	100	108	172
Min V (in^3)	283,100	227,700	207,600

SOLUTION

IBM 360/65

Total time 14 min, 34 sec.

TABLE VI
FINAL DESIGN OF EXAMPLE (5.7), (PROBLEM 2)

		RGM	
		Final point	
$s = \frac{A}{a} = \frac{B}{b}$	6	non-integer	7
X(1) = a (in)	120	106.5	103
X(2) = $I_x(\text{in}^4)$	9,000	9,000	9,000
X(3) = λ_x	172	172	172
X(4) = b (in)	120	106.5	103
X(5) = $I_y(\text{in}^4)$	9,000	9,000	9,000
X(6) = λ_y	172	172	172
Min V (in ³)	180,200	207,600	215,900
All constraints satisfied	NO	YES	YES

FINAL DESIGN

5.3 Problem 3

The mathematical model of PROBLEM 3 given in Section 4.6 is tested here numerically. PROBLEM 3 yielded the same solution as PROBLEM 2 (Table V); therefore, the final design of PROBLEM 3 is identical to that of PROBLEM 2 (Table VI).

PROBLEM 3 was found to be a nonconvex programming problem, using the definition of Formula (2.5). The RGM Code (Ricochet Gradient Method) took 14 min. 21 sec. in computer time.

Example (5.8)

Simple supports,

p = UDL = uniformly distributed load, and

Symmetrical I-section welded plate girders.

In addition to the data given in Example (5.7), more data pertinent to this example are:

$$B_1 = 1.59,$$

$$B_2 = 0.54, \text{ and}$$

$$G = 12,000 \text{ ksi.}$$

5.4 Discussion of Numerical Results

PROBLEM 1 was solved by the analytical approach (Section 3.5) and also by using the Computer Code MSLP (Modified Sequence of Linear Programming Solutions). Exs (5.1) to (5.5) numerically verified the three cases in Section 3.5.2. Upper bounds are linear constraints. When a design variable reaches its upper bound in the solution of the problem, it steps on its linear constraint, outside which the solution is infeasible. It can be said that the variable has reached its maximum possible value allowed by its design upper bound or design

linear constraint. The same can be said about a nonlinear constraint; the variable has reached its maximum possible value allowed by its design criterion or design nonlinear constraint. With these implications the spacing b was set at its maximum value allowed by its linear constraint (upper bound) according to Section 3.5.1 and the web slenderness λ reached its maximum value allowed by its design criterion or nonlinear constraint. In Ex (5.5), λ was preselected and had nothing to do with bounds.

Ex (5.6) was used to check on the validity of the Computer Code MSLP and also to compare with one of the previous examples, namely Ex (5.3). The results of these two examples came to a good agreement, as seen from Tables II and IV. Both b and λ in Ex (5.6) reached their maximum values as described above. The execution time for Ex (5.3) was 3.98 seconds (total time 20.82 seconds) on an IBM 360/65 model. Certainly the preparation for a Mathematical Programming problem is shorter than that for the analytical approach. A problem of the size approximate to the size of PROBLEM 1 needed about 2 minutes 17 seconds using the RGM Code (Ricochet Gradient Method). This is no surprise since the gradient method is known for its slow convergence.

PROBLEM 2 revealed some interesting results (Table V). The web slenderness λ reached its maximum value as allowed by the web buckling constraint, in spite of the notion speculated in Section 3.5.1 that the spacing of beams was expected to attain its upper bound. As a matter of fact, at some intermediate point (Table V) the spacing did reach its upper bound with the web slenderness being still low. However, at this intermediate point, the volume was still going on decreasing.

From the viewpoint of Nonlinear Programming, the final point (Table V) is the local minimum. On the other hand, the designer may take the intermediate point as an alternate design if he prefers to use a larger spacing at the expense of about 10 percent bigger volume for this case. It was noted that the solution did not give $s = \frac{B}{b}$ as an integer. Comparing the two points for the two adjacent integers, the point at which all the constraints are satisfied will be the final design.

PROBLEM 3 yielded a solution identical to that of PROBLEM 2. In Formulas (4.10), (4.20), and (4.21), PROBLEM 3 and PROBLEM 2 differ in the parameter H, being zero for the latter. A typical section shows that H is very small compared to D_x or D_y . As H is comparatively negligible, PROBLEM 3 for this case can be considered as practically the same as PROBLEM 2.

All three problems were found to be nonconvex problems. Approximation by linearization in PROBLEM 1 was successful, making it possible for the MSLP Code (Modified Sequence of Linear Programming Solutions) to work. However, as the degree of nonconvexity increased in PROBLEMS 2 and 3, in addition to the complexity, the RGM Code (Ricochet Gradient Method) took more computer time and the MSLP Code did not work. Since the MSLP Code does not take too much time to run, it is recommended that it should be tried first and if it fails then the RGM Code is used.

CHAPTER VI
BOUNDARY CONDITIONS, LOADING CONDITIONS,
AND BOUNDARY DIMENSIONS

6.1 Introduction

Boundary conditions and loading conditions belong to Step 1 in Chapter II, Section 2.7.

So far the only boundary condition imposed on the grid has been of simple supports. Whereas any other types of boundary condition can be dealt with by the Theory of Plates, at least approximately, two of them are probably most frequently used in practice, namely the simple supports and the clamped (or fixed) supports (Fig 16). The Navier's Method is primarily for a simply supported plate. However, the Timoshenko-Ritz Variational Method, in some features similar to the Navier's Method, can be applied to the clamped supports. By this means, as a result of the use of other functions (e. g. , Formulas 6.8 to 6.12) than sines in the series used previously, only the set of parameters A_{mn} which appears in the solution will change. The immediate benefit is that the same formulation can be used with the exception of this modification.

6.2 Timoshenko-Ritz Method

The principle used here is that the total potential energy is stationary,

$$\delta(U - V_w) = 0 \quad (6.1)$$

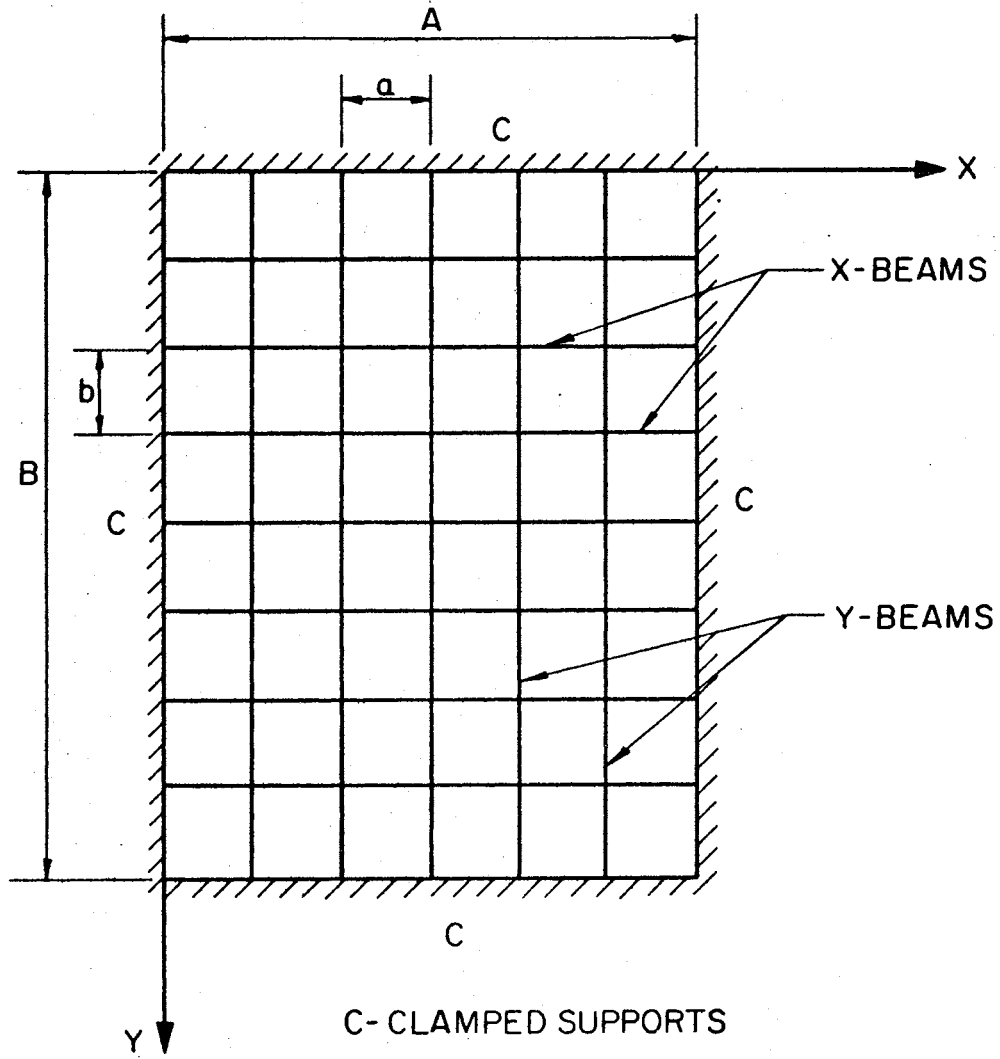


Figure 16. A Grid Clamped on All Four Sides

where U = strain energy of bending, and

V_w = virtual work.

For an orthotropic plate,

$$U = \frac{1}{2} \int_0^A \int_0^B (D_x w_{,xx}^2 + 2H w_{,xx} w_{,yy} + D_y w_{,yy}^2) dx dy \quad (6.2)$$

and

$$V_w = \int_0^A \int_0^B q(x, y) w dx dy \quad (6.3)$$

where $q(x, y)$ = loading function per unit area.

For uniform loading, $q(x, y) = p$. The assumed solution

$$w(x, y) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} w(x) w(y) \quad (6.4)$$

must satisfy at least the geometrical boundary conditions. With the origin of the coordinates at the center of the plate,

$$\text{at } x = \pm \frac{A}{2}, \quad w_x = 0, \quad w_{,x} = 0, \quad (6.5)$$

$$\text{and at } y = \pm \frac{B}{2}, \quad w_y = 0, \quad w_{,y} = 0, \quad (6.6)$$

for the clamped plate. By the principle of stationary potential energy, taking the partial derivatives of the total potential energy with respect to A_{mn} ,

$$\frac{\partial (U - V_w)}{\partial A_{mn}} = 0, \quad (6.7)$$

a set of algebraic equations for the unknown coefficients A_{mn} will be obtained.

6.3 A_{mn} for Clamped Supports

Using algebraic feasible functions $w(x)$ and $w(y)$ in Formula (6.4), an assumed solution might take the form

$$w(x, y) = \sum_{m=1}^{\infty} \sum_{n=1}^{\infty} A_{mn} \left(x^2 - \frac{A^2}{4}\right)^2 \left(y^2 - \frac{B^2}{4}\right)^2 x^{m-1} y^{n-1}. \quad (6.8)$$

Other feasible functions might be trigonometric expressions, for instance,

$$w(x) = 1 + \cos \frac{2m\pi x}{A} \quad (m = \text{odd}), \quad (6.9)$$

$$w(x) = 1 - \cos \frac{2m\pi x}{A} \quad (m = \text{even}). \quad (6.10)$$

Similarly,

$$w(y) = 1 + \cos \frac{2n\pi y}{B} \quad (n = \text{odd}), \quad (6.11)$$

$$w(y) = 1 - \cos \frac{2n\pi y}{B} \quad (n = \text{even}). \quad (6.12)$$

Without going into detailed calculations, the value of A_{mn} for the feasible functions, Formulas (6.9) to (6.12), resulted in Formula (6.14). Four-term approximation was used with $m = 1, 2$ and $n = 1, 2$. Solution from the trigonometric expressions called for a set of simultaneous algebraic equations in the form of

$$[C] \{A\} = \{D\}. \quad (6.13)$$

The column matrix $\{D\}$ has four identical elements of (pAB) , A and B being the two sides of the grid. The column matrix of the unknown coefficients is

$$\{A\} = (A_{11} \ A_{12} \ A_{21} \ A_{22}). \quad (6.14)$$

$$[C] = \text{a square matrix,} \quad (6.15)$$

with its elements defined as follows:

$$c_{11} = 8\pi^4 \left(\frac{3D_x}{A^3} + \frac{4H}{AB} + \frac{3D_y}{B^3} \right)$$

$$c_{12} = \frac{16\pi^4 D_x}{A^3}$$

$$c_{13} = \frac{16\pi^4 D_y}{B^3}$$

$$c_{14} = 0$$

$$c_{21} = \frac{16\pi^4 D_x}{A^3}$$

$$c_{22} = 8\pi^4 \left(\frac{3D_x}{A^3} + \frac{16}{AB} + \frac{48D_y}{B^3} \right)$$

$$c_{23} = 0$$

$$c_{24} = \frac{256\pi^4 D_y}{B^3}$$

$$c_{31} = \frac{16\pi^4 D_y}{B^3}$$

$$c_{32} = 0$$

$$c_{33} = 8\pi^4 \left(\frac{48D_x}{A^3} + \frac{4}{AB} + \frac{3D_y}{B^3} \right)$$

$$c_{34} = \frac{256\pi^4 D_x}{AB}$$

$$c_{41} = 0$$

$$c_{42} = \frac{256\pi^4 D_y}{B^3}$$

$$c_{43} = \frac{256\pi^4 D_x}{A^3}$$

$$c_{44} = 8\pi^4 \left(\frac{48D_x}{A^3} + \frac{16}{AB} + \frac{48D_y}{B^3} \right).$$

One-term approximation for the trigonometric expressions was also done, resulting in

$$A_{11} = \frac{+p}{8\pi^4} \frac{A^4 B^4}{(4D_x B^4 + HA^2 B^2 + 4D_y A^4)}. \quad (6.16)$$

6.4 Loading Conditions

So far the grid under investigation has been loaded by a uniform load. If the loading conditions are others, the design procedure remains similar. The loading $q(x, y)$ is frequently expressed as a product of two functions, each depending on one direction, or

$$q(x, y) = p f(x) g(y). \quad (6.17)$$

The constant uniform load $q(x, y) = p$ is thus replaced by the loading function. Both functions $f(x)$ and $g(y)$ are expanded by the series of a similar type as used in the expression for the deflection w .

Suppose in PROBLEM 3 the loading is a concentrated load P at the point (x_1, y_1) instead of a uniformly distributed load, $UDL = p$. First, replace the concentrated load P by a local uniform load UDL of the size u by v , with its center at (x_1, y_1) , (Fig 17). The deflection is found to be

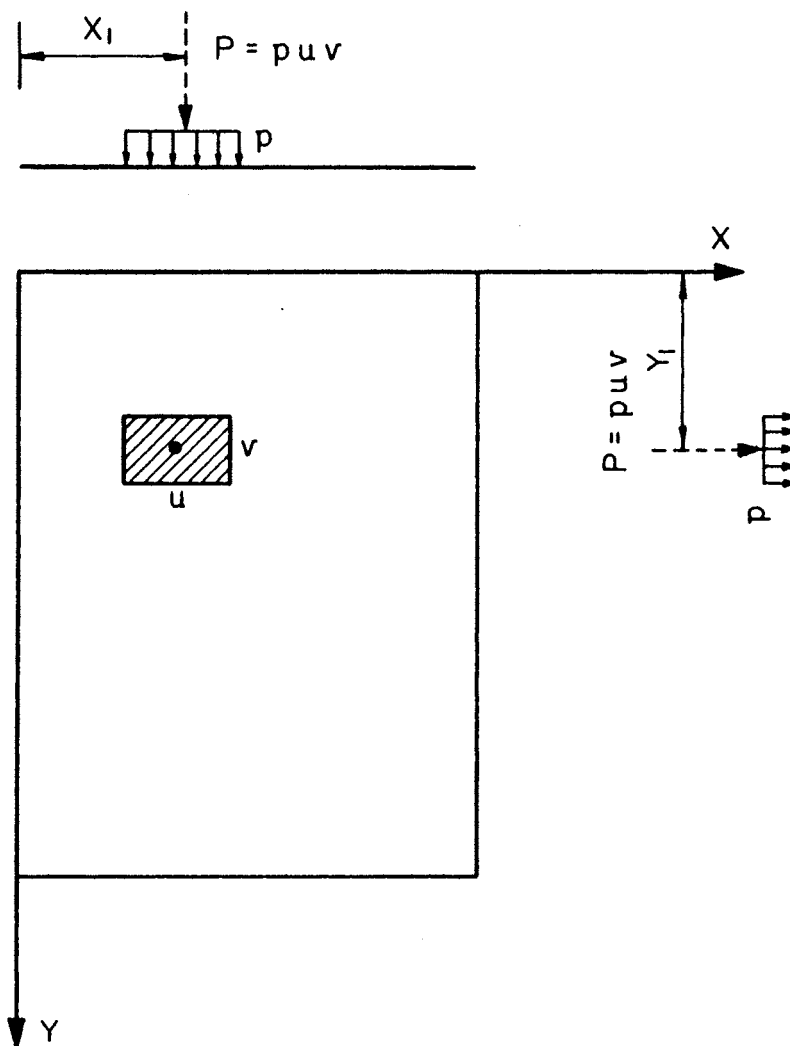


Figure 17. Local Uniform Load Shrinking to a Concentrated Load

$$\begin{aligned}
w(x, y) = & \sum_{m=1,3}^{\infty} \sum_{n=1,3}^{\infty} \\
& \frac{16p}{\pi^6 mn \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \\
& \left[\sin \frac{m\pi u}{2A} \sin \frac{n\pi v}{2B} \sin \frac{m\pi x_1}{A} \sin \frac{n\pi y_1}{B} \right. \\
& \left. \sin \frac{m\pi x}{A} \sin \frac{n\pi y}{B} \right]. \tag{6.18}
\end{aligned}$$

Replacing p by $\frac{P}{uv}$ and rearranging, Formula (6.18) becomes

$$\begin{aligned}
w(x, y) = & \sum_{m=1,3}^{\infty} \sum_{n=1,3}^{\infty} \\
& \frac{16P}{\pi^6 mn \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \\
& \left[\frac{\sin \frac{m\pi x_1}{A} \sin \frac{n\pi y_1}{B}}{\frac{2A}{m\pi} \frac{2B}{n\pi}} \right] \left[\frac{\sin \frac{m\pi u}{2A} \sin \frac{n\pi v}{2B}}{\frac{m\pi u}{2A} \frac{n\pi v}{2B}} \right] \\
& \left[\sin \frac{m\pi x}{A} \sin \frac{n\pi y}{B} \right]. \tag{6.19}
\end{aligned}$$

When the local UDL is shrinking to a point, the limit is taken as

$$\lim_{u, v \rightarrow 0} w(x, y). \tag{6.20}$$

Applying the L'Hospital's Rule,

$$\lim_{t \rightarrow 0} \frac{\sin t}{t} = \lim_{t \rightarrow 0} \frac{\cos t}{1} = 1, \tag{6.21}$$

the second to the last term in brackets of Formula (6.19) becomes unity. The solution is

$$w(x, y) = \sum_{m=1,3,\dots}^{\infty} \sum_{n=1,3,\dots}^{\infty} \frac{4P}{\pi^4 AB \left[\left(\frac{m}{A}\right)^4 D_x + \left(\frac{mn}{AB}\right)^2 2H + \left(\frac{n}{B}\right)^4 D_y \right]} \left[\sin \frac{m\pi x}{A} \sin \frac{n\pi y}{B} \sin \frac{m\pi x}{A} \sin \frac{n\pi y}{B} \right]. \quad (6.22)$$

This looks similar to the solution of PROBLEM 3, also with a modification factor of four sine-terms due to this particular case.

6.5 Boundary Dimensions

The boundary dimensions were chosen to be such that the dimension in the x-direction was identical to that in the y-direction because the design variables associated with x were expected to yield solutions identical to those associated with y. This symmetrical solution served as one qualitative check on itself.

This postulation was tested by considering the simplest grid, with one beam in each direction (Fig 18). Let an x-beam with moment of inertia I_x rest on top of a y-beam with a single load P at the intersection and with four ends simply supported. At the intersection, the reaction on the x-beam,

$$R = P \left(\frac{I_y A^3}{I_x B^3 + I_y A^3} \right), \quad (6.23)$$

$$\begin{aligned} \max M_x &= \frac{PA}{4} - \frac{RA}{4} \\ &= \frac{PA}{4} \left(\frac{I_x B^3}{I_x B^3 + I_y A^3} \right). \end{aligned} \quad (6.24)$$

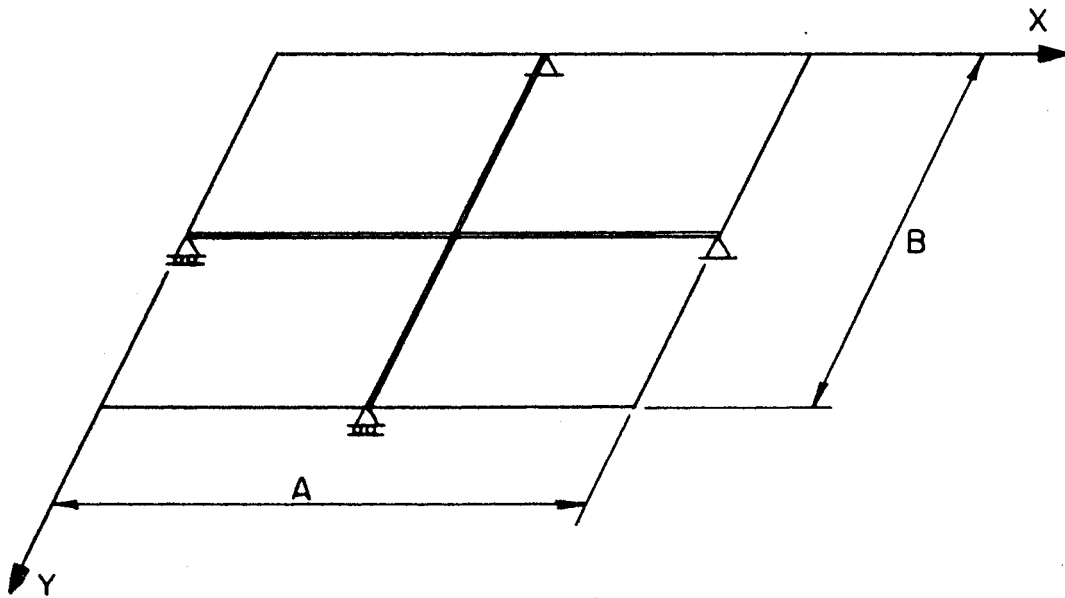


Figure 18. Simplest Grid with One Beam in Each Direction

For the y-beam,

$$\max M_y = \frac{PB}{4} \left(\frac{I_y A^3}{I_x B^3 + I_y A^3} \right) . \quad (6.25)$$

Then, for both beams to be fully stressed to the same amount,

$$M_x = M_y,$$

$$\frac{PA}{4} \left(\frac{I_x B^3}{I_x B^3 + I_y A^3} \right) = \frac{PB}{4} \left(\frac{I_y A^3}{I_x B^3 + I_y A^3} \right) . \quad (6.26)$$

If $A = B$, then

$$I_x = I_y. \quad (6.27)$$

Therefore, the postulation was proved, using the simplest grid.

With this postulation, the square grid could have been treated with less design variables, namely using "I" in place of I_x and I_y , and so on. However, this was not done so that the problem could deal with a general rectangular grid.

Usually, the two sides of the rectangular grid are given. However, one side, say A in the x-direction, can be specified and the other side can be expressed as a multiple of A , or

$$B = R_1 A, \quad (6.28)$$

where R_1 = ratio of the two sides.

By introducing this new variable R_1 , the optimal solution will reveal whether a square grid or a rectangular grid with a particular ratio of sides is an economical shape of the grid.

Another approach along this line is to make several runs of the problem with

$$R_1 = 1, 2, 3, 4, \dots$$

and to plot a graph of R_1 versus Min V. The minimum point of this curve will be the most economical ratio of the two sides.

CHAPTER VII

DISCUSSION OF RESULTS AND CONCLUSION

The three problems were labeled in Chapters III and IV as follows:

PROBLEM 1 --- Beams spanning in one direction ,

PROBLEM 2 --- A grid with beams spanning in two orthogonal directions (simple-connection),

PROBLEM 3 --- A grid with beams spanning in two orthogonal directions (rigid-connection).

All of them had a boundary condition of simple supports and a uniformly distributed load.

It should be pointed out again that the formulation was intended to be for a general rectangular grid, as explained in Section 6.5. The square grid was used as a qualitative check on its own solution, where it might seem to suggest superfluous use of design variables.

From the solutions of PROBLEMS 1, 2, and 3 (Tables IV and V), one common feature was evident. The spacings of beams a and b and the web slendernesses λ_x and λ_y reached the maximum as explained in Section 5.4. This is a reasonable result because maximum spacing and maximum web slenderness contribute to economy. However, it should not be concluded that maximum spacings (upper bound) can always be achieved. With increasing magnitude of uniform load (greater than the one used), bigger beams would be required to carry the bigger load until the upper bound of moment of inertia was reached. With even

heavier load, the spacings of beams would decrease so that more beams of the same largest section specified could carry this heavy load.

Similarly, suppose for some reason that it is desired to use small beams. If all these small beams with upper bounds of spacings cannot carry the load, then more beams of this section will be required, or smaller spacings. Also, it should not be concluded that the upper bound of web slenderness can always be achieved. Within safety limits (constraints) maximum values of spacings and web slenderness would be attained, as explained in Section 5.4.

As mentioned in Chapter I, the objective function could be a cost function if a unit cost was incorporated with the volume. Also, it is possible to cut holes in the appropriate locations of the web of the beam or to use the castellated beam (Fig 19) in order to reduce the volume further. However, although it is of academic interest, it is not recommended because of increase of fabrication cost unless weight is the prime important factor. Efforts in deriving expressions for this beam and high fabrication costs may not warrant this saving of material.

Another factor of economy is in the choice between plate girders and standard rolled sections. The design was based on I-shaped plate girders. However, for other shapes, section properties could be derived in a similar manner. Their costs vary from company to company and also from locality to locality. Similarly, simply-connection (PROBLEM 2) should be compared with rigid-connection (PROBLEM 3) for economy in fabrication and erection costs since material cost is the same in both cases. Because of the variable nature of different kinds of cost, the volume of material was chosen as the objective function. With known unit cost function the objective function will

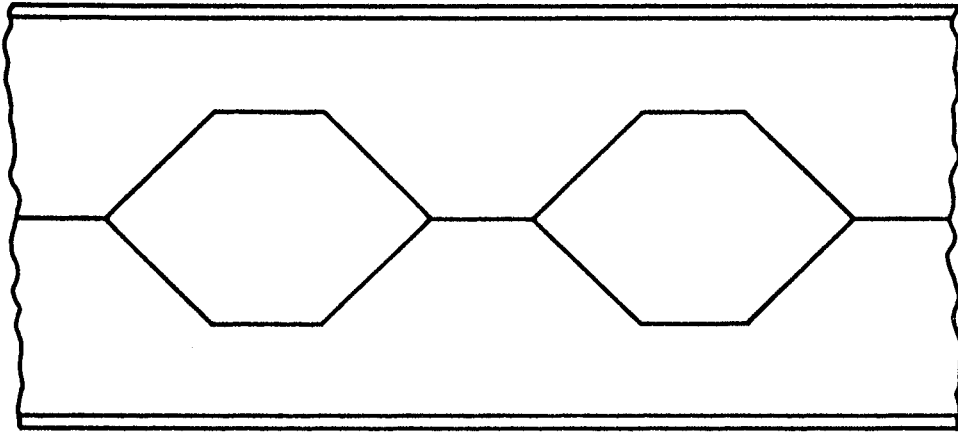


Figure 19. Castellated Beam

yield a minimum cost rather than a minimum volume.

It is concluded as follows:

- (1) This approach by Mathematical Programming is feasible in the design of a grid and is superior to the conventional design procedure, because of material economy.
- (2) This approach is flexible in the choice of the two essential components: the Structural Analysis method and the Mathematical Programming technique.
- (3) The analysis by the Theory of Plates proves to be economical in computer computations.
- (4) Exs (5.1) to (5.5) illustrated the three cases discussed in Section 3.5.2 and demonstrated that it is not always the fully stressed design which is the optimum. PROBLEM 1 was solved by using the analytical approach (Section 3.5) and the MSLP Code (Modified Sequence of Linear Programming Solutions), coming to a good agreement.
- (5) PROBLEM 2 and PROBLEM 3 produced the same solution because the nonvanishing parameter H in the latter was relatively small for the case considered. These two problems were found to be fully stressed designs. However, the web slendernesses reached their upper bounds but the spacings did not.
- (6) In a sense, as explained in Section 5.4, spacings and web slendernesses reached their maximum possible values in all three problems.
- (7) All three problems are nonconvex problems. The MSLP Code (Modified Sequence of Linear Programming Solutions)

works only for PROBLEM 1; the RGM Code (Ricochet Gradient Method) works for all three problems.

A few points should be made as suggestions:

- (1) As a generalization, the problem is not limited to a Civil Engineering problem but may be one that has been idealized to a model of rectangular grid.
- (2) This approach is also flexible in the choice of objective function. Depending on applications, if the weight of the structure is an important factor, the volume will be chosen as the objective function. If the weight is of no concern, then the cost function should be the objective function.
- (3) The section properties for other shapes could be derived in a similar manner to the I-shaped welded plate girder used here. Holes may be cut from the web of the beam (Fig 19) to reduce the weight.
- (4) As explained in Section 6.5, the ratios of the two sides of the rectangular grid could be regarded as a variable to be optimized.

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APPENDIX A

COMPUTER PROGRAM LISTING--THE MSLP CODE,
(MODIFIED SEQUENCE OF LINEAR
PROGRAMMING SOLUTIONS)

80/80 LIST

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0000000011111111222222223333333344444444555555556666666677777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0001 C-----***** IP 10
0002 C ***** IP 250
0003 C ***** IP 240
0004 C ***** IP 130
0005 C ***** IP 80
0006 C
0007 C
0008 C
0009 C
0010 C
0011 C.....-----
0012 C-----COMPUTER CODE MSLP
0013 C (MODIFIED SEQUENCE OF LINEAR PROGRAMMING SOLUTIONS)
0014 C-----PROGRAMMED BY M.S. CHAN AT OKLAHOMA STATE UNIVERSITY,
0015 C STILLWATER, OKLAHOMA, UNDER THE SUPERVISION OF DR. M.
0016 C TOCHACEK IN 1971.
0017 C.....-----
0018 C ..... IP 300
0019 C ..... MAIN PROGRAM ..... IP 310
0020 C REQUIRED SUBROUTINES IP 320
0021 C (1) SUBROUTINES PTCN ----- REQ 3 IP 330
0022 C (2) SUBROUTINES BNEG1 IP 340
0023 C (3) SUBROUTINES LP2 ----- REQ 1 IP 350
0024 C ..... IP 360
0025 C
0026 C
0027 C
0028 C
0029 C
0030 DIMENSION X(12), XM(12), IZERO(12), XP(12)
0031 DIMENSION A(65, 65), M(65), L(65)
0032 DIMENSION PRDT(45), VALX(65)
0033 DIMENSION C1(30,12), C2(30,25), C(30,50)
0034 DIMENSION FX(12), GC(25), GX(25,12),G(25)
0035 DIMENSION GLX(25,12), GLC(25)
0036 DIMENSION BL(12), UB(12)
0037 COMMON MN, LIN,>NNL, NPR
0038 5 FORMAT (15, E13.4 )
0039 11 FORMAT ( 10I5) IP 460
0040 12 FORMAT (4(8E10.4//)) IP 470
0041 13 FORMAT (4(5X, 8E15.4//)) IP 480
0042 14 FORMAT ( 3I5, F10.2) IP 490
0043 15 FORMAT ( T5,'ITER**',T20,'RATIO*',T35,'ZVAL**',T50 , 'VMIN ****' IP 500
0044 1*****', / 5X, 15, 8E15.4 / ) IP 510
0045 16 FORMAT ( 5X, 8E15.4 / ) IP 520
0046 17 FORMAT ( 110, 10F10.2 ) IP 530
0047 22 FORMAT ( 7E11.4 ) IP 540
0048 151 FORMAT ( '1' ) IP 550
0049 152 FORMAT ( ///5X, ' X(1)' / )
0050 154 FORMAT (/////10X, 'PROBLEM ', I3/) IP 560
0051 155 FORMAT ( 5X, ' RATIO .LE. 0.05 ***** CONVERGENCE *****' / IP 570
0052 1 5X, '*****' ) IP 580
0053 156 FORMAT ( 5X, 'CASE = ', I3 ) IP 590
0054 157 FORMAT ( 5X,I5, 10(1PE12.3 ) ) IP 600

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80/80 LIST

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0000000011111111222222223333333344444444555555556666666677777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0055 207 FORMAT (5X,'FOR .LE.-INEQ, JUST THE UNIT MATRIX ' / IP 610
0056 1 5X,'FOR .GE.-INEQ, INSERT -1.0 IN THE REAR AND -M ON OBJ FCN' IP 620
0057 2 / ) IP 630
0058 208 FORMAT(5X,' INPUT DATA FORM' / ) IP 640
0059 201 FORMAT(5X,' 1 ' / ) IP 650
0060 202 FORMAT(5X,' 4 7 ' / ) IP 660
0061 203 FORMAT(5X,' 1.0 0 0 0 -G1X1-G1X2-G1X3' / IP 670
0062 1 5X,' 0 1.0 0 0 -G2X1-G2X2-G2X3' / IP 680
0063 2 5X,' 0 0 1.0 0 -G3X1-G3X2-G3X3' / IP 690
0064 3 5X,' 0 0 0 1.0 -G4X1-G4X2-G4X3' / ) IP 700
0065 204 FORMAT(5X,' 0 0 0 0 FX1 FX2 FX3 ' / ) IP 710
0066 205 FORMAT(5X,' G1C G2C G3C G4C ' / ) IP 720
0067 209 FORMAT ( T5,'X(1) ',T20,'X(2) ',T35,'X(3) ' / , 5X, 8E15.4/) IP 730
0068 C IP 800
0069 C ----- LINEARIZATION ----- IP 810
0070 C IP 820
0071 101 FORMAT ( 2X, 10E13.4///) IP 830
0072 102 FORMAT ( T5,' ',T20,' ',T35,' ',T50,' ', IP 840
0073 1 T65,' ',T80,' ',T95,' ',T110,' ') IP 850
0074 103 FORMAT ( T5,'XL ',T20,'PLL1 ',T35,'SIGA ',T50,'XK ', IP 860
0075 3 T65,'E ',T80,'XM ',T95,'PI ',T110,'P ') IP 870
0076 104 FORMAT ( T5,'C1 ',T20,'C2 ',T35,'C3 ',T50,'C4 ', IP 880
0077 4 T65,'C5 ',T80,'B1 ',T95,'B2 ',T110,'B3 ') IP 890
0078 105 FORMAT ( T5,'F ',T20,'G1 ',T35,'G2 ',T50,'G3 ', IP 900
0079 1 T65,'G4 ',T80,'G5 ',T95,'G6 ',T110,'G7 ') IP 910
0080 106 FORMAT ( T5,'FX1 ',T20,'FX2 ',T35,'FX3 ',T50,'G1X1 ', IP 920
0081 1 T65,'G1X2 ',T80,'G1X3 ',T95,' ',T110,' ') IP 930
0082 107 FORMAT ( T5,'G2X1 ',T20,'G2X2 ',T35,'G2X3 ',T50,'G3X1 ', IP 940
0083 1 T65,'G3X2 ',T80,'G3X3 ',T95,' ',T110,' ') IP 950
0084 108 FORMAT ( T5,'G4X1 ',T20,'G4X2 ',T35,'G4X3 ',T50,'FC ', IP 960
0085 1 T65,'G1C ',T80,'G2C ',T95,'G3C ',T110,'G4C ') IP 970
0086 109 FORMAT ( T5,'FC ',T20,'FX1 ',T35,'FX2 ',T50,'FT ', IP 980
0087 1 T65,'FTMX ',T80,'FTMIN ',T95,' ',T110,' ') IP 990
0088 111 FORMAT ( 20X, '0 - EXACT G3' / IP 1000
0089 1 20X, '1 - EXACT G2' ) IP 1010
0090 112 FORMAT ( 20X, '0 - APPROX G3' / IP 1020
0091 1 20X, '1 - APPROX G2' ) IP 1030
0092 113 FORMAT ( 20X, '0 - EXACT G3', 10X, '2 - APPROX G3' / IP 1040
0093 1 20X, '1 - EXACT G2', 10X, '3 - APPROX G2' ) IP 1050
0094 114 FORMAT ( 8F10.3 ) IP 1060
0095 115 FORMAT ( 5X, 3I5, 8E12.4 ) IP 1070
0096 591 FORMAT ( /15X, ' COEF VECTOR ' ) IP 1080
0097 592 FORMAT ( /15X, 'ART-VAR VECTOR ' ) IP 1090
0098 593 FORMAT ( /15X, 'OBJ-FCN VECTOR ' ) IP 1100
0099 594 FORMAT ( /15X, 'VECTOR B (PRDT) ' ) IP 1110
0100 595 FORMAT ( /15X, 'MATRIX C (1ST PART ' ) IP 1120
0101 596 FORMAT ( /15X, 'MATRIX C (2ND PART ' ) IP 1130
0102 597 FORMAT ( /15X, 'MATRIX C (3RD PART ' ) IP 1140
0103 C IP 1150
0104 C ----- READ IN DATA -----(CON 01--17 ) IP 1160
0105 C IP 1170
0106 C IP 1180
0107 900 READ 11, NPROB IP 750
0108 PRINT 151 IP 760

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80/80 LIST

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CARD
0109      IF (NPROB) 1000, 9999, 1000                                IP 770
0110      1000      NPROB = NPROB + 1
0111      PRINT 154, NPROB
0112      C----- 1ST RUN                                          IP 1190
0113      C
0114      ITER = 0                                                    IP 1200
0115      C ITAB= 1 FOR ALL TABLES, 0 FOR JUST RESULTS              IP 1210
0116      C N= NO. OF CONSTRAINTS, M= NO. OF VARIABLES              IP 1220
0117      READ 11, NPR
0118      PRINT 11, NPR
0119      READ 14, LIN, NNL, MN, VART                                IP 1260
0120      PRINT 14, LIN, NNL, MN, VART
0121      N = LIN + NNL
0122      M = N + MN
0123      PRINT 14,N,M
0124      READ 22, ( XM(J),J=1, MN )
0125      PRINT 209
0126      PRINT 101,( XM(J),J=1, MN )
0127      DO 880 I = 1, MN
0128      880      XP(I) = XM(I)
0129      CALL LFCN ( GLX, GLC, BL, UB )
0130      C
0131      C----- FOR .LE.-INEQ, JUST THE UNIT MATRIX                IP 1340
0132      C----- FOR .GE.-INEQ, INSERT -1.0 IN THE REAR AND -M ON OBJ FCN IP 1350
0133      C----- INPUT DATA FORM                                  IP 1360
0134      C-----
0135      C-----
0136      C-----
0137      C-----
0138      C-----
0139      C-----
0140      C-----
0141      C-----
0142      C-----
0143      C-----
0144      C-----
0145      C
0146      PRINT 207
0147      PRINT 208
0148      PRINT 201
0149      PRINT 202
0150      PRINT 203
0151      PRINT 204
0152      PRINT 205
0153      C
0154      C ----- READ IN DATA -----(CON 01--17 )
0155      C
0156      705 CONTINUE
0157      M = N + MN
0158      IF ( ITER .GT. 5 ) GO TO 9999
0159      CALL PTCN ( N, M, C1, VALX, PROT, XM, ITER, FC) IP 1610
0160      CALL BNEGL( N, M, C1, PRDT, VALX, VART, NC, C, C2,NG) IP 1620
0161      C
0162      C ..... PRINT MATRIX C ..... IP 1630

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80/80 LIST

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CARD
0163 C IP 1650
0164 M = NC IP 1660
0165 C----- VALX = COEF OF X IN OBJ FCN IP 1670
0166 C----- PRDT = VECTOR B IP 1680
0167 C---- PRINT 11, ITAB
0168 PRINT 11, N, M IP 1700
0169 C IP 1710
0170 C----- SIMPLEX METHOD IP 1720
0171 C IP 1730
0172 LP = 2 IP 1740
0173 IF ( LP .EQ. 2 ) GO TO 706 IP 1750
0174 DO 751 K = 1, NC IP 1760
0175 VALX (K) = -VALX(K) IP 1770
0176 751 CONTINUE IP 1780
0177 LP = 1 IP 1790
0178 PRINT 209 IP 1800
0179 PRINT 101, ( X(I), I = 1, MN ) IP 1810
0180 GO TO 707 IP 1820
0181 706 CONTINUE IP 1830
0182 CALL LP2( PRDT, A, I, II, JJ, L, X, C1, C2, N, M, NG, ZVAL, VALX ) IP 1840
0183 707 CONTINUE IP 1850
0184 C IP 1860
0185 C IP 1870
0186 C----- 2ND RUN TILL CONVERGENCE IP 1880
0187 C IP 1890
0188 C---- GO TO 871
0189 872 NZ = 0
0190 PRINT 152
0191 DO 870 I = 1, MN
0192 IF ( X(I) .GT. BL(I) ) GO TO 873
0193 NZ = NZ + 1
0194 X(I) = XP(I)
0195 873 PRINT 5, I, X(I)
0196 870 CONTINUE
0197 PRINT 5, NZ
0198 CALL FCNGR ( X, F, FC, FX, G, GC, GX ) IP 8230
0199 VMIN = F
0200 C----- VMIN = -ZVAL + FC
0201 871 CONTINUE
0202 ITER = ITER + 1 IP 1920
0203 IF ( ITER .EQ. 1 ) GO TO 701 IP 1930
0204 AVMIN = ABS(VMIN) IP 1940
0205 DIFF = AVMIN - AZMAX IP 1950
0206 ADIFF = ABS(DIFF)
0207 DIV = ADIFF / AZMAX
0208 RATIO = ABS ( DIV ) IP 1970
0209 PRINT 16, AZMAX, AVMIN, DIFF, RATIO IP 1980
0210 860 IF ( RATIO .LE. 0.05 ) GO TO 901 IP 1990
0211 701 CONTINUE
0212 C---- DO 754 I = 1, MN
0213 C 754 XM(I) = 0.0
0214 NZERO = 0 IP 2040
0215 DO 703 I = 1, MN IP 2050
0216 XM(I) = X(I) IP 2060

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80/80 LIST

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CARD
0217          IF ( XM(I) .NE. 0.0 ) GO TO 703          IP 2070
0218          NZERO = NZERO + 1                          IP 2080
0219          IZERO(NZERO) = 1                          IP 2090
0220    703    CONTINUE                                  IP 2100
0221          IF (NZERO .LE. 1 ) GO TO 752              IP 2110
0222          PRINT 11, NZERO, ( IZERO(K), K = 1, NZERO ) IP 2120
0223          GO TO 753                                  IP 2130
0224    752    PRINT 11, NZERO, IZERO(1)                IP 2140
0225    753    CONTINUE                                  IP 2150
0226          PRINT 16 , FC, F
0227          ZMAX = VMIN
0228          AZMAX = ABS ( ZMAX )                        IP 2010
0229          RATIO = 0.0
0230    702    PRINT 15, ITER, RATIO, ZVAL, VMIN        IP 2160
0231          IF ( VMIN .LT. 0.0 ) GO TO 9999           IP 2170
0232          IF ( NZERO .GT. 0 ) GO TO 9999           IP 2180
0233          PRINT 151                                  IP 2190
0234          GO TO 705
0235    901    CONTINUE                                  IP 2200
0236          PRINT 16 , FC, F
0237          PRINT 15, ITER, RATIO, ZVAL, VMIN
0238          PRINT 155                                  IP 2220
0239    9999  STOP                                       IP 2240
0240          END                                       IP 2250

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80/80 LIST

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CARD
0001      SUBROUTINE PTCO ( N,      M,      C1, VALX, PRDT, XM, ITER, FC)  IP 7590
0002      COMMON MN, LIN, NNL, NPR
0003      DIMENSION X(12), XM(12), XA(12)
0004      DIMENSION          PRDT(45), VALX(65)
0005      DIMENSION          C1(30,25), C(30,50)
0006      DIMENSION FX(12), G(25), GC(25), GX(25,12)
0007      DIMENSION GLX(25,12), GLC(25)
0008      DIMENSION BL(12), UB(12)
0009 C
0010 C
0011 C.....
0012 C          REQUIRED SUBROUTINES
0013 C          (1) SUBROUTINES FCNGR
0014 C          (2) SUBROUTINES FPINTP-----REQ 4
0015 C          (3) SUBROUTINES LFCN
0016 C.....
0017      16  FORMAT ( 5X, 8E15.4 / )
0018      22  FORMAT ( 7E11.4 )
0019      23  FORMAT ( ' PTCO      ', I5, 8E13.4 / 14X, 8E13.4)
0020      101  FORMAT ( 2X, 8E15.4 /// )
0021      102  FORMAT ( T5, '          ', T20, '          ', T35, '          ', T50, '          ',
0022              T65, '          ', T80, '          ', T95, '          ', T110, '          ' )
0023      103  FORMAT ( T5, 'XL      ', T20, 'PLL1 ', T35, 'SIGA ', T50, 'XK      ',
0024              T65, 'E          ', T80, 'XM      ', T95, 'PI      ', T110, 'P          ' )
0025      104  FORMAT ( T5, 'C1      ', T20, 'C2      ', T35, 'C3      ', T50, 'C4      ',
0026              T65, 'C5      ', T80, 'B1      ', T95, 'B2      ', T110, 'B3      ' )
0027      105  FORMAT ( T5, 'F          ', T20, 'G1      ', T35, 'G2      ', T50, 'G3      ',
0028              T65, 'G4      ', T80, 'G5      ', T95, 'G6      ', T110, 'G7      ' )
0029      106  FORMAT ( T5, 'FX1     ', T20, 'FX2     ', T35, 'FX3     ', T50, 'G1X1    ',
0030              T65, 'G1X2    ', T80, 'G1X3    ', T95, '          ', T110, '          ' )
0031      107  FORMAT ( T5, 'G2X1    ', T20, 'G2X2    ', T35, 'G2X3    ', T50, 'G3X1    ',
0032              T65, 'G3X2    ', T80, 'G3X3    ', T95, '          ', T110, '          ' )
0033      108  FORMAT ( T5, 'G4X1    ', T20, 'G4X2    ', T35, 'G4X3    ', T50, 'FC          ',
0034              T65, 'G1C     ', T80, 'G2C     ', T95, 'G3C     ', T110, 'G4C     ' )
0035      109  FORMAT ( T5, 'FC          ', T20, 'FX1     ', T35, 'FX2     ', T50, 'FT          ',
0036              T65, 'FTMX    ', T80, 'FTMIN   ', T95, '          ', T110, '          ' )
0037      114  FORMAT ( 8F10.3 )
0038      200  FORMAT ( 5X, E15.4, 7E15.7 )
0039      202  FORMAT (/5X, ' INFEASIBLE POINT ' / )
0040      203  FORMAT (/5X, ' OUTSIDE FEASIBLE REGION, BACK UP ' / )
0041      204  FORMAT (/5X, ' POINT ON THE CONSTRAINT G1 ' / )
0042      205  FORMAT (/5X, ' POINT ON THE CONSTRAINT G2 ' / )
0043      206  FORMAT (/5X, ' POINT ON THE CONSTRAINT G3 ' / )
0044      207  FORMAT (/5X, ' POINT ON THE CONSTRAINT G', I2, / )
0045      601  FORMAT (1X, 'A', E15.4, 7E15.7 )
0046      602  FORMAT (1X, 'B', E15.4, 7E15.7 )
0047      603  FORMAT (1X, 'C', E15.4, 7E15.7 )
0048      604  FORMAT (1X, 'D', E15.4, 7E15.7 )
0049      605  FORMAT (1X, 'E', E15.4, 7E15.7 )
0050      606  FORMAT (1X, 'F', E15.4, 7E15.7 )
0051      607  FORMAT (1X, 'G', E15.4, 7E15.7 )
0052      608  FORMAT (1X, 'H', E15.4, 7E15.7 )
0053      609  FORMAT (1X, 'I', E15.4, 7E15.7 )
0054      610  FORMAT (1X, 'J', E15.4, 7E15.7 )
    
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80/80 LIST

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0000000011111111222222223333333344444444555555556666666677777777778
1234567890123456789012345678901234567890123456789012345678901234567890

CARD
0055 611 FORMAT (1X,'K',E15.4, 7E15.7 ) IP 8130
0056 612 FORMAT (1X,'L',E15.4, 7E15.7 ) IP 8140
0057 613 FORMAT ( /5X, ' AWAY FROM G1' / ) IP 8150
0058 614 FORMAT ( /5X, ' AWAY FROM G2' / ) IP 8160
0059 615 FORMAT ( /5X, ' AWAY FROM G3' / ) IP 8170
0060 616 FORMAT ( /5X, ' AWAY FROM G', I2, / ) IP 8180
0061 DO 218 I = 1, MN IP 8190
0062 218 XA(I) = XM(I) IP 8200
0063 DO 219 I = 1, MN IP 8210
0064 219 X(I) =XA(I) IP 8220
0065 PRINT 23 , ITER, ( X(I), I = 1, MN )
0066 CALL FCNGR ( X, F, FC, FX, G, GC, GX ) IP 8230
0067 FCT = FC IP 8240
0068 DO 515 I = 1, N IP 8250
0069 515 VALX(I) = 0.0 IP 8260
0070 K = 0 IP 8270
0071 DO 516 K = 1, MN IP 8280
0072 516 VALX (N+K) = FX(K) IP 8290
0073 PRINT 16, ( VALX(I), I = 1, M ) IP 8300
0074 IF ( ITER .NE. 0 ) GO TO 710 IP 8310
0075 C----- TEST FOR FEASIBILITY IP 8320
0076 IF ( NNL .EQ. 0 ) GO TO 708 IP 8330
0077 PRINT 16, ( G(K), K = 1, NNL )
0078 DO 720 K = 1, NNL IP 8350
0079 IF ( G(K) .GE. 0.0 ) GO TO 720 IP 8360
0080 PRINT 202 IP 8370
0081 720 CONTINUE IP 8380
0082 PRINT 16, ( G(K), K = 1, NNL )
0083 708 CONTINUE IP 8390
0084 710 CONTINUE
0085 C IP 8400
0086 C ..... IP 8410
0087 C IP 8420
0088 C----- WHEN VECTOR B IS NEGATIVE DO THE FCLLOWING IP 8430
0089 C ---- FOUR CASES IP 8440
0090 C 1---- ALL +VE IP 8450
0091 C 2---- (1) -VE,/ (2) -VE,/ (3) -VE IP 8460
0092 C 3---- (1), (2), -VE/ (1) (3) -VE /(2), (3) -VE IP 8470
0093 C 4---- (1), (2), (3) -VE IP 8480
0094 C IP 8490
0095 C ..... ALL +VE IP 8500
0096 C IP 8510
0097 KCASE = 100. IP 8520
0098 C IP 8530
0099 C ..... IP 8540
0100 C IP 8550
0101 C IP 8560
0102 C----- NON-LINEAR COND IP 8570
0103 C ..... PUT POINT ON SURFACE ..... IP 8580
0104 C IP 8600
0105 NEQ = 7
0106 NEQ = 0
0107 KNN = 0
0108 KN = 0

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80/80 LIST

00000000111111112222222233333333444444445555555566666666777777778
 1234567890123456789012345678901234567890123456789012345678901234567890

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CARD
0109          KC = 0
0110          IF ( NNL .EQ. 0 ) GO TO 391
0111    350    CONTINUE
0112          KN = KN + 1
0113          IF ( KN .GT. NNL ) GO TO 391
0114          IF ( ITER .NE. 0 ) GO TO 392
0115    C----- FOR EQUALITY FROM NEQ ON
0116          IF ( NEQ .EQ. 0 ) GO TO 394
0117          IF ( KN .LT. NEQ ) GO TO 394
0118          KNN = KNN + 1
0119          DO 395 I = 1, MN
0120    395    XA(I) = XM(I)
0121          CALL      FPINTP ( XA, KNN, X )
0122          GO TO 392
0123    394    CONTINUE
0124          DO 303 I = 1, MN
0125    303    XA(I) = XM(I)
0126          CALL      FPINTP ( XA, KN, X )
0127    392    CONTINUE
0128          PRINT 23 , ITER, ( X(I), I = 1, MN )
0129          CALL      FCNGR ( X,      F, FC, FX, G, GC, GX )
0130          PRDT(KN) = GC(KN)
0131    C
0132    C ..... FORM MATRIX C1 ROW-WISE .....
0133    C
0134          DO 390 J = 1, MN
0135    390    C1(KN,J) = -GX(KN,J)
0136          GO TO 350
0137    391    CONTINUE
0138          IF ( NNL .NE. 0 ) GO TO 393
0139          KN = 1
0140    393    CONTINUE
0141    C----- LINEAR COND
0142          IF ( LIN .EQ. 0 ) GO TO 9999
0143          CALL      LFCN ( GLX, GLC, BL, UB )
0144    370    CONTINUE
0145          KC = KC + 1
0146          IF ( KN .GT. (LIN+NNL) ) GO TO 9999
0147          PRDT (KN) = GLC(KC )
0148          DO 360 J = 1, MN
0149    360    C1 (KN,J) =-GLX(KC,J)
0150          KN = KN + 1
0151          GO TO 370
0152    9999    FC = FCT
0153          RETURN
0154          END

```

IP 8680
 IP 8690
 IP 8700
 IP 8710
 IP 8720
 IP 8730
 IP 8740
 IP 8750
 IP 8760
 IP 8770
 IP 8780
 IP 8790
 IP 8800
 IP 8810
 IP 8820
 IP 8830
 IP 8840
 IP 8850
 IP 8870
 IP 8880
 IP 8890
 IP 8900
 IP 8910
 IP 8920
 IP 8930
 IP 8940
 IP 8950
 IP 8960
 IP 8970

80/80 LIST

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00000000111111112222222233333333444444445555555566666666777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARO
0055          GO TO 201                                IP 9400
0056      202          X(K) = X1(K)                    IP 9410
0057      201          CONTINUE                          IP 9420
0058          PRINT 101, ( X(K), K = 1, MN )           IP 9430
0059      301          CONTINUE                          IP 9450
0060          RETURN                                    IP 9460
0061          END                                       IP 9470
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80/80 LIST

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0000000001111111112222222222333333333344444444445555555555666666666677777777778
12345678901234567890123456789012345678901234567890123456789012345678901234567890
CARD
0001      SUBROUTINE FPM (FUNCT, X,F,G,EST,EPS,LIMIT,IER,H, NFCN,      IP  10
0002      1          FA, X1, X2 )      IP  20
0003      COMMON MN, LIN, NNL, NPR
0004      C          DIMENSIONED DUMMY VARIABLES      IP  40
0005      DIMENSION H(1),X(1),G(1)      IP  50
0006      DIMENSION FA(8 ),          X1(12), X2(12)
0007      C
0008      C          .....      IP  80
0009      C
0010      C          SUBROUTINE FPM      IP 100
0011      C          MODIFIED FMFP      IP 110
0012      C
0013      C          .....      IP 130
0014      C
0015      C          .....      IP 140
0016      C
0017      C
0018      C          .....      IP 180
0019      C          REQUIRED SUBROUTINES      IP 190
0020      C          (1) SUBROUTINES  FUNCT ----- REQ 1      IP 200
0021      C          .....      IP 210
0022      101  FORMAT ( 7E11.4 )      IP 220
0023      105  FORMAT (/ ' F1' , 7E11.4 / )      IP 230
0024      102  FORMAT (/ ' F2' , 7E11.4 / )      IP 240
0025      103  FORMAT (/ ' F3' , 7E11.4 / )      IP 250
0026      104  FORMAT (/ ' F4' , 7E11.4 / )      IP 260
0027      C          COMPUTE FUNCTION VALUE AND GRADIENT VECTOR FOR INITIAL ARGUMENT      IP 270
0028      N = MN      IP 280
0029      MN = N      IP 290
0030      CALL  FUNCT(      X, F , G, NFCN, X1, X2, FA )      IP 300
0031      IF ( NPR .EQ. 0 ) GO TO 60
0032      PRINT 105, F
0033      CONTINUE
0034      60      IF ( F ) 61, 56, 62      IP 320
0035      62      FA(1) = F      IP 330
0036      DO 64 I = 1, N      IP 340
0037      64      X1(I) = X(I)      IP 350
0038      IF ( ABS(F) .GT. 0.2E-02 ) GO TO 67
0039      DO 66 I = 1, N
0040      66      X2(I) = X1(I)
0041      RETURN
0042      67      CONTINUE
0043      IF ( NPR .EQ. 0 ) GO TO 68
0044      PRINT 101, (X1(K), K = 1, MN )
0045      CONTINUE
0046      GO TO 63      IP 370
0047      61      FA(2) = F      IP 380
0048      DO 65 I = 1, N      IP 390
0049      65      X2(I) = X(I)      IP 400
0050      IF ( NPR .EQ. 0 ) GO TO 69
0051      PRINT 101, (X2(K), K = 1, MN )
0052      69      CONTINUE
0053      GO TO 56      IP 420
0054      63      CONTINUE      IP 430

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80/80 LIST

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00000000111111112222222233333333444444445555555566666666777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0055 C IP 440
0056 C RESET ITERATION COUNTER AND GENERATE IDENTITY MATRIX IP 450
0057 IER=0 IP 460
0058 KOUNT=0 IP 470
0059 N2=N+N IP 480
0060 N3=N2+N IP 490
0061 N31=N3+1 IP 500
0062 1 K=N31 IP 510
0063 DO 4 J=1,N IP 520
0064 H(K)=1. IP 530
0065 NJ=N-J IP 540
0066 IF(NJ)5,5,2 IP 550
0067 2 DO 3 L=1,NJ IP 560
0068 KL=K+L IP 570
0069 3 H(KL)=0. IP 580
0070 4 K=KL+1 IP 590
0071 C IP 600
0072 C START ITERATION LOOP IP 610
0073 5 KOUNT=KOUNT +1 IP 620
0074 C IP 630
0075 C SAVE FUNCTION VALUE, ARGUMENT VECTOR AND GRADIENT VECTOR IP 640
0076 OLDF=F IP 650
0077 DO 9 J=1,N IP 660
0078 K=N+J IP 670
0079 H(K)=G(J) IP 680
0080 K=K+N IP 690
0081 H(K)=X(J) IP 700
0082 C IP 710
0083 C DETERMINE DIRECTION VECTOR H IP 720
0084 K=J+N3 IP 730
0085 T=0. IP 740
0086 DO 8 L=1,N IP 750
0087 T=T-G(L)*H(K) IP 760
0088 IF(L-J)6,7,7 IP 770
0089 6 K=K+N-L IP 780
0090 GO TO 8 IP 790
0091 7 K=K+1 IP 800
0092 8 CONTINUE IP 810
0093 9 H(J)=T IP 820
0094 C IP 830
0095 C CHECK WHETHER FUNCTION WILL DECREASE STEPPING ALONG H. IP 840
0096 DY=0. IP 850
0097 HNRM=0. IP 860
0098 GNRM=0. IP 870
0099 C IP 880
0100 C CALCULATE DIRECTIONAL DERIVATIVE AND TESTVALUES FOR DIRECTION IP 890
0101 C VECTOR H AND GRADIENT VECTOR G. IP 900
0102 DO 10 J=1,N IP 910
0103 HNRM=HNRM+ABS(H(J)) IP 920
0104 GNRM=GNRM+ABS(G(J)) IP 930
0105 10 DY=DY+H(J)*G(J) IP 940
0106 C IP 950
0107 C REPEAT SEARCH IN DIRECTION OF STEEPEST DESCENT IF DIRECTIONAL IP 960
0108 C DERIVATIVE APPEARS TO BE POSITIVE OR ZERO. IP 970

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80/80 LIST

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00000000111111112222222233333333444444445555555566666666777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0109          IF(DY)11,51,51                                IP 980
0110 C                                               IP 990
0111 C          REPEAT SEARCH IN DIRECTION OF STEEPEST DESCENT IF DIRECTION
0112 C          VECTOR H IS SMALL COMPARED TO GRADIENT VECTOR G.      IP 1000
0113 C          11 IF(HNRM/GNRM-EPS)51,51,12                    IP 1010
0114 C                                               IP 1020
0115 C          SEARCH MINIMUM ALONG DIRECTION H                  IP 1030
0116 C                                               IP 1040
0117 C          SEARCH ALONG H FOR POSITIVE DIRECTIONAL DERIVATIVE  IP 1050
0118 C          12 FY=F                                           IP 1060
0119 C          ALFA=2.*(EST-F)/DY                                 IP 1070
0120 C          AMBDA=1.                                          IP 1080
0121 C                                               IP 1090
0122 C          USE EST(MATE FOR STEPSIZE ONLY IF IT IS POSITIVE AND LESS THAN
0123 C          1. OTHERWISE TAKE 1. AS STEPSIZE                    IP 1100
0124 C          IF(ALFA)15,15,13                                   IP 1110
0125 C          13 IF(ALFA-AMBDA)14,15,15                         IP 1120
0126 C          14 AMBDA=ALFA                                     IP 1130
0127 C          15 ALFA=0.                                        IP 1140
0128 C                                               IP 1150
0129 C          SAVE FUNCTION AND DERIVATIVE VALUES FOR OLD ARGUMENT
0130 C          16 Fx=FY                                           IP 1160
0131 C          Dx=DY                                             IP 1170
0132 C                                               IP 1180
0133 C          STEP ARGUMENT ALONG H                              IP 1190
0134 C          DD 17 (=1,N)                                       IP 1200
0135 C          17 X(I)=X(I)+AMBDA*H(I)                            IP 1210
0136 C                                               IP 1220
0137 C          COMPUTE FUNCTION VALUE AND GRADIENT FOR NEW ARGUMENT
0138 C          CALL FUNCT( X, F, G, NFCN, X1, X2, FA )             IP 1230
0139 C          IF ( NPR .EQ. 0 ) GO TO 70                          IP 1240
0140 C          PRINT 102, F                                         IP 1250
0141 C          CONTINUE                                           IP 1260
0142 C          IF ( F ) 71, 56, 72                                IP 1270
0143 C          72 FA(1) = F                                         IP 1290
0144 C          DO 74 I = 1, N                                       IP 1300
0145 C          74 X1(I) = X(I)                                       IP 1310
0146 C          IF ( ABS(F) .GT. 0.2E-02 ) GO TO 77                 IP 1320
0147 C          DO 76 I = 1, N                                       IP 1330
0148 C          76 X2(I) = X1(I)                                       IP 1340
0149 C          RETURN                                               IP 1350
0150 C          CONTINUE                                           IP 1360
0151 C          IF ( NPR .EQ. 0 ) GO TO 78                          IP 1370
0152 C          PRINT 101, (X1(K), K = 1, MN )
0153 C          CONTINUE
0154 C          GO TO 73                                               IP 1340
0155 C          71 FA(2) = F                                         IP 1350
0156 C          DO 75 I = 1, N                                       IP 1360
0157 C          75 X2(I) = X(I)                                       IP 1370
0158 C          IF ( NPR .EQ. 0 ) GO TO 79
0159 C          PRINT 101, (X2(K), K = 1, MN )
0160 C          CONTINUE
0161 C          GO TO 56                                               IP 1390
0162 C          73 CONTINUE                                           IP 1400

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80/80 LIST

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0000000011111111222222223333333344444444555555556666666677777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0163          FY=F                                IP 1410
0164          C                                  IP 1420
0165          C          COMPUTE DIRECTIONAL DERIVATIVE DY FOR NEW ARGUMENT.  TERMINATE  IP 1430
0166          C          SEARCH, IF DY IS POSITIVE. IF DY IS ZERO THE MINIMUM IS FOUND  IP 1440
0167          DY=0.                                IP 1450
0168          DO 18 I=1,N                          IP 1460
0169          18 DY=DY+G(I)*H(I)                  IP 1470
0170          IF(DY)19,36,22                      IP 1480
0171          C                                  IP 1490
0172          C          TERMINATE SEARCH ALSO IF THE FUNCTION VALUE INDICATES THAT  IP 1500
0173          C          A MINIMUM HAS BEEN PASSED  IP 1510
0174          19 IF(FY-FX)20,22,22                IP 1520
0175          C                                  IP 1530
0176          C          REPEAT SEARCH AND DOUBLE STEPSIZE FOR FURTHER SEARCHES  IP 1540
0177          20 AMBDA=AMBDA*ALFA                  IP 1550
0178          ALFA=AMBDA                           IP 1560
0179          C          END OF SEARCH LOOP        IP 1570
0180          C                                  IP 1580
0181          C          TERMINATE IF THE CHANGE IN ARGUMENT GETS VERY LARGE  IP 1590
0182          IF(HNRH*AMBDA-1.E10)16,16,21       IP 1600
0183          C                                  IP 1610
0184          C          LINEAR SEARCH TECHNIQUE INDICATES THAT NO MINIMUM EXISTS  IP 1620
0185          21 IER=2                             IP 1630
0186          RETURN                               IP 1640
0187          C                                  IP 1650
0188          C          INTERPOLATE CUBICALLY IN THE INTERVAL DEFINED BY THE SEARCH  IP 1660
0189          C          ABOVE AND COMPUTE THE ARGUMENT X FOR WHICH THE INTERPOLATION  IP 1670
0190          C          POLYNOMIAL IS MINIMIZED  IP 1680
0191          22 T=0.                              IP 1690
0192          23 IF(AMBDA)24,36,24                 IP 1700
0193          24 Z=3.*(FX-FY)/AMBDA+DX+DY         IP 1710
0194          ALFA=AMAX1(ABS(Z),ABS(DX),ABS(DY))  IP 1720
0195          DALFA=Z/ALFA                         IP 1730
0196          DALFA=DALFA*DALFA-DX/ALFA*DY/ALFA  IP 1740
0197          IF(DALFA)151,25,25                  IP 1750
0198          25 W=ALFA*SQRT(DALFA)               IP 1760
0199          ALFA=DY-DX+W+W                       IP 1770
0200          IF(ALFA) 250,251,250               IP 1780
0201          250 ALFA=(DY-Z+W)/ALFA              IP 1790
0202          GO TO 252                            IP 1800
0203          251 ALFA=(Z+DY-W)/(Z+DX+Z+DY)       IP 1810
0204          252 ALFA=ALFA*AMBDA                 IP 1820
0205          DO 26 I=1,N                          IP 1830
0206          26 X(I)=X(I)+(T-ALFA)*H(I)         IP 1840
0207          C                                  IP 1850
0208          C          TERMINATE, IF THE VALUE OF THE ACTUAL FUNCTION AT X IS LESS  IP 1860
0209          C          THAN THE FUNCTION VALUES AT THE INTERVAL ENDS. OTHERWISE REDUCE  IP 1870
0210          C          THE INTERVAL BY CHOOSING ONE END-POINT EQUAL TO X AND REPEAT  IP 1880
0211          C          THE INTERPOLATION. WHICH END-POINT IS CHOOSEN DEPENDS ON THE  IP 1890
0212          C          VALUE OF THE FUNCTION AND ITS GRADIENT AT X                IP 1900
0213          C                                  IP 1910
0214          CALL  FUNCT( X, F, G, NFCN, X1, X2, FA )  IP 1920
0215          IF ( NPR .EQ. 0 ) GO TO 80
0216          PRINT 103, F

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80/80 LIST

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0000000001111111112222222222333333333344444444445555555555666666666677777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0217      80      CONTINUE
0218              IF ( F ) 81, 56, 82
0219      82              FA(1) = F
0220              DO 84 I = 1, N
0221      84              X1(I) = X(I)
0222              IF ( ABS(F) .GT. 0.2E-02 ) GO TO 87
0223              DO 86 I = 1, N
0224      86              X2(I) = X1(I)
0225              RETURN
0226      87      CONTINUE
0227              IF ( NPR .EQ. 0 ) GO TO 88
0228              PRINT 101, (X1(K), K = 1, MN )
0229      88      CONTINUE
0230              GO TO 83
0231      81              FA(2) = F
0232              DO 85 I = 1, N
0233      85              X2(I) = X(I)
0234              IF ( NPR .EQ. 0 ) GO TO 89
0235              PRINT 101, (X2(K), K = 1, MN )
0236      89      CONTINUE
0237              GO TO 56
0238      83      CONTINUE
0239              IF(F-FX)27,27,28
0240      27 IF(F-FY)36,36,28
0241      28 DALFA=0.
0242              DO 29 I=1,N
0243      29 DALFA=DALFA+G(I)*H(I)
0244              IF(DALFA)30,33,33
0245      30 IF(F-FX)32,31,33
0246      31 IF(DX-DALFA)32,36,32
0247      32 FX=F
0248              DX=DALFA
0249              T=ALFA
0250              AMBDA=ALFA
0251              GO TO 23
0252      33 IF(FY-F)35,34,35
0253      34 IF(DY-DALFA)35,36,35
0254      35 FY=F
0255              DY=DALFA
0256              AMBDA=AMBDA-ALFA
0257              GO TO 22
0258 C
0259 C          TERMINATE, IF FUNCTION HAS NOT DECREASED DURING LAST ITERATION
0260      36 IF(OLDF-F+EPS)51,38,38
0261 C
0262 C          COMPUTE DIFFERENCE VECTORS OF ARGUMENT AND GRADIENT FROM
0263 C          TWO CONSECUTIVE ITERATIONS
0264      38 DO 37 J=1,N
0265              K=N+J
0266              H(K)=G(J)-H(K)
0267              K=N+K
0268      37 H(K)=X(J)-H(K)
0269 C
0270 C          TEST LENGTH OF ARGUMENT DIFFERENCE VECTOR AND DIRECTION VECTOR
0270 C

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80/80 LIST

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12345678901234567890123456789012345678901234567890123456789012345678901234567890
CARD
0271 C IF AT LEAST N ITERATIONS HAVE BEEN EXECUTED. TERMINATE, IF IP 2380
0272 C BOTH ARE LESS THAN EPS IP 2390
0273 IER=0 IP 2400
0274 IF(KOUNT-N)42,39,39 IP 2410
0275 39 T=0. IP 2420
0276 Z=0. IP 2430
0277 DO 40 J=1,N IP 2440
0278 K=N+J IP 2450
0279 W=H(K) IP 2460
0280 K=K+N IP 2470
0281 T=T+ABS(H(K)) IP 2480
0282 40 Z=Z+W*H(K) IP 2490
0283 IF(HNRN-EPS)41,41,42 IP 2500
0284 41 IF(T-EPS)56,56,42 IP 2510
0285 C IP 2520
0286 C TERMINATE, IF NUMBER OF ITERATIONS WOULD EXCEED LIMIT IP 2530
0287 42 IF(KOUNT-LIMIT)43,50,50 IP 2540
0288 C IP 2550
0289 C PREPARE UPDATING OF MATRIX H IP 2560
0290 43 ALFA=0. IP 2570
0291 DO 47 J=1,N IP 2580
0292 K=J+N3 IP 2590
0293 W=0. IP 2600
0294 DO 46 L=1,N IP 2610
0295 KL=N+L IP 2620
0296 W=W+H(KL)*H(K) IP 2630
0297 IF(L-J)44,45,45 IP 2640
0298 44 K=K+N-L IP 2650
0299 GO TO 46 IP 2660
0300 45 K=K+1 IP 2670
0301 46 CONTINUE IP 2680
0302 K=N+J IP 2690
0303 ALFA=ALFA+W*H(K) IP 2700
0304 47 H(J)=W IP 2710
0305 C IP 2720
0306 C REPEAT SEARCH IN DIRECTION OF STEEPEST DESCENT IF RESULTS IP 2730
0307 C ARE NOT SATISFACTORY IP 2740
0308 IF(Z*ALFA)48,1,48 IP 2750
0309 C IP 2760
0310 C UPDATE MATRIX H IP 2770
0311 48 K=N31 IP 2780
0312 DO 49 L=1,N IP 2790
0313 KL=N2+L IP 2800
0314 DO 49 J=L,N IP 2810
0315 NJ=N2+J IP 2820
0316 H(K)=H(K)+H(KL)*H(NJ)/Z-H(L)*H(J)/ALFA IP 2830
0317 49 K=K+1 IP 2840
0318 GO TO 5 IP 2850
0319 C END OF ITERATION LOOP IP 2860
0320 C IP 2870
0321 C NO CONVERGENCE AFTER LIMIT ITERATIONS IP 2880
0322 50 IER=1 IP 2890
0323 RETURN IP 2900
0324 C IP 2910

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80/80 LIST

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0000000001111111112222222223333333334444444445555555556666666667777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0325 C RESTORE OLD VALUES OF FUNCTION AND ARGUMENTS IP 2920
0326 51 DO 52 J=1,N IP 2930
0327 K=N2+J IP 2940
0328 52 X(J)=H(K) IP 2950
0329 CALL FUNCT( X, F, G, NFCN, X1, X2, FA ) IP 2960
0330 IF ( NPR .EQ. 0 ) GO TO 90
0331 PRINT 104, F
0332 90 CONTINUE
0333 IF ( F ) 91, 56, 92 IP 2980
0334 92 FA(1) = F IP 2990
0335 DO 94 I = 1, N IP 3000
0336 94 X1(I) = X(I) IP 3010
0337 IF ( ABS(F) .GT. 0.2E-02 ) GO TO 97
0338 DO 96 I = 1, N
0339 96 X2(I) = X1(I)
0340 RETURN
0341 97 CONTINUE
0342 IF ( NPR .EQ. 0 ) GO TO 98
0343 PRINT 101, (X1(K), K = 1, MN )
0344 98 CONTINUE
0345 GO TO 93 IP 3030
0346 91 FA(2) = F IP 3040
0347 DO 95 I = 1, N IP 3050
0348 95 X2(I) = X(I) IP 3060
0349 IF ( NPR .EQ. 0 ) GO TO 99
0350 PRINT 101, (X2(K), K = 1, MN )
0351 99 CONTINUE
0352 GO TO 56 IP 3080
0353 93 CONTINUE IP 3090
0354 C IP 3100
0355 C REPEAT SEARCH IN DIRECTION OF STEEPEST DESCENT IF DERIVATIVE IP 3110
0356 C FAILS TO BE SUFFICIENTLY SMALL IP 3120
0357 IF (GNRM-EPS)55,55,53 IP 3130
0358 C IP 3140
0359 C TEST FOR REPEATED FAILURE OF ITERATION IP 3150
0360 53 IF (IER)56,54,54 IP 3160
0361 54 IER=-1 IP 3170
0362 GOTO 1 IP 3180
0363 55 IER=0 IP 3190
0364 56 RETURN IP 3200
0365 END IP 3210

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80/80 LIST

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00000000111111112222222233333333444444445555555566666666777777778
1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0001      SUBROUTINE FUNCT(      X, FS, G, NFCN, X1, X2, FA )      IP 3220
0002      COMMON MN, LIN, NML, NPR
0003      DIMENSION GF(15)      IP 3240
0004      DIMENSION FA(8 ),      X1(12), X2(12)
0005      DIMENSION X(12 ),G(12)
0006      DIMENSION FX(12),      GC(25), GX(25,12)
0007      C      IP 3300
0008      C      IP 3310
0009      C      IP 3360
0010      C      IP 3370
0011      C..... IP 3380
0012      C          REQUIRED SUBROUTINES      IP 3390
0013      C          (1) SUBROUTINES FCNGR      IP 3400
0014      C..... IP 3410
0015      C----- DEFINE NNL,LF, J, IN SUBR FUNCT      IP 3320
0016      790      FORMAT ( 1X, ' FS NEGATIVE ', E15.4 )      IP 3420
0017      801      FORMAT ( 5X, 2I5, 7E14.4 / 15X, 7E14.4 / 15X, 7E14.4 / )      IP 3430
0018      802      FORMAT ( 5X, 6E14.4 / )      IP 3440
0019      804      FORMAT(15X, ' A ', 6E14.4/18X,6E14.4//)
0020      805      FORMAT ( 5X, 6E14.4 )      IP 3460
0021      806      FORMAT ( 15X, ' FC' , 6E14.4 //)      IP 3470
0022      807      FORMAT (/ , ' FS' , 6E14.4 //)      IP 3480
0023      C      IP 3490
0024      LF = 0      IP 3500
0025      CALL      FCNGR ( X,      F, FC, FX,GF, GC, GX )      IP 3520
0026      C..... IP 3530
0027      J = NFCN      IP 3540
0028      FS = GF(J)      IP 3560
0029      DO 850 K = 1, MN      IP 3570
0030      850      G(K) = GX(J,K)      IP 3580
0031      C      IP 3590
0032      C..... IP 3600
0033      C----- PRINT 804, ( G(I), I = 1,MN ), FS
0034      C----- PRINT 807, FS
0035      9999      RETURN      IP 3630
0036      END

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80/80 LIST

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CARD
0001      SUBROUTINE BNEG1( N,      M, C1, PRDT, VALX, VART, NC, C, C2,NG) IP 2260
0002      DIMENSION                PRDT(45), VALX(65)
0003      DIMENSION C1(30,12), C2(30,25), C(30,50)
0004      DIMENSION                UM(30,50),ZC1(30,50),ZC2(30,50)
0005      COMMON MN, LIN, NNL, NPR
0006      C ..... IP 2300
0007      C ..... (2) SUBROUTINE BNEG ... HANDLES NEGATIVE ELE IN B ... IP 2310
0008      C ..... IP 2320
0009      C ..... IP 2330
0010      C ..... REQUIRED SUBROUTINES IP 2340
0011      C ..... NONE IP 2350
0012      C ..... IP 2360
0013      11  FORMAT ( 10I5) IP 2370
0014      22  FORMAT ( 7E11.4 ) IP 2380
0015      101 FORMAT ( 2X, 8E15.4 /// ) IP 2390
0016      156 FORMAT ( 5X, 'CASE = ', I3 ) IP 2400
0017      591 FORMAT ( /15X, ' COEF VECTOR ' ) IP 2410
0018      592 FORMAT ( /15X, 'ART-VAR VECTOR ' ) IP 2420
0019      593 FORMAT ( /15X, 'OBJ-FCN VECTOR ' ) IP 2430
0020      594 FORMAT ( /15X, 'VECTOR B (PRDT) ' ) IP 2440
0021      595 FORMAT ( /15X, 'MATRIX C (1ST PART ' ) IP 2450
0022      596 FORMAT ( /15X, 'MATRIX C (2ND PART ' ) IP 2460
0023      597 FORMAT ( /15X, 'MATRIX C (3RD PART ' ) IP 2470
0024      598 FORMAT ( /15X, 'MATRIX C ' / ) IP 2480
0025      C ..... IP 2490
0026      C ..... TEST FOR NEG ELE IN VECTOR B (PRDT) ..... IP 2500
0027      C ..... IP 2510
0028      NPR = 0 IP 2520
0029      NPR = 1
0030      KCASE = 100 IP 2530
0031      NG = 0 IP 2540
0032      DO 515 J = 1, N IP 2550
0033      IF ( PRDT(J) .GT. 0.0 ) GO TO 515 IP 2560
0034      NG = NG + 1 IP 2570
0035      515 CONTINUE IP 2580
0036      PRINT 156, KCASE IP 2590
0037      C ..... IP 2600
0038      C ..... COEF VECTOR ..... IP 2610
0039      C ..... IP 2620
0040      DO 520 I = 1, N IP 2630
0041      DO 520 J = 1, MN IP 2640
0042      IF (PRDT(I) .LT. 0.0 ) GO TO 521 IP 2650
0043      C1(I,J) = C1(I,J) IP 2660
0044      GO TO 520 IP 2670
0045      521 C1(I,J) = -C1(I,J) IP 2680
0046      520 CONTINUE IP 2690
0047      PRINT 591 IP 2700
0048      IF ( NPR .EQ. 0 ) GO TO 701 IP 2710
0049      DO 525 I = 1, N IP 2720
0050      C ----- PUNCH 22, ( C1(I,J), J = 1, MN ) IP 2730
0051      525 PRINT 101, ( C1(I,J), J = 1, MN ) IP 2740
0052      701 CONTINUE IP 2750
0053      C ..... IP 2760
0054      C ..... ART-VAR VECTOR ..... IP 2770

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80/80 LIST

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CARD
0055 C IP 2780
0056 PRINT 11 , NG IP 2790
0057 IF ( NG .EQ. 0 ) GO TO 544 IP 2800
0058 IF ( NG .GT. 1 ) GO TO 533 IP 2810
0059 J = 1 IP 2820
0060 DO 536 I = 1, N IP 2830
0061 C2(I,J) = 0.0 IP 2840
0062 536 CONTINUE IP 2850
0063 K = 0 IP 2860
0064 DO 537 I = 1, N IP 2870
0065 IF ( PRD(I) .GT. 0.0 ) GO TO 537 IP 2880
0066 C2(I,J) = -1.0 IP 2890
0067 537 CONTINUE IP 2900
0068 PRINT 592 IP 2910
0069 IF ( NPR .EQ. 0 ) GO TO 702 IP 2920
0070 DO 538 I = 1, N IP 2930
0071 538 PRINT 101, C2(I,J) IP 2940
0072 702 CONTINUE IP 2950
0073 GO TO 544 IP 2960
0074 533 DO 531 J = 1, NG IP 2970
0075 DO 531 I = 1, N IP 2980
0076 C2(I,J) = 0.0 IP 2990
0077 531 CONTINUE IP 3000
0078 K = 0 IP 3010
0079 532 DO 556 I = 1, N IP 3020
0080 IF = 0 IP 3030
0081 DO 557 J = 1, NG IP 3040
0082 IF ( IF .EQ. 1 ) GO TO 557 IP 3050
0083 IF ( PRD(I) .GT. 0.0 ) GO TO 557 IP 3060
0084 K = K + 1 IP 3070
0085 C2(I,K) = -1.0 IP 3080
0086 IF = 1 IP 3090
0087 557 CONTINUE IP 3100
0088 556 CONTINUE IP 3110
0089 PRINT 592 IP 3120
0090 IF ( NPR .EQ. 0 ) GO TO 703 IP 3130
0091 DO 535 I = 1, N IP 3140
0092 C ----- PUNCH 22, ( C2(I,J), J = 1, NG ) IP 3150
0093 535 PRINT 101, ( C2(I,J), J = 1, NG ) IP 3160
0094 703 CONTINUE IP 3170
0095 C IP 3180
0096 C ..... OBJ-FCN VECTOR ..... IP 3190
0097 C IP 3200
0098 544 NC = N + MN + NG IP 3210
0099 PRINT 11 , NC IP 3220
0100 DO 540 J = 1, NC IP 3230
0101 IF ( J .GT. N ) GO TO 542 IP 3240
0102 IF ( PRD(J) .LT. 0.0 ) VALX(J) = VART IP 3250
0103 GO TO 540 IP 3260
0104 542 IF ( J .GT. N .AND. J .LE. M ) GO TO 543 IP 3270
0105 VALX(J) = 0.0 IP 3280
0106 GO TO 540 IP 3290
0107 543 VALX(J) = VALX(J) IP 3300
0108 540 CONTINUE IP 3310

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80/80 LIST

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CARD
0109          PRINT 593                                IP 3320
0110          IF ( NPR .EQ. 0 ) GO TO 704              IP 3330
0111 C ----- PUNCH 22, ( VALX(J), J = 1, NC )      IP 3340
0112          PRINT 101, ( VALX(J), J = 1, NC )       IP 3350
0113          704  CONTINUE                             IP 3360
0114 C                                                IP 3370
0115 C-----PUT THIS CONTROL CARD BEFORE //-CARD     IP 3380
0116 C----- CHANGE CC TO //                         IP 3390
0117 CCGO.SYSPUNCH DD SYSQUT=B                       IP 3400
0118 C                                                IP 3410
0119 C                                                IP 3420
0120 C ..... VECTER B (PRDT) .....                  IP 3430
0121 C                                                IP 3440
0122          DO 550 J = 1, N                           IP 3450
0123          IF ( PRDT(J) .LT. 0.0 ) PRDT(J) = -PRDT(J) IP 3460
0124          PRDT(J) = PRDT(J)                         IP 3470
0125          550  CONTINUE                             IP 3480
0126          PRINT 594                                 IP 3490
0127          IF ( NPR .EQ. 0 ) GO TO 705              IP 3500
0128 C ----- PUNCH 22, ( PRDT(J), J = 1, N )       IP 3510
0129          PRINT 101, ( PRDT(J), J = 1, N )       IP 3520
0130          705  CONTINUE                             IP 3530
0131 C                                                IP 3540
0132 C ..... FORM MATRIX C .....                    IP 3550
0133 C                                                IP 3560
0134 C ..... 1ST PART .....                          IP 3570
0135 C                                                IP 3580
0136          DO 570 I = 1, N                           IP 3590
0137          DO 570 J = 1, NC                           IP 3600
0138          IF ( I .EQ. J ) GO TO 571                IP 3610
0139          UM(I,J) = 0.0                             IP 3620
0140          GO TO 570                                 IP 3630
0141          571  UM(I,J) = 1.0                         IP 3640
0142          570  CONTINUE                             IP 3650
0143          PRINT 595                                 IP 3660
0144 C                                                IP 3670
0145 C ..... 2ND PRAT .....                           IP 3680
0146 C                                                IP 3690
0147          DO 575 I = 1, N                           IP 3700
0148          DO 575 J = 1, NC                           IP 3710
0149          ZC1(I,J) = 0.0                             IP 3720
0150          575  CONTINUE                             IP 3730
0151          DO 576 I = 1, N                           IP 3740
0152          DO 576 J = 1, MN                           IP 3750
0153          K = J + N                                  IP 3760
0154          ZC1(I,K) = C1(I,J)                        IP 3770
0155          576  CONTINUE                             IP 3780
0156          PRINT 596                                 IP 3790
0157 C                                                IP 3800
0158 C ..... 3RD PART .....                           IP 3810
0159 C                                                IP 3820
0160          DO 577 I = 1, N                           IP 3830
0161          DO 577 J = 1, NC                           IP 3840
0162          ZC2(I,J) = 0.0                             IP 3850

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CARD
0163 577 CONTINUE IP 3860
0164 IF ( NG .EQ. 0 ) GO TO 582 IP 3870
0165 IF ( NG .GT. 1 ) GO TO 583 IP 3880
0166 J = 1 IP 3890
0167 DO 584 I = 1, N IP 3900
0168 K = J + M IP 3910
0169 ZC2(I,K) = C2(I,J) IP 3920
0170 584 CONTINUE IP 3930
0171 GO TO 582 IP 3940
0172 583 DO 578 I = 1, N IP 3950
0173 DO 578 J = 1, NG IP 3960
0174 K = J + M IP 3970
0175 ZC2(I,K) = C2(I,J) IP 3980
0176 578 CONTINUE IP 3990
0177 PRINT 597 IP 4000
0178 582 DO 580 I = 1, N IP 4010
0179 DO 580 J = 1, NC IP 4020
0180 C(I,J) = UM(I,J) + ZC1(I,J) + ZC2(I,J) IP 4030
0181 580 CONTINUE IP 4040
0182 PRINT 598 IP 4050
0183 RETURN IP 4060
0184 END IP 4070

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80/80 LIST

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CARD
0001      SUBROUTINE CALA2(      II, (II, JJ, A, PRDT ,C1,C2,N,M,NG,VALX) IP 6650
0002      COMMON MN, LIN, NNL, NPR
0003      DIMENSION      PRDT(40), VALX(65)
0004      DIMENSION A(65, 65), DA(65, 65)
0005      DIMENSION C1(30,12), C2(30,25)
0006      DIMENSION UM(30,50)
0007 C----- SUBR CALA2 IS TO TRANSFORM MATRIX C TO MATRIX A      IP 6720
0008 C----- SO THAT SUBR LP2 CAN BE USED      IP 6730
0009 C      IP 6740
0010 C..... IP 6750
0011 C      REQUIRED SUBROUTINES      IP 6760
0012 C      NONE      IP 6770
0013 C..... IP 6780
0014      1      FORMAT (10I5)      IP 6790
0015      13     FORMAT (4(5X, 8E15.4/))      IP 6800
0016      22     FORMAT (7E11.4 )      IP 6810
0017      580    FORMAT (/5X, ' MATRIX A ' / )      IP 6820
0018      591    FORMAT ( /15X, ' COEF VECTOR ' )      IP 6830
0019      592    FORMAT ( /15X, 'ART-VAR VECTOR ' )      IP 6840
0020      593    FORMAT ( /15X, 'OBJ-FCN VECTOR ' )      IP 6850
0021      594    FORMAT ( /15X, 'VECTOR B (PRDT) ' )      IP 6860
0022      NPR = 1
0023      NPRT= 1
0024      DO 502 I = 1, N      IP 6890
0025      DO 502 J = 1, N      IP 6900
0026      IF ( I .EQ. J ) GO TO 501      IP 6910
0027      UM(I,J) = 0.0      IP 6920
0028      GO TO 502      IP 6930
0029      501    UM(I,J) = 1.0      IP 6940
0030      502    CONTINUE      IP 6950
0031      PRINT 591      IP 6960
0032      IF ( NPRT.EQ. 0 ) GO TO 701
0033      DO 510 I = 1, N      IP 6980
0034      PRINT 13, ( C1(I,J), J = 1, MN )      IP 6990
0035      510    CONTINUE      IP 7000
0036      701    CONTINUE      IP 7010
0037      PRINT 592      IP 7020
0038      PRINT 1, NG      IP 7030
0039      IF ( NG .LT. 2 ) GO TO 521      IP 7040
0040      IF ( NPRT.EQ. 0 ) GO TO 702
0041      DO 520 I = 1, N      IP 7060
0042      PRINT 13, ( C2(I,J), J = 1, NG )      IP 7070
0043      520    CONTINUE      IP 7080
0044      702    CONTINUE      IP 7090
0045      GO TO 522      IP 7100
0046      521    CONTINUE      IP 7110
0047      IF ( NG .EQ. 0 ) GO TO 522      IP 7120
0048      J = NG      IP 7130
0049      IF ( NPRT.EQ. 0 ) GO TO 703
0050      DO 523 I = 1, N      IP 7150
0051      523    PRINT 13, C2(I,J)      IP 7160
0052      703    CONTINUE      IP 7170
0053      522    CONTINUE      IP 7180
0054 C----- LP2 HAS THE FORM OF MAX Z      IP 7190

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80/80 LIST

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CARD
0055 C----- -C1*X1 - C2*X2 ..... + Z = 0.0 IP 7200
0056 PRINT 593 IP 7210
0057 DO 526 J = 1, JJ IP 7220
0058 526 DA(I,J) = 0.0 IP 7230
0059 DO 527 J = 1, MN IP 7240
0060 527 DA(I,J) = VALX(J+N) IP 7250
0061 IF ( NPRT.EQ. 0 ) GO TO 704
0062 PRINT 13, ( DA(I, J), J = 1, JJ ) IP 7270
0063 704 CONTINUE IP 7280
0064 PRINT 594 IP 7290
0065 IF ( NPRT.EQ. 0 ) GO TO 705
0066 PRINT 13, ( PRDT(J), J = 1, N ) IP 7310
0067 705 CONTINUE IP 7320
0068 DO 540 I = 1, N IP 7330
0069 540 DA(I+1, JJ) = PRDT(I) IP 7340
0070 DO 550 I = 1, N IP 7350
0071 DO 550 J = 1, MN IP 7360
0072 550 DA(I+1, J) = C1(I, J) IP 7370
0073 DO 560 I = 1, N IP 7380
0074 DO 560 J = 1, N IP 7390
0075 560 DA(I+1, J+MN) = UM(I, J) IP 7400
0076 IF ( NG .LT. 2 ) GO TO 621 IP 7410
0077 DO 570 I = 1, N IP 7420
0078 DO 570 J = 1, NG IP 7430
0079 570 DA(I+1, J+MN+N) = C2(I, J) IP 7440
0080 621 CONTINUE IP 7450
0081 IF ( NG .EQ. 0 ) GO TO 622 IP 7460
0082 J = NG IP 7470
0083 IF ( NPRT.EQ. 0 ) GO TO 706
0084 DO 623 I = 1, N IP 7490
0085 623 PRINT 13, C2(I, J) IP 7500
0086 706 CONTINUE IP 7510
0087 622 CONTINUE IP 7520
0088 PRINT 580 IP 7530
0089 DO 590 I = 1, II IP 7540
0090 DO 590 J = 1, JJ IP 7550
0091 590 A(I, J) = DA(I, J) IP 7560
0092 RETURN IP 7570
0093 END IP 7580

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CARD
0001          SUBROUTINE LP2 (PRDT,A,I,II,JJ,L,XT,C1,C2,N,M,NG,ZVAL,VALX)      IP 4080
0002          DIMENSION A(65, 65), M(65), L(65), XT(65)
0003          DIMENSION          PRDT(45), VALX(65)
0004          DIMENSION C1(30,12), C2(30,25)
0005          COMMON MN, LIN, NNL, NPR
0006 C          ***** IP 4130
0007 C..... IP 4210
0008 C          REQUIRED SUBROUTINES IP 4220
0009 C          (1) SUBROUTINES CALA2 IP 4230
0010 C..... IP 4240
0011 C***** M A X ***** IP 210
0012          1  FORMAT (10I5) IP 4250
0013 C----4  FORMAT (7F10.4) IP 4260
0014          4  FORMAT (11F7.3) IP 4270
0015          5  FORMAT (15, E13.4 ) IP 4280
0016          6  FORMAT ( //' FEASIBLE ' ) IP 4290
0017          7  FORMAT ( ' VARIABLE VALUE ' ) IP 4300
0018          8  FORMAT ( //' OBJ. FUNCTION' , E13.4 / ) IP 4310
0019          13  FORMAT (4(5X, 8E15.4/)) IP 4320
0020          14  FORMAT (1X, 9E13.4 ) IP 4330
0021          100  FORMAT ( //' THE FINAL MATRIX ' ) IP 4340
0022          101  FORMAT ( / ' ITERATION OBJ. FUNCTION NEW BASIC VAR. ' ) IP 4350
0023          103  FORMAT ( //' FEASIBLE ' ) IP 4360
0024          105  FORMAT (1X, 14, 6X, E13.4, 10X, 14) IP 4370
0025          130  FORMAT (/' UNBOUNDED ***** '/') IP 4380
0026          150  FORMAT (///35X, ' ROW ' , 12/) IP 4390
0027          151  FORMAT ( ///5X, ' X(1)' / ) IP 4400
0028          444  FORMAT (16F5.3) IP 4410
0029 C----- IP 4420
0030          5555  FORMAT (/////10X, 'PROBLEM', 13/) IP 4430
0031          6666  FORMAT ( '1' ) IP 4440
0032          7777  FORMAT ( 16I5 ) IP 4450
0033          NPRT = 0 IP 4460
0034          II = N + 1 IP 4470
0035          JJ = M + 1 IP 4480
0036          PRINT 1, N, M IP 4490
0037          PRINT 1, II, JJ IP 4500
0038          III = II + 1 IP 4510
0039          DO 10 I = 1, III IP 4520
0040          W (I) = 0. IP 4530
0041          10  L (I) = 0 IP 4540
0042 C IP 4550
0043 C          READ IN THE ELEMENTS OF THE MATRIX ROW BY ROW IP 4560
0044 C IP 4570
0045          CALL          CALA2(          II, III, JJ, A, PRDT ,C1,C2,N,M,NG,VALX) IP 4580
0046          DO 581 J = 1, JJ IP 4590
0047          581  A(III, J) = 0.0 IP 4600
0048          IF ( NPRT .EQ. 0 ) GO TO 310 IP 4610
0049          DO 582 I = 1, II IP 4620
0050          582  PRINT 13, ( A(I,J), J = 1, JJ ) IP 4630
0051          583  PRINT 13, ( A(III, J), J = 1, JJ ) IP 4640
0052          310  CONTINUE IP 4650
0053 C IP 4660
0054 C          ..... READ IN THE SUBSCRIPT FOR THE SLACK VARIABLE ON ROW I IP 4670

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CARD
0055 C ..... WHERE I IS NOT EQUAL TO 1 AND III IP 4680
0056 C IP 4690
0057 402 DO 403 I = 2, II IP 4700
0058 403 L(II) = I + MN-1 IP 4710
0059 PRINT 1,( L(II), I = 2, II ) IP 4720
0060 C IP 4730
0061 C NEXT STATEMENT FOR INITIALIZATION IP 4740
0062 C IP 4750
0063 KKK = 0 IP 4760
0064 C IP 4770
0065 C THE NEXT STATEMENTS ARE TO LOOK FOR THE ROW AT WHICH THERE IP 4780
0066 C IS NO SLACK VARIABLE (NOT INCLUDING THE FIRST ROW IP 4790
0067 C IP 4800
0068 22 I = 1 IP 4810
0069 23 I = I + 1 IP 4820
0070 IF ( I - III ) 23, 40, 40 IP 4830
0071 24 IF ( L(II) ) 23, 25, 23 IP 4840
0072 C IP 4850
0073 C CALCULATE IP 4860
0074 C IP 4870
0075 C NEW LAST ROW = LAST ROW WITHOUT SLACK VARIABLE IP 4880
0076 C IP 4890
0077 25 DO 27 J = 1, JJ IP 4900
0078 IF ( A (I,J) ) 26, 27,26 IP 4910
0079 26 A(III,J)= A (III,J) - A (I,J) IP 4920
0080 27 CONTINUE IP 4930
0081 GO TO 23 IP 4940
0082 C IP 4950
0083 C NEXT STATEMENTS FOR SEARCHING FOR THE COLUMN AT WHICH THE IP 4960
0084 C MOST NEGATIVE ENTRY APPEARS EITHER IN THE FIRST (OBJECTIVE IP 4970
0085 C FUNCTION) OR LAST (FORM P) ROW IP 4980
0086 C IP 4990
0087 40 K = III IP 5000
0088 44 J = 0 IP 5010
0089 W (K) = 0. IP 5020
0090 L ( K ) = 0 IP 5030
0091 42 J = J + 1 IP 5040
0092 IF ( J - JJ ) 41, 45, 45 IP 5050
0093 41 IF ( A(K,J) ) 43, 42,42 IP 5060
0094 43 IF ( W (K) - A (K,J) ) 42,42,47 IP 5070
0095 47 W ( K ) = A (K,J) IP 5080
0096 L (K) = J IP 5090
0097 GO TO 42 IP 5100
0098 C ..... IP 5110
0099 C IP 5120
0100 C TEST FOR L(K). IF L(K) IS EQUAL TO ZERO, THAT IS, ALL THE IP 5130
0101 C ENTRIES EXCEPT THE EXTREME RIGHT ONE EITHER IN THE FIRST IP 5140
0102 C OR LAST ROW ARE POSITIVE, GO TO ST. 62 FOR FURTHER EXAM. IP 5150
0103 C IP 5160
0104 C ..... IP 5170
0105 45 IF ( L(K) ) 46,62,46 IP 5180
0106 C IP 5190
0107 C FIND OUT THE PIVOT COLUMN IP 5200
0108 C IP 5210

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CARD
0109 46 KJ = L(K) IP 5220
0110 C IP 5230
0111 C TEST EVERY ENTRY IN THE PIVOT COLUMN TO SEE IF IT IS IP 5240
0112 C POSITIVE OR NOT . IF IT IS, GO TO ST. 121 TO COMPUTE IP 5250
0113 C RATIO DEFINED IN THE LAST SECTION. IP 5260
0114 DO 120 I = 2, II IP 5270
0115 IF (A (I,KJ )) 120,120,121 IP 5280
0116 120 CONTINUE IP 5290
0117 C IP 5300
0118 C IF ALL THE ENTRIES IN THE PIVOT COLUMN ARE ZERO OR NEGATIVE IP 5310
0119 C NUMBERS, 'UNBOUNDED' IS GOING TO BE TYPED IP 5320
0120 C IP 5330
0121 PRINT 130 IP 5340
0122 GO TO 70 IP 5350
0123 C IP 5360
0124 C THE FOLLOWING STATEMENTS ARE FOR COMPUTING THE RATIO DEFINED IP 5370
0125 C IN SECTION 15.4, AND FOR DETERMINING THE LOCATION OF THE PIVOT IP 5380
0126 C IP 5390
0127 121 I = 1 IP 5400
0128 JK = 0 IP 5410
0129 50 I = I + 1 IP 5420
0130 IF (I - II ) 52, 52,56 IP 5430
0131 52 IF (A(I,KJ)) 50, 50, 51 IP 5440
0132 51 X = A(I,JJ)/A(I,KJ) IP 5450
0133 IF (JK) 55, 53, 55 IP 5460
0134 55 IF (X - XMIN) 53,50,50 IP 5470
0135 53 XMIN = X IP 5480
0136 JK = I IP 5490
0137 GO TO 50 IP 5500
0138 C IP 5510
0139 C THE NEXT STATEMENT INDICATES THE PIVOT ELEMENT BEFORE NORMALIZA IP 5520
0140 C IP 5530
0141 56 X = A(JK,KJ) IP 5540
0142 L (JK) = KJ IP 5550
0143 C IP 5560
0144 C NEXT STATEMENTS FOR CALCULATING THE NEW ROWS ABOVE THE PIVOT RO IP 5570
0145 C IP 5580
0146 DO 57 I = 1, III IP 5590
0147 57 W (I) = A (I,KJ) IP 5600
0148 IJ = JK - 1 IP 5610
0149 DO 59 I = 1, IJ IP 5620
0150 DO 59 J = 1, JJ IP 5630
0151 IF (A(JK,J)) 58, 59,58 IP 5640
0152 58 IF (W(I)) 580,59,580 IP 5650
0153 580 A(I,J) = A(I,J) - W(I)*(A(JK,J)/X) IP 5660
0154 59 CONTINUE IP 5670
0155 C ..... IP 5680
0156 C IP 5690
0157 C NEXT STATEMENTS FOR CALCULATING NEW ROWS BEFORE THE PIVOT ROW IP 5700
0158 C IP 5710
0159 C ..... IP 5720
0160 IJ = JK + 1 IP 5730
0161 DO 61 I = IJ, III IP 5740
0162 DO 61 J = 1, JJ IP 5750
    
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CARD
0163          IF ( A(JK,J) ) 60,61,60                      IP 5760
0164          60      IF ( W ( I ) ) 600,61,600              IP 5770
0165          600     A ( I,J ) = A ( I,J ) - W(I) * ( A(JK,J) / X ) IP 5780
0166          61      CONTINUE                               IP 5790
0167 C ..... IP 5800
0168 C ..... IP 5810
0169 C ..... IP 5820
0170 C          NEXT STATEMENTS FOR NORMALIZATION           IP 5830
0171 C ..... IP 5840
0172          DO 205 J = 1, JJ                                IP 5850
0173          205     A ( JK,J ) = A(JK,J) / X                IP 5860
0174          KKK = KKK + 1                                    IP 5870
0175          PRINT 105, KKK, A(K,JJ), L(JK)                 IP 5880
0176          IF ( NPRT .EQ. 0 ) GO TO 300                   IP 5890
0177          DO 200 I = 1, II                                IP 5900
0178          200     PRINT 13, ( A(I,J), J = 1, JJ )         IP 5910
0179          300     CONTINUE                               IP 5920
0180          GO TO 44                                        IP 5930
0181 C ..... IP 5940
0182 C ..... IP 5950
0183 C          NEXT STATEMENT FOR TESTING TO SEE IF IT IS THE FIRST ROW ON
0184 C          WHICH ALL THE ENTRIES ARE POSITIVE EXCEPT EXTREME RIGHT ONE
0185 C          IF IT IS, THAT MEANS, NO FURTHER IMPROVEMENT ON THE SOLUTION
0186 C          CAN BE MADE, GO TO ST. 70 AND THE ANSWER WILL BE TYPED OUT
0187 C ..... IP 6000
0188 C ..... IP 6010
0189          62      IF ( K - 1 ) 70, 70, 63                 IP 6020
0190          63      IJ = JJ - 1                             IP 6030
0191 C ..... IP 6040
0192 C          TEST TO SEE WHETHER ALL THE ELEMENTS ON THE LAST ROW (NOT
0193 C          INCLUDING THE EXTREME RIGHT ONE) ARE CLOSE TO ZERO. IP 6050
0194 C          IT IS DEFINED IN THE NEXT STATEMENTS THAT THE PROBLEM IS
0195 C          INFEASIBLE IF ONE (OR MORE) OF THEM IS LARGER THAN 0.0001 IP 6060
0196 C ..... IP 6070
0197          DO 65 J=1, IJ                                    IP 6080
0198          IF ( A(K,J) - .0001 ) 65, 65,66                IP 6090
0199          65      CONTINUE                               IP 6100
0200          PRINT 103                                       IP 6110
0201          PRINT 101                                       IP 6120
0202 C ..... IP 6130
0203 C          IF, AFTER ITERATION, ALL THE ELEMENTS IN THE LAST ROW HAVE
0204 C          BECOME POSITIVE BUT NEAR ZERO, DEFINI ALL THEM TO BE ZERO IP 6140
0205 C ..... IP 6150
0206          DO 140 J = I, JJ                                IP 6160
0207          140     A(III,J) = 0.                            IP 6170
0208 C ..... IP 6180
0209 C          IN CASE OF NONARTIFICIAL PROBLEMS, DEFINE K=1, AND GO TO ST. 44 IP 6190
0210 C          FOR SEARCHING GOR THE PIVOT COLUMN            IP 6200
0211 C ..... IP 6210
0212          K = 1                                           IP 6220
0213          KKK = 0                                         IP 6230
0214          GO TO 44                                       IP 6240
0215 C ..... IP 6250
0216 C ..... IP 6260
          ..... IP 6270
          ..... IP 6280
          ..... IP 6290

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APPENDIX B

PROBLEM LISTINGS

(1) PROBLEM 1

(2) PROBLEM 3

80/80 LIST

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CARD
 0001 SUBROUTINE FCNCR (X, F, FC, FX, G, GC, GX) IP 3650
 0002 C
 0003 C
 0004 C
 0005 C
 0006 C
 0007 C.....-----
 0008 C PROBLEM 1
 0009 C-----PROGRAMMED BY M.S. CHAN AT OKLAHOMA STATE UNIVERSITY,
 0010 C STILLWATER, OKLAHOMA, UNDER THE SUPERVISION OF DR. M.
 0011 C TOCMACEK IN 1971.
 0012 C-----PROBLEM 1-----BEAMS SPANNING IN ONE DIRECTION
 0013 C.....-----
 0014 C PROBLEM 1
 0015 C-----NOMENCLATURE
 0016 C LIN = NO. OF LINEAR CONSTRAINTS
 0017 C>NNL = NO. OF NONLINEAR CONSTRAINTS
 0018 C>MN = NO. OF VARIABLES
 0019 C>X(1) = SPACING OF BEAMS ORIENTED IN THE X-DIRECTION
 0020 C>(*X-BEAMS*)
 0021 C>X(2) = MOMENT OF INERTIA OF THE X-BEAM WITH RESPECT TO THE
 0022 C>HORIZONTAL PRINCIPAL AXIS
 0023 C>X(3) = WEB DEPTH-THICKNESS RATIO OF THE X-BEAM (WEB
 0024 C>SLENDERNES)
 0025 C>G(1) = CCNSTRANT ON FLEXURAL STRENGTH
 0026 C>G(2) = CONSTRAINT ON DEFLECTION
 0027 C>G(3) = CCNSTRANT ON WEB BUCKLING
 0028 C>BL(1)-(3) = LOWER BOUNDS FOR THE VARIABLES
 0029 C>UB(1)-(3) = UPPER BOUNDS FOR THE VARIABLES
 0030 C>P = UDL = UNIFORMLY DISTRIBUTED LOAD
 0031 C>SIGMA = ALLOWABLE BENDING STRESS
 0032 C>A = DIMENSION IN THE X-DIRECTION OF A GIVEN RECTANGULAR
 0033 C>AREA SPANNED BY BEAMS
 0034 C>B = DIMENSION IN THE Y-DIRECTION OF A GIVEN RECTANGULAR
 0035 C>AREA SPANNED BY BEAMS
 0036 C>XK1 = COEFFICIENT DEPENDING ON THE STRESS DISTRIBUTION
 0037 C>E = YOUNG'S MODULUS OF ELASTICITY
 0038 C>XMU = POISSON'S RATIO
 0039 C>PI = 3.1416
 0040 C>C1 = MOMENT COEFFICIENT FOR SIMPLE SUPPORTS AND UNIFORMLY
 0041 C>DISTRIBUTED LOAD
 0042 C>C2 = DEFLECTION COEFFICIENT FOR UNIFORMLY DISTRIBUTED
 0043 C>LOAD
 0044 C>C3 = COEFFICIENT FOR DEFLECTION SPECIFIED IN BUILDING
 0045 C>CODES
 0046 C>C4 = COEFFICIENT FOR CRITICAL STRESS
 0047 C>AX = TOTAL CROSS-SECTIONAL AREA OF X-BEAM
 0048 C>F = OBJECTIVE FUNCTION (VOLUME)
 0049 C>H = DEFLECTION
 0050 C>BMX = BENDING MOMENT PER UNIT WIDTH IN X-BEAM
 0051 C>SMX = SECTION MODULUS OF X-BEAM
 0052 C>RSTX = BENDING STRESS IN X-BEAM
 0053 C>WALL = ALLOWABLE DEFLECTION
 0054 C>FCRX = CRITICAL BUCKLING STRESS

80/80 LIST

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CARD
0055 C
0056 C
0057 C
0058 C
0059 C
0060 C
0061 C
0062 C ..... IP 3820
0063 C----- SUBROUTINE FCNGR COMPUTES VALUES OF OBJ FCN & CONSTRAINTS
0064 C          REQUIRED SUBROUTINES IP 3950
0065 C          NONE IP 3960
0066 C#..... IP 3970
0067 C#..... IP 3940
0068 COMMON MN, LIN, NNL, NPR
0069 DIMENSION X(12)
0070 DIMENSION BL(12), UB(12)
0071 DIMENSION B3X(12)
0072 DIMENSION FX(12), GC(25), GX(25,12),G(25),GCT(25)
0073 DIMENSION DX(12), DY(12), GSUM(12), GSUMX(12,12), XYHX(12)
0074 DIMENSION GXYHX(12, 12), GXYH(12)
0075 DIMENSION HX(12),XJX(12),YJX(12)
0076 DIMENSION BMXX(12),BMYX(12),BMXYX(12),XSIGX(12),YSIGX(12)
0077 DIMENSION TXYX(12),TERM1X(12),TERM2X(12),TERM3X(12)
0078 DIMENSION SHEARX(12),SIG1X(12),SIG2X(12)
0079 DIMENSION SIGDX(12)
0080 DIMENSION SMXX(12), BSTXX(12)
0081 801 FORMAT ( 5X, 2I5, 7E14.4 / 15X, 7E14.4 / 15X, 7E14.4 / ) IP 3980
0082 802 FORMAT ( 5X, 6E14.4 / ) IP 3990
0083 C IP 4000
0084 C----- PROB 1-1B A X B LOT FEB 3 / 71
0085 C----- PROBLEM 1
0086 LIN = 6
0087 NNL = 3
0088 MN = 3
0089 N = MN
0090 A = 720.0
0091 B = 720.0
0092 XK1 = 23.9
0093 E = 30000.0
0094 XMU = 0.3
0095 PI = 3.1416
0096 P = 0.001
0097 C1 = 1.0 / 8.0
0098 C2 = 5.0 / 384.0
0099 C3=1./360.
0100 SIGMA = 22.0
0101 C4={(XK1)*((PI**2)*E)/(12.*(1.-XMU**2))}
0102 B6=3.46*A
0103 B7 = C1 * P * A ** 2.0 / SIGMA
0104 B8 = ( C2 *P* A ** 3.0 ) / E
0105 BL(1) = 24.0
0106 BL(2) = 11.3 IP 6820
0107 BL(3) = 100.0
0108 UB(1) = 120.0

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80/80 LIST

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CARD
0109          UB(2) = 349000.0
0110          UB(3) = 350.0
0111 C----- DELETE IF-STATEMENTS WHEN USED IN RGM
0112 C----- START OF IF- STAT
0113          IF ( NPR .EQ. 0 ) GO TO 825
0114          PRINT 802, (X(I), I = 1, MN )                      IP 4200
0115      825  CONTINUE
0116 C
0117 C ..... TO AVOID NEGATIVITY SET                          IP 4290
0118 C ..... LOWER BOUND                                      IP 4300
0119 C                                                         IP 4310
0120 C                                                         IP 4320
0121 C                                                         IP 4330
0122 C                                                         IP 4350
0123          NSKBL = MN + 1
0124          NSKUB = MN + 1
0125      DO 820 K = 1, MN
0126          IF ( K .EQ. NSKBL ) GO TO 821
0127          IF ( X(K) .LT. BL(K) ) X(K) = BL(K)
0128      821  CONTINUE
0129          IF ( K .EQ. NSKUB ) GO TO 820
0130          IF ( X(K) .GT. UB(K) ) X(K) = UB(K)
0131      820  CONTINUE                                          IP 4550
0132          IF ( NPR .EQ. 0 ) GO TO 826
0133          PRINT 802, (X(I), I = 1, MN )                      IP 4560
0134      826  CONTINUE
0135 C----- END OF IF- STAT
0136 C
0137          AX = 3.46 * ( X(2) ** 0.5 ) * ( X(3) ** (-0.5) )
0138          SMX = 1.51 * ( X(2) ** 0.75 ) * ( X(3) ** (-0.25) )
0139      DO 204 K = 1, MN
0140          SMXX(K) = 0.0
0141          SMXX(2) = 0.75*1.51*(X(2)**(-0.25))*(X(3)**(-0.25))
0142          SMXX(3) = -0.25*1.51*(X(2)**0.75)*(X(3)**(-1.25))
0143      204  CONTINUE
0144 C -----OBJ FCN -----                                PROB 43
0145 C
0146          LF = 0
0147          F = +86 * ( B * X(1) ** ( -1 ) - 1.0 ) * ( X (2) ** 0.5 )
0148          * ( X (3) ** ( -0.5 ) )
0149      DO 200 K = 1, MN
0150          FX(K) = 0.0
0151          FX(1) = -86 * ( X (2) ** 0.5 ) * ( X (3) ** ( - 0.5 ) )
0152          * B * X (1) ** ( -2 )
0153          FX(2) = +0.5 * 86 * ( B * X(1) ** ( -1 ) - 1.0 ) * ( X(2)
0154          * * ( -0.5 ) ) * ( X(3) ** ( -0.5 ) )
0155          FX(3) = -0.5 * 86 * ( B * X(1) ** ( -1 ) - 1.0 ) *
0156          ( X(2) ** 0.5 ) * ( X(3) ** ( -1.5 ) )
0157 C ----- CONSTRAINTS -----                            PROB0047
0158 C
0159          DO 201 J = 1, NNL
0160          DO 201 K = 1, MN
0161      201  GX(J,K) = 0.0
0162          G(1) = 1.51 * ( X(2) ** 0.75 ) * ( X(3) ** (-0.25) )

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80/80 LIST

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CARD
0163      1          - B7 * X(1)
0164          GX(1,1) = - B7
0165          GX(1,2) = 0.75 * 1.51 * ( X(2) ** (-0.25) ) *
0166      1          ( X(3) ** (-0.25) )
0167          GX(1,3) = -0.25 * 1.51 * ( X(2) ** 0.75) * ( X(3) **
0168      1          (-1.25) )
0169          G(2) = C3 - B8 * X (1) * X(2) ** (-1.0 )
0170          WALL = C3 * A
0171          W = WALL - G(2)
0172          GX(2,1) = -B8 * X(2) **(-1.0)
0173          GX(2,2) = B8 * X(1) * X(2) ** (-2.0)
0174          GX(2,3) = 0.0
0175          BSTX = B7*X(1)*SIGMA/SMX
0176          G(3) = C4 * ( X(3) ** (-2.0))- BSTX
0177          SIGCR = G(3) + BSTX
0178      DO 202 K = 1,MN
0179          BSTXX(K) = 0.0
0180      IF ( SMXX(K) .EQ. 0.0) GO TO 203
0181          BSTXX(K) = SIGMA * B7 * X(1) * (-1.0) * ( SMX ** (-2.0) ) *
0182      1          SMXX(K)
0183      203      CONTINUE
0184          BSTXX(1) = BSTXX(K)
0185      1          + B7 * SIGMA/ SMX
0186          GX(3,K) = -BSTXX(K)
0187          GX(3,3) = -BSTXX(K) -2.0 * C4*( X(3) ** (-3.0))
0188      202      CONTINUE
0189          IF ( NPR .EQ. 0 ) GO TO 690
0190          PRINT 802,F
0191          PRINT 802, (FX(K), K=1, MN )
0192          PRINT 802, X(2), X(3), X(4), AX, SMX
0193          PRINT 802, SIGMA, BSTX
0194          PRINT 802, WALL, W
0195          PRINT 802, SIGCR, BSTX
0196          DO 688 J = 1, NNL
0197      688          PRINT 802, G(J)
0198          DO 689 J = 1, NNL
0199      689          PRINT 802, ( GX(J,K), K = 1, MN )
0200          PRINT 802, FC
0201          PRINT 802, F
0202      690          CONTINUE
0203          IF ( NNL .NE. 0 ) GO TO 890
0204          G(1) = 0.0
0205          GC(1) = 0.0
0206          GX(1,1) = 0.0
0207          GX(1,2) = 0.0
0208          GO TO 880
0209      890          CONTINUE
0210          IF ( NNL .NE. 1 ) GO TO 870
0211          J = NNL
0212          GCT(J) = 0.0
0213          DO 670 K = 1, MN
0214      670          GCT(J) = GCT(J) + GX(J,K) * X(K)
0215          GC(J) = G(J) - GCT(J)
0216          GO TO 880

```

IP 6220

IP 6290

IP 6300

IP 6310

IP 6320

IP 6630

IP 6330

IP 6340

IP 6350

IP 6360

IP 6370

IP 6380

IP 6390

IP 6400

IP 6410

IP 6420

IP 6430

IP 6440

IP 6450

IP 6460

IP 6470

80/80 LIST

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CARD
0001          SUBROUTINE LFCN ( GLX, GLC, BL, UB )
0002          COMMON MN, LIN, NNL, NPR
0003          DIMENSION GLX(25,12), GLC(25)
0004          DIMENSION BL(12), UB(12)
0005 C..... IP 6730
0006 C          REQUIRED SUBROUTINES IP 6740
0007 C          MCNE IP 6750
0008 C..... IP 6760
0009 C          PROBLEM 1
0010 C..... IP 6760
0011 C          IP 6770
0012 C          IP 6800
0013          BL(1) = 24.0
0014          BL(2) = 11.3
0015          BL(3) = 100.0
0016          UB(1) = 120.0
0017          UB(2) = 349000.0
0018          UB(3) = 350.0
0019          NBL = 3
0020          NUB = 3
0021          DO 401 K = 1, NBL IP 6950
0022 401          GLC(K) = -BL(K) IP 6960
0023          DO 402 K = 1, NUB IP 6970
0024 402          GLC(NBL + K) = UB(K) IP 6980
0025          DO 200 J = 1, LIN IP 6990
0026          DO 200 K = 1, MN IP 7000
0027 200          GLX(J,K) = 0.0 IP 7010
0028          DO 403 K = 1, NBL IP 7020
0029 403          GLX(K, K) = +1.0 IP 7030
0030          DO 404 K = 1, NUB IP 7040
0031 404          GLX(NBL + K, K) = -1.0 IP 7050
0032          RETURN IP 7060
0033          END IP 7070

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80/80 LIST

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CARD
 0001 SUBROUTINE FCNGR (X, F, FC, FX, G, GC, GX) FGR 280
 0002 C
 0003 C
 0004 C
 0005 C
 0006 C
 0007 C.....-----
 0008 C PROBLEM 3
 0009 C-----PROGRAMMED BY M.S. CHAN AT OKLAHOMA STATE UNIVERSITY,
 0010 C STILLWATER, OKLAHOMA, UNDER THE SUPERVISION OF DR. H.
 0011 C TOCHACEK IN 1971.
 0012 C PROBLEM 3-----A GRID WITH BEAMS SPANNING IN TWO ORTHOGONAL
 0013 C DIRECTIONS-----RIGID-CONNECTION.
 0014 C.....-----
 0015 C-----NOMENCLATURE
 0016 C LIN = NO. OF LINEAR CONSTRAINTS
 0017 C NNL = NO. OF NONLINEAR CONSTRAINTS
 0018 C MN = NO. OF VARIABLES
 0019 C X(1) = SPACING OF BEAMS ORIENTED IN THE Y-DIRECTION
 0020 C (*Y-BEAMS*)
 0021 C X(2) = MOMENT OF INERTIA OF THE X-BEAM WITH RESPECT TO THE
 0022 C HORIZONTAL PRINCIPAL AXIS
 0023 C X(3) = WEB DEPTH-THICKNESS RATIO OF THE X-BEAM (WEB
 0024 C SLENDERNESS)
 0025 C X(4) = SPACING OF BEAMS ORIENTED IN THE X-DIRECTION
 0026 C (*X-BEAMS*)
 0027 C X(5) = MOMENT OF INERTIA OF THE Y-BEAM WITH RESPECT TO THE
 0028 C HORIZONTAL PRINCIPAL AXIS
 0029 C X(6) = WEB DEPTH-THICKNESS RATIO OF THE Y-BEAM (WEB
 0030 C SLENDERNESS)
 0031 C G(1) = CCASTRAINT ON FLEXURAL STRENGTH IN X-DIRECTION
 0032 C G(2) = CONSTRAINT ON FLEXURAL STRENGTH IN Y-DIRECTION
 0033 C G(3) = CONSTRAINT ON DEFLECTION IN X-DIRECTION
 0034 C G(4) = CCONSTRAINT ON DEFLECTION IN Y-DIRECTION
 0035 C G(5) = CONSTRAINT ON WEB BUCKLING IN X-DIRECTION
 0036 C G(6) = CCONSTRAINT ON WEB BUCKLING IN Y-DIRECTION
 0037 C G(7) = CONSTRAINT ON FLEXURAL STRENGTH AT THE JOINT
 0038 C BL(1)-(6) = LOWER BOUNDS FOR THE VARIABLES
 0039 C UB(1)-(6) = UPPER BOUNDS FOR THE VARIABLES
 0040 C P = UDL = UNIFORMLY DISTRIBUTED LOAD
 0041 C SIGMA = ALLOWABLE BENDING STRESS
 0042 C A = DIMENSION IN THE X-DIRECTION OF A GIVEN RECTANGULAR
 0043 C AREA SPANNED BY BEAMS
 0044 C B = DIMENSION IN THE Y-DIRECTION OF A GIVEN RECTANGULAR
 0045 C AREA SPANNED BY BEAMS
 0046 C XK1 = COEFFICIENT DEPENDING ON THE STRESS DISTRIBUTION
 0047 C E = YOUNG'S MODULUS OF ELASTICITY
 0048 C XMU = POISSON'S RATIO
 0049 C PI = 3.1416
 0050 C C1 = MOMENT COEFFICIENT FOR SIMPLE SUPPORTS AND UNIFORMLY
 0051 C DISTRIBUTED LOAD
 0052 C C2 = DEFLECTION COEFFICIENT FOR UNIFORMLY DISTRIBUTED
 0053 C LOAD
 0054 C C3 = COEFFICIENT FOR DEFLECTION SPECIFIED IN BUILDING

80/80 LIST

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CARD
0055 C          CODES
0056 C          C4 = COEFFICIENT FOR CRITICAL STRESS
0057 C          B1 = RATIO OF FLANGE THICKNESS TO WEB THICKNESS
0058 C          B2 = RATIO OF FLANGE AREA TO WEB AREA
0059 C          XG = SHEAR MODULUS
0060 C          DX = UNIT FLEXURAL RIGIDITY OF X-BEAM
0061 C          DY = UNIT FLEXURAL RIGIDITY OF Y-BEAM
0062 C          AX = TOTAL CROSS-SECTIONAL AREA OF X-BEAM
0063 C          AY = TOTAL CROSS-SECTIONAL AREA OF Y-BEAM
0064 C          AWX = WEB CROSS-SECTIONAL AREA OF X-BEAM
0065 C          AWY = WEB CROSS-SECTIONAL AREA OF Y-BEAM
0066 C          DELX = WEB THICKNESS OF X-BEAM
0067 C          DELY = WEB THICKNESS OF Y-BEAM
0068 C          H = WEB DEPTH
0069 C          F = OBJECTIVE FUNCTION (VOLUME)
0070 C          AA = PARAMETERS ASSOCIATED WITH THE ASSUMED SOLUTION
0071 C          W = DEFLECTION
0072 C          WXX = 2ND PARTIAL DERIVATIVE OF DEFLECTION WITH RESPECT
0073 C          TO X
0074 C          BMX = BENDING MOMENT PER UNIT WIDTH IN X-BEAM
0075 C          WYY = 2ND PARTIAL DERIVATIVE OF DEFLECTION WITH RESPECT
0076 C          TO Y
0077 C          BMY = BENDING MOMENT PER UNIT WIDTH IN Y-BEAM
0078 C          WXY = 2ND MIXED PARTIAL DERIVATIVE OF DEFLECTION WITH
0079 C          RESPECT TO X AND Y
0080 C          BMXY = TORSIONAL MOMENT PER UNIT WIDTH IN X-BEAM
0081 C          SMX = SECTION MODULUS OF X-BEAM
0082 C          SMY = SECTION MODULUS OF Y-BEAM
0083 C          BSTX = BENDING STRESS IN X-BEAM
0084 C          BSTY = BENDING STRESS IN Y-BEAM
0085 C          XSIG = BENDING STRESS IN X-BEAM
0086 C          YSIG = BENDING STRESS IN Y-BEAM
0087 C          TXY = SHEARING STRESS
0088 C          WXA = ALLOWABLE DEFLECTION OF X-BEAM
0089 C          WYA = ALLOWABLE DEFLECTION OF Y-BEAM
0090 C          FCRX = CRITICAL BUCKLING STRESS IN X-BEAM
0091 C          FCRY = CRITICAL BUCKLING STRESS IN Y-BEAM
0092 C          SIGD = COMBINED STRESS AT A JOINT
0093 C.....
0094 C
0095 C
0096 C
0097 C
0098 C
0099 C.....
0100 C----- SUBROUTINE FCNCR COMPUTES VALUES OF OBJ FCN & CONSTRAINTS
0101 C          REQUIRED SUBROUTINES                                FGR 560
0102 C          NONE                                             FGR 570
0103 C          .....                                           FGR 540
0104 C          .....                                           FGR 550
0105 C----- PROBLEM 3
0106 C          .....                                           FGR 580
0107 C          COMMON MN, LIN, NNL, NPR                            FGR 290
0108 C          DIMENSION X(12)                                    FGR 300
    
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80/80 LIST

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CARD						
0109		DIMENSION	BL(12), UB(12)			FGR 310
0110		DIMENSION	FX(12), GC(25), GX(25,12), G(25), GCT(25)			FGR 320
0111		DIMENSION	DXX(8), DYX(8)			FGR 330
0112		DIMENSION	HX(8), XJX(8), YJX(8)			FGR 340
0113		DIMENSION	BMXX(8), BMYX(8), BMXYX(8), XSIGX(8), YSIGX(8)			FGR 350
0114		DIMENSION	TXYX(8), TERM1X(8), TERM2X(8), TERM3X(8)			FGR 360
0115		DIMENSION	SHEARX(8), SIG1X(8), SIG2X(8)			FGR 370
0116		DIMENSION	SIGDX(8)			FGR 380
0117		DIMENSION		FCRXX(8), FAXX(8)		FGR 390
0118		DIMENSION		FCRYX(8), FAYX(8)		FGR 400
0119		DIMENSION	WX(8), WSUMX(8), WXXX(8)			FGR 410
0120		DIMENSION		WSUMY(8), WYK(8), WXYX(8)		FGR 420
0121		DIMENSION	AA(5,5), D1(5,5), D2(5,5), D3(5,5), D4(5,5),			FGR 430
0122	1		WW(5,5), WXX(5,5), WYY(5,5), WXY(5,5), DAA(5,5)			FGR 440
0123		DIMENSION	AAX(5,5,8), WXX(5,5,8), DAA(5,5,8)			FGR 450
0124		DIMENSION	WXXX(5,5,8), WYYX(5,5,8), WXYX(5,5,8)			FGR 460
0125		DIMENSION	SMXX(8), SMYX(8)			FGR 470
0126		DIMENSION	8STXX(8), 8STYX(8), 8STYX(8)			FGR 480
0127		DIMENSION	WX3K(8), WYXK(8), QXK(8)			FGR 490
0128		DIMENSION	WXX3(5,5), WYYX(5,5)			FGR 500
0129		DIMENSION	WXX3K(5,5,8), WYYXK(5,5,8)			FGR 510
0130		DIMENSION	DELXK(8), DELYK(8)			
0131	801	FORMAT	(5X, 2I5, 7E14.4 / 15X, 7E14.4 / 15X, 7E14.4 /)			FGR 590
0132	802	FORMAT	(5X, 9E14.4 /)			FGR 600
0133	C-----		PROBLEM 302	WEB BUCKLING	APRIL 19 / 71	FGR 610
0134			LIN = 12			
0135			NNL = 7			FGR 630
0136			MN = 6			
0137			N = MN			FGR 650
0138		NTEMP	= N			FGR 660
0139		SFB	= 1.0			FGR 670
0140			P = 0.001			FGR 680
0141		YIELD ₀	= 36.0			FGR 690
0142			SIGMA = 22.0			FGR 700
0143		SIGSH	= 14.3			FGR 710
0144			A = 720.0			FGR 720
0145			B = 720.0			FGR 730
0146		XK1	= 23.9			
0147		K2	= 0			FGR 750
0148			E = 30000.0			FGR 760
0149		XMU	= 0.3			FGR 770
0150		PI	= 3.1416			FGR 780
0151			C1 = (16.0 * P / (PI ** 6.0)) * E / 24.0			FGR 790
0152		C2	= 360.0			FGR 800
0153			C4 = XK1 * (PI ** 2.0) * E /			FGR 810
0154	1		(12.0 * (1.0 - XMU ** 2.0))			FGR 820
0155			C5 = (P * A / 2.0) * SFB			FGR 830
0156			C6 = (P * B / 2.0) * SFB			FGR 840
0157			C7 = 16.0 * P / (PI ** 6.0)			FGR 850
0158		B1	= 1.59			FGR 860
0159			B2 = 0.54			
0160			B3 = (2.0 * B2 * (B1 ** 2.0) + 1.0) / 3.0			FGR 880
0161		XG	= 12000.0			FGR 890
0162			B4 = (16.0 * P / (PI ** 6.0)) * XG / 24.0			FGR 900

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CARD
0163          B5 = K2 * C4 /XK1
0164 C----- LOWER BOUNDS                                FGR 920
0165          BL(1) = 24.0                                  FGR 930
0166          BL(2) = 11.3                                  FGR 940
0167          BL(3) = 100.0                                 FGR 950
0168          BL(4) = BL(1)                                FGR 960
0169          BL(5) = BL(2)                                FGR 970
0170          BL(6) = BL(3)                                FGR 980
0171 C----- UPPER BOUNDS                                FGR1010
0172          UB(1) = 120.0                                 FGR1020
0173          UB(2) = 349000.0                             FGR1030
0174          UB(3) = 350.0                                 FGR1040
0175          UB(4) = UB(1)                                FGR1050
0176          UB(5) = UB(2)                                FGR1060
0177          UB(6) = UB(3)                                FGR1070
0178 C----- DELETE IF-STATEMENTS WHEN USED IN RGM      FGR1100
0179 C----- START OF IF- STAT                            FGR1110
0180          IF ( NPR .EQ. 0 ) GO TO 825                  FGR1120
0181          PRINT 802, (X(I), I = 1, MN )                FGR1130
0182          825 CONTINUE                                  FGR1140
0183 C                                                     FGR1150
0184 C ..... TO AVOID NEGATIVITY SET                      FGR1160
0185 C ..... LOWER BOUND                                  FGR1170
0186 C                                                     FGR1180
0187 C                                                     FGR1190
0188 C                                                     FGR1200
0189          NSKBL = MN + 1                                FGR1210
0190          NSKUB = MN + 1                                FGR1220
0191          DO 820 K = 1, MN                              FGR1230
0192             IF ( K .EQ. NSKBL ) GO TO 821              FGR1240
0193             IF ( X(K) .LT. BL(K) ) X(K) = BL(K)       FGR1250
0194          821 CONTINUE                                  FGR1260
0195             IF ( K .EQ. NSKUB ) GO TO 820              FGR1270
0196             IF ( X(K) .GT. UB(K) ) X(K) = UB(K)       FGR1280
0197          820 CONTINUE                                  FGR1290
0198             IF ( NPR .EQ. 0 ) GO TO 826                FGR1300
0199             PRINT 802, (X(I), I = 1, MN )              FGR1310
0200          826 CONTINUE                                  FGR1320
0201 C----- END OF IF- STAT                              FGR1330
0202             DX = E * X(2) / X(4)                       FGR1340
0203             DY = E * X(5) / X(1)                       FGR1350
0204          DO 672 K = 1, MN                              FGR1360
0205             DELX(K) = 0.0
0206             DELY(K) = 0.0
0207             DXX(K) = 0.0
0208          672             DYX(K) = 0.0
0209             DXX(2) = E / X(4)
0210             DXX(4) = -E * X(2) * X(4) ** (-2.0)
0211             DYX(1) = -E * X(5) * X(1) ** (-2.0)
0212             DYX(5) = E / X(1)
0213             AX = 3.46 *( X(2) ** 0.5 ) * ( X(3) ** (-0.5) )
0214             AY = 3.46 *( X(5) ** 0.5 ) * ( X(6) ** (-0.5) )
0215             AMX = AX / ( 2.0 * B2 + 1.0 )
0216             AMY = AY / ( 2.0 * B2 + 1.0 )

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CARD
0217          DELX = ( AWX / X(3) ) ** 0.5
0218          DELY = ( AWY / X(6) ) ** 0.5
0219 C----- ALTERNATELY
0220 C----- DELX = 1.29 * ( X(2) ** 0.25 ) * ( X(3) ** 0.75 )
0221 C----- DELY = 1.29 * ( X(5) ** 0.25 ) * ( X(6) ** 0.75 )
0222          DELXK(2) = 0.25 * 1.29 * ( X(2) ** (-0.75) ) *
0223          ( X(3) ** 0.75 )
0224          1 DELXK(3) = 0.75 * 1.29 * ( X(2) ** 0.25 ) *
0225          ( X(3) ** (-0.25) )
0226          DELYK(5) = 0.25 * 1.29 * ( X(5) ** (-0.75) ) *
0227          ( X(6) ** 0.75 )
0228          1 DELYK(6) = 0.75 * 1.29 * ( X(5) ** 0.25 ) *
0229          ( X(6) ** (-0.25) )
0230          XJ = B3 * X(3) * DELX **4.0
0231          YJ = B3 * X(6) * DELY **4.0
0232          H = 0.5 * XG* ( XJ / X(4) + YJ / X(1) )
0233          XJG = XJ * XG
0234          YJG = YJ * XG
0235          XEI = X(2) * E
0236          YEI = X(5) * E
0237          DO 501 K = 1,MN
0238          XJX(K) = 0.0
0239          YJX(K) = 0.0
0240          HX(K) = 0.0
0241          501 CONTINUE
0242          DO 1501 K = 1, MN
0243          XJX(K) = 4.0 * B3 * X(3) * ( DELX ** 3.0 ) * DELXK(K)
0244          YJX(K) = 4.0 * B3 * X(6) * ( DELY ** 3.0 ) * DELYK(K)
0245          1501 CONTINUE
0246          XJX(3) = B3 * DELX ** 4.0 +
0247          1 B3 * X(3) * 4.0 * ( DELX ** 3.0 ) * DELXK(3)
0248          YJX(6) = B3 * DELY ** 4.0 +
0249          1 B3 * X(6) * 4.0 * ( DELY ** 3.0 ) * DELYK(6)
0250          DO 502 K = 1,MN
0251          HX(K) = 0.5 * XG* ( XJX(K) / X(4) + YJX(K)/X(1) )
0252          502 CONTINUE
0253          HX(1) = 0.5 * XG* ( XJX(1) / X(4) + YJX(1) / X(1)-YJ
0254          1 * X(1) ** (-2.0) )
0255          HX(4) = 0.5 * XG* ( XJX(4) / X(4) -XJ * X(4) ** (-2.0)
0256          1 + YJX(4) / X(1) )
0257 C
0258 C ..... OBJ FCN .....
0259 C----- MIN F
0260 C
0261          LF = 0
0262          DO 671 K = 1, MN
0263          671 FX(K) = 0.0
0264          F = 3.46 * A * ( B / X(4) - 1.0 ) * ( X(2) ** 0.5)
0265          1 * ( X(3) ** (-0.5) ) + 3.46 * B * ( A / X(1) -
0266          2 1.0 ) * ( X(5) ** 0.5 ) * ( X(6) ** (-0.5) )
0267          FX(1) = - A * ( X(1) ** (-2.0) ) * 3.46 * B *
0268          1 ( X(5) ** 0.5 ) * ( X (6) ** (-0.5) )
0269          FX(2) = 0.5 * ( X(2) ** (-0.5) ) * 3.46 * A *
0270          1 ( B / X(4) -1.0 ) * ( X(3) ** (-0.5) )

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FGR1450
FGR1460
FGR1470
FGR1480
FGR1490
FGR1500
FGR1510
FGR1520
FGR1530
FGR1540

FGR1590
FGR1620
FGR1630
FGR1640
FGR1650
FGR1660
FGR1670
FGR1680
FGR1690
FGR1700
FGR1710
FGR1720
FGR1730
FGR1740
FGR1750
FGR1760
FGR1770
FGR1780
FGR1790
FGR1800
FGR1810

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CARD
0271          FX(3) = -0.5 * ( X(3) ** (-1.5) ) * 3.46 * A *          FGR1820
0272          1          ( B / X(4) - 1.0 ) * ( X(2) ** 0.5 )          FGR1830
0273          FX(4) = - B * ( X(4) ** (-2.0) ) * 3.46 * A * ( X(2)** FGR1840
0274          1          0.5 ) * ( X(3) ** (-0.5) )          FGR1850
0275          FX(5) = 0.5 * ( X(5) ** (-0.5) ) * 3.46 * B *          FGR1860
0276          1          ( A / X(1) - 1.0 ) * ( X(6) ** (-0.5) )          FGR1870
0277          FX(6) = -0.5 * ( X(6) ** (-1.5) ) * 3.46 * B *          FGR1880
0278          1          ( A / X(1) - 1.0 ) * ( X(5) ** 0.5 )          FGR1890
0279 C
0280 C ..... CONSTRAINTS ..... FGR1900
0281 C
0282          NT = 5 FGR1920
0283          MT = 5 FGR1930
0284          MI = 2 FGR1940
0285          NI = 2 FGR1950
0286          DO 810 M = 1 , MT , MI FGR1960
0287          DO 810 N = 1 , NT , NI FGR1970
0288          DO 810 K = 1, MN FGR1980
0289          D1(M,N) = 0.0 FGR1990
0290          D2(M,N) = 0.0 FGR2000
0291          D3(M,N) = 0.0 FGR2010
0292          D4(M,N) = 0.0 FGR2020
0293          DAA(M,N) = 0.0 FGR2030
0294          AA(M,N) = 0.0 FGR2040
0295          WW(M,N) = 0.0 FGR2050
0296          WWXX(M,N) = 0.0 FGR2060
0297          WWYY(M,N) = 0.0 FGR2070
0298          WWXY(M,N) = 0.0 FGR2080
0299          DAAX(M,N,K) = 0.0 FGR2090
0300          AAX(M,N,K) = 0.0 FGR2100
0301          WWX(M,N,K) = 0.0 FGR2110
0302          WWXX(M,N,K) = 0.0 FGR2120
0303          WWYK(M,N,K) = 0.0 FGR2130
0304          WWYX ( M,N,K ) = 0.0 FGR2140
0305          WWX3K(M,N,K) = 0.0 FGR2150
0306          D1 ( M,N ) = ( M / A ) ** 4.0 FGR2160
0307          D2 ( M,N ) = 2.0 * (( M * N ) / ( A * B )) ** 2.0 FGR2170
0308          D3 ( M,N ) = ( N / B ) ** 4.0 FGR2180
0309          D4 ( M,N ) = 16.0 / ( M * N * PI ** 6.0 ) FGR2190
0310          810 CONTINUE FGR2200
0311 C----- SIMPLE-SUPP MAR 27, 71 FGR2210
0312 C----- ORIGIN AT NW CORNER ( 0.0 , 0.0 ) FGR2220
0313 C----- AT CENTER ( A/2.0,B/2.0 ) FGR2230
0314 C----- ON X-BOUNDARY( A/2.0, 0.0 ) FGR2240
0315 C----- ON Y-BOUNDARY( 0.0 ,B/2.0 ) FGR2250
0316          XXT = A / 2.0 FGR2260
0317          YYT = B / 2.0 FGR2270
0318          XX = XXT FGR2280
0319          YY = YYT FGR2290
0320          W = 0.0 FGR2300
0321          DO 811 M = 1 , MT , MI FGR2310
0322          DO 811 N = 1 , NT , NI FGR2320
0323          DAA(M,N) = D1(M,N) * DX + D2(M,N) * H + D3(M,N) * DY FGR2330
0324          AA(M,N) = D4(M,N) * P/DAA(M,N) FGR2340

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CARD
0325          IF ( XX .EQ. 0.0 ) GO TO 531          FGR2360
0326          CX = XX * XX                          FGR2370
0327          GO TO 533                              FGR2380
0328  531          CX = 0.0                          FGR2390
0329  533          IF ( YY .EQ. 0.0 ) GO TO 532      FGR2400
0330          CY = YY * YY                          FGR2410
0331          GO TO 534                              FGR2420
0332  532          CY = 0.0                          FGR2430
0333  534          CONTINUE                          FGR2440
0334          CWX = SIN ( M * PI * XX / A ) * SIN ( N * PI * YY / B ) FGR2450
0335          WY(M,N) = AA(M,N) * CWX              FGR2460
0336          W = W + WY(M,N)                       FGR2470
0337  C----- PARTIALS                            FGR2480
0338          DO 859 K = 1,MN                        FGR2490
0339  859          WX(K) = 0.0                       FGR2500
0340          DO 851 K = 1,MN                        FGR2510
0341          DAA(M,N,K) = D1(M,N) * DX(K) + D2(M,N) * HY(K) + FGR2520
0342  1          D3(M,N) * DY(K)                   FGR2530
0343          AAX(M,N,K) = -D4(M,N) * P * (DAA(M,N) **(-2.0)) * FGR2540
0344  1          DAA(M,N,K)                       FGR2550
0345          WXY(M,N,K) = AAX(M,N,K) * CWX         FGR2560
0346          WX(K) = WX(K) + WXY(M,N,K)           FGR2570
0347          851 CONTINUE                          FGR2580
0348          811 CONTINUE                          FGR2590
0349  C----- BENDING MOMENT IN X, BMX           FGR2600
0350          XX = XXT                               FGR2610
0351          WSUM = 0.0                             FGR2620
0352          WXX = 0.0                             FGR2630
0353          WX3 = 0.0                              FGR2640
0354          DO 857 K = 1, MN                       FGR2650
0355          WSUMX(K) = 0.0                         FGR2660
0356          WXXX(K) = 0.0                         FGR2670
0357          WX3K(X) = 0.0                         FGR2680
0358  857          CONTINUE                          FGR2690
0359          DO 813 I = 1,3                          FGR2700
0360          XX = XXT                               FGR2710
0361          YY = YYT                               FGR2720
0362          IF(I-2) 814,815,816                   FGR2730
0363  814          YY = -X(4) + YY                 FGR2740
0364          GO TO 817                              FGR2750
0365  815          YY = YY                          FGR2760
0366          GO TO 817                              FGR2770
0367  816          YY = X(4) + YY                   FGR2780
0368  817          DO 812 M = 1, MT, MI             FGR2790
0369          DO 812 N = 1, NT, NI                   FGR2800
0370          IF ( XX .EQ. 0.0 ) GO TO 541          FGR2810
0371          CX = XX * XX                          FGR2820
0372          GO TO 543                              FGR2830
0373  541          CX = 0.0                          FGR2840
0374  543          IF ( YY .EQ. 0.0 ) GO TO 542      FGR2850
0375          CY = YY * YY                          FGR2860
0376          GO TO 544                              FGR2870
0377  542          CY = 0.0                          FGR2880
0378  544          CONTINUE                          FGR2890

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CARD
0379          CWWXX = - ( ( M * PI / A ) ** 2.0 ) * SIN ( M * PI * XX / A ) * FGR2900
0380          1      SIN ( N * PI * YY / B ) FGR2910
0381          WXXX(M,N) = AA(M,N) * CWWXX FGR2920
0382          CWWX3 = - ( ( M * PI / A ) ** 3.0 ) * COS ( M * PI * XX / A ) FGR2930
0383          1      * SIN ( N * PI * YY / B ) FGR2940
0384          WXX3(M,N) = AA(M,N) * CWWX3 FGR2950
0385          DO 852 K = 1, MN FGR2960
0386          WXXX(M,N,K) = AAX(M,N,K) * CWWXX FGR2970
0387          WXX3K(M,N,K) = AAX(M,N,K) * CWWX3 FGR2980
0388          852  CONTINUE FGR2990
0389          IF(I-2) 818,819,818 FGR3000
0390          818  WSUM = WSUM + WXXX(M,N) FGR3010
0391          DO 853 K = 1, MN FGR3020
0392          853  WSUMX(K) = WSUMX(K) + WXXX(M,N,K) FGR3030
0393          GO TO 1812 FGR3040
0394          819  WSUM = WSUM + 22.0 * WXXX(M,N) FGR3050
0395          WX3 = WX3 + WXX3(M,N) FGR3060
0396          DO 854 K = 1, MN FGR3070
0397          WXX3(K) = WXX3(K) + WXX3(M,N,K) FGR3080
0398          WSUMX(K) = WSUMX(K) + WXXX(M,N,K) * 22.0 FGR3090
0399          1812 CONTINUE FGR3100
0400          812 CONTINUE FGR3110
0401          WXX = WXX + WSUM FGR3120
0402          813 CONTINUE FGR3130
0403          BMX = -(E * X(2) / 24.0) * WXX FGR3140
0404          DO 856 K = 1, MN FGR3150
0405          856  BMXX(K) = -(E * X(2)/24.0) * WXXX(K) FGR3160
0406          BMXX(2) = -(E * X(2)/24.0) * WXXX(2) FGR3170
0407          1      - ( E / 24.0 ) * WXX FGR3180
0408          C-----GO TO 9000 FGR3190
0409          C----- BENDING MOMENT IN Y, BMY FGR3200
0410          YY = YYT FGR3210
0411          WSUM = 0.0 FGR3220
0412          WYY = 0.0 FGR3230
0413          WYYX = 0.0 FGR3240
0414          DO 867 K = 1, MN FGR3250
0415          WYYK(K) = 0.0 FGR3260
0416          WSUMY(K) = 0.0 FGR3270
0417          WYYXK(K) = 0.0 FGR3280
0418          867  CONTINUE FGR3290
0419          DO 833 I = 1,3 FGR3300
0420          XX = XXT FGR3310
0421          YY = YYT FGR3320
0422          IF(I-2) 834,835,836 FGR3330
0423          834  XX = - X(1) + XX FGR3340
0424          GO TO 837 FGR3350
0425          835  XX = XX FGR3360
0426          GO TO 837 FGR3370
0427          836  XX = X(1) + XX FGR3380
0428          837  DO 832 M = 1, MT, MI FGR3390
0429          DO 832 N = 1, NT, NI FGR3400
0430          IF ( XX .EQ. 0.0 ) GO TO 551 FGR3410
0431          CX = XX * XX FGR3420
0432          GO TO 553 FGR3430

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CARD
0433 551 CX = 0.0 FGR3440
0434 553 IF ( YY .EQ. 0.0 ) GO TO 552 FGR3450
0435 CY = YY * YY FGR3460
0436 GO TO 554 FGR3470
0437 552 CY = 0.0 FGR3480
0438 554 CONTINUE FGR3490
0439 CWYY = - ( ( N * PI / B ) ** 2.0 ) * SIN ( M * PI * XX / A ) * FGR3500
0440 1 SIN ( N * PI * YY / B ) FGR3510
0441 WWYY ( M,N ) = AA ( M,N ) * CWYY FGR3520
0442 CWYYX = - ( ( N * PI / B ) ** 2.0 ) * ( M * PI / A ) * FGR3530
0443 2 COS ( M * PI * XX / A ) * SIN ( N * PI * YY / B ) FGR3540
0444 WWYYX(M,N) = AA(M,N) * CWYYX FGR3550
0445 DO 862 K = 1, MN FGR3560
0446 WWYYK( M,N,K ) = AAX ( M,N,K ) * CWYY FGR3570
0447 862 WWYYXK(M,N,K) = AAX(M,N,K) * CWYYX FGR3580
0448 IF ( I-2 ) 838,839,838 FGR3590
0449 838 WSUM = WSUM + WWYY(M,N) FGR3600
0450 DO 863 K = 1, MN FGR3610
0451 863 WSUMY(K) = WSUMY(K) + WWYYK(M,N,K) FGR3620
0452 GO TO 1832 FGR3630
0453 839 WSUM = WSUM + 22.0 * WWYY(M,N) FGR3640
0454 WYYX = WYYX + WWYYX(M,N) FGR3650
0455 DO 864 K = 1, MN FGR3660
0456 WYYXK(K) = WYYXK(K) + WWYYXK(M,N,K) FGR3670
0457 864 WSUMY(K) = WSUMY(K) + WWYYK(M,N,K) * 22.0 FGR3680
0458 1832 CONTINUE FGR3690
0459 832 CONTINUE FGR3700
0460 WYY = WYY + WSUM FGR3710
0461 DO 865 K = 1, MN FGR3720
0462 WYYK(K) = WYYK(K) + WSUMY(K) FGR3730
0463 865 CONTINUE FGR3740
0464 833 CONTINUE FGR3750
0465 BMY = -(E * X(5) / 24.0) * WYY FGR3760
0466 DO 866 K = 1, MN FGR3770
0467 866 BMYX(K) = -(E * X(5) / 24.0) * WYYK(K) FGR3780
0468 BMYX(5) = -(E * X(5) / 24.0) * WYYK(5) FGR3790
0469 1 - ( E / 24.0 ) * WYY FGR3800
0470 C----- TORSIONAL MOMENT, BMXY FGR3810
0471 XX = XXT FGR3820
0472 WSUM = 0.0 FGR3830
0473 WXY = 0.0 FGR3840
0474 DO 877 K = 1, MN FGR3850
0475 WXYX(K) = 0.0 FGR3860
0476 WSUMX(K) = 0.0 FGR3870
0477 877 CONTINUE FGR3880
0478 DO 843 I = 1,3 FGR3890
0479 XX = XXT FGR3900
0480 YY = YYT FGR3910
0481 IF(I-2) 844,845,846 FGR3920
0482 844 YY = X(4) + YY FGR3930
0483 GO TO 847 FGR3940
0484 845 YY = YY FGR3950
0485 GO TO 847 FGR3960
0486 846 YY = X(4) + YY FGR3970

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80/80 LIST

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12345678901234567890123456789012345678901234567890123456789012345678901234567890
CARD
0487      847      DO 842 M = 1 , MT , MI                      FGR3980
0488      DO 842 N = 1 , NT , NI                      FGR3990
0489      IF ( XX .EQ. 0.0 ) GO TO 571                FGR4000
0490      CX = XX * XX * XX                            FGR4010
0491      GO TO 573                                     FGR4020
0492      571      CX = 0.0                             FGR4030
0493      573      IF ( YY .EQ. 0.0 ) GO TO 572        FGR4040
0494      CY = YY * YY * YY                            FGR4050
0495      GO TO 574                                     FGR4060
0496      572      CY = 0.0                             FGR4070
0497      574      CONTINUE                             FGR4080
0498      CWMXY = ( M * PI / A ) * ( N * PI / B ) *    FGR4090
0499      1      COS ( M * PI * XX / A ) * COS ( N * PI * YY / B ) FGR4100
0500      DO 872 K = 1, MN                             FGR4110
0501      IF ( XX .EQ. 0.0 ) GO TO 561                FGR4120
0502      CX = XX * XX                                  FGR4130
0503      GO TO 563                                     FGR4140
0504      561      CX = 0.0                             FGR4150
0505      563      IF ( YY .EQ. 0.0 ) GO TO 562        FGR4160
0506      CY = YY * YY * YY                            FGR4170
0507      GO TO 564                                     FGR4180
0508      872      WXYX( M,N,K ) = AAX( M,N,K ) * CWMXY FGR4190
0509      562      CY = 0.0                             FGR4200
0510      564      CONTINUE                             FGR4210
0511      IF(I-2) 848,849,848                          FGR4220
0512      848      WSUM = WSUM + WXYX(M,N)             FGR4230
0513      DO 873 K = 1, MN                             FGR4240
0514      873      WSUMX(K) = WSUMX(K) + WXYX(M,N,K)   FGR4250
0515      GO TO 842                                     FGR4260
0516      849      WSUM = WSUM + 22.0*WXY(M,N)         FGR4270
0517      DO 874 K = 1, MN                             FGR4280
0518      874      WSUMX(K) = WSUMX(K) + WXYX(M,N,K) * 22.0 FGR4290
0519      842 CONTINUE                                 FGR4300
0520      WXY = WXY + WSUM                              FGR4310
0521      DO 875 K = 1, MN                             FGR4320
0522      875      WXYX(K) = WXYX(K) + WSUMX(K)        FGR4330
0523      843 CONTINUE                                 FGR4340
0524      BMXY = (XJG / 24.0 ) * WXY                   FGR4350
0525      DO 876 K = 1, MN                             FGR4360
0526      876 8MXYX(K) = (XJG/24.0) * WXYX(K)         FGR4370
0527      1      +(XG*XJX(K) / 24.0 ) * WXY           FGR4380
0528      QX = DX * WX3 + H * WYXX                    FGR4390
0529      DO 3100 K = 1, MN                            FGR4400
0530      3100 1      QXX(K) = DXX(K) * WX3 + DX * WX3K(K) + HX(K) * WYYX FGR4410
0531      1      + H * WYXX(K)                         FGR4420
0532      C----- N = NTEMP                            FGR4430
0533      SMX = 1.51 * ( X(2) ** 0.75 ) * ( X(3) ** (-0.25) ) FGR4460
0534      SMY = 1.51 * ( X(5) ** 0.75 ) * ( X(6) ** (-0.25) ) FGR4470
0535      WXA = A / C2                                  FGR4480
0536      WYA = B / C2                                  FGR4490
0537      FCRX = 0.0                                    FGR4500
0538      FCRY = 0.0                                    FGR4510
0539      FAX = 0.0                                     FGR4520
0540      FAY = 0.0                                     FGR4530

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80/80 LIST

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1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0541          BMX = ABS(BMX)                      FGR4540
0542          BMY = ABS(BMY)                      FGR4550
0543          BMXY = ABS(BMXY)                    FGR4560
0544          BSTX = BMX * SMX ** (-1.0)          FGR4600
0545          BSTY = BMY * SMY ** (-1.0)          FGR4610
0546          XSIG = BSTX                         FGR4630
0547          YSIG = BSTY                         FGR4640
0548          TXY = BMXY * DELX * XJ ** (-1.0)
0549          TERM1 = 0.5 * (XSIG+ YSIG)
0550          TERM2 = (0.5 * (XSIG - YSIG)) * (0.5 * (XSIG - YSIG))
0551          TERM3 = TXY **2.0
0552          SHEAR = SQRT(TERM2 + TERM3)
0553          SIG1 = TERM1 + SHEAR
0554          SIG2 = (TERM1 - SHEAR)
0555          SIGD = SQRT(SIG1**2.0-SIG1*SIG2+SIG2**2.0)
0556          FCRX = C4 * X(3) ** (-2.0)
0557          FAX =BMX
0558          FCRY = C4 * X(6) ** (-2.0)
0559          FAY = BMX
0560  C----- NONLINEAR CONSTRAINTS
0561  C----- G1 TO G6 > 0.0
0562          G(1) = SIGMA - BSTX
0563          G(2) = SIGMA - BSTY
0564          G(3) = WXA - W
0565          G(4) = WYA - W
0566          G(5) = FCRX - BSTX
0567          G(6) = FCRY - BSTY
0568          G(7) = SIGMA - SIGD
0569          DO 791 J = 1,>NNL
0570          DO 791 K = 1, MN
0571  791      GX(J,K) = 0.0
0572          DO 891 K = 1, MN
0573          FCRXX(K) = 0.0
0574          SMXX(K) = 0.0
0575          SMXX(2) = 0.75 *1.51 * ( X(2) ** (-0.25) ) *
0576          ( X(3) ** (-0.25) )
0577          SMXX(3) = -0.25 *1.51 * ( X(2) **0.75 ) *
0578          ( X(3) ** (-1.25) )
0579          FCRXX(3) = - 2.0 * C4 * X(3) ** (-3.0)
0580          FAXX(K) = BMXX(K)
0581          FCRYX(K) = 0.0
0582          SMYX(K) = 0.0
0583          SMYX(5) = 0.75 * 1.51 * ( X(5) ** (-0.25) ) *
0584          ( X(6) ** (-0.25) )
0585          SMYX(3) = -0.25 *1.51 * ( X(5) **0.75 ) *
0586          ( X(6) ** (-1.25) )
0587          FCRYX(6) = - 2.0 * C4 * X(6) ** (-3.0)
0588          FAYX(K) = BMYX(K)
0589  891      CONTINUE
0590          DO 520 K = 1,MN
0591          BSTXX(K) = BMXX(K) * SMX ** (-1.0)
0592          - BMX * ( SMX ** (-2.0) ) * SMXX(K)
0593          BSTYX(K) = BMYX(K) * SMY ** (-1.0)
0594          - BMY * ( SMY ** (-2.0) ) * SMYX(K)

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1234567890123456789012345678901234567890123456789012345678901234567890
CARD
0595          TXYX(K) = BMXYX(K) * DELX * XJ ** (-1.0) +
0596          1          BMXY * DELX(K) * XJ ** (-1.0) -
0597          2          BMXY * DELX * ( XJ ** (-2.0) ) * XJX(K)
0598          520      CONTINUE
0599          DO 3000 K = 1,MN
0600          XSIGX(K) = BSTXX(K)
0601          YSIGX(K) = BSTYX(K)
0602          TERM1X(K) = 0.5 * (XSIGX(K) + YSIGX(K))
0603          TERP2X(K) = 2.0 * (0.5 * (XSIG - YSIG)) * 0.5 *
0604          1          (XSIGX(K) - YSIGX(K))
0605          TERM3X(K) = 2.0 * TXY * TXYX(K)
0606          SHEARX(K) = 0.5 * (TERM2 + TERM3) * (TERM2X(K) +
0607          1          TERM3X(K))
0608          SIG1X(K) = TERM1X(K) + SHEARX(K)
0609          SIG2X(K) = TERM1X(K) - SHEARX(K)
0610          SIGDX(K) = 0.5 * ((SIG1**2.0 - SIG1*SIG2 + SIG2**2.0)**(-0.5))
0611          1          * ( 2.0 * SIG1 * SIG1X(K) - SIG2 )
0612          C----- GRADIENTS
0613          GX(1,K) = -BSTXX(K)
0614          GX(2,K) = -BSTYX(K)
0615          GX(3,K) = - WX(K)
0616          GX(4,K) = - WX(K)
0617          GX(5,K) = FCRXX(K) - BSTXX(K)
0618          GX(6,K) = FCRYX(K) - BSTYX(K)
0619          GX(7,K) = - SIGDX(K)
0620          3000      CONTINUE
0621          C9000 CONTINUE
0622          N = NTEMP
0623          C----- CUT OFF HERE WHEN USED IN RGM
0624          IF ( NPR .EQ. 0 ) GO TO 690
0625          DO 881 M = 1, MT , MI
0626          881      PRINT 802, (D1(M,N), N = 1, NT , NI )
0627          DO 882 M = 1, MT , MI
0628          882      PRINT 802, (D2(M,N), N = 1, NT , NI )
0629          DO 883 M = 1, MT , MI
0630          883      PRINT 802, (D3(M,N), N = 1, NT , NI )
0631          DO 884 M = 1, MT , MI
0632          884      PRINT 802, (D4(M,N), N = 1, NT , NI )
0633          DO 899 M = 1, MT, MI
0634          899      PRINT 802, (DAA(M,N), N = 1, NT , NI )
0635          DO 892 M = 1, MT, MI
0636          892      PRINT 802, (AA(M,N), N = 1, NT , NI )
0637          PRINT 802,F
0638          PRINT 802, (FX(K), K=1, MN )
0639          PRINT 802, X(2), X(3), DELX, AX, SMX
0640          PRINT 802, X(5), X(6), DELY, AY, SMY
0641          PRINT 802, XE1, YE1
0642          PRINT 802, XJ, YJ
0643          PRINT 802, XJG, YJG, H
0644          PRINT 802, BMX, BMY, BMXY
0645          PRINT 802, DX, WX3, H, WYXX, QX
0646          C-----RETURN
0647          PRINT 802, XSIG, YSIG, TXY
0648          PRINT 802, TERM1, TERM2, TERM3
FGR5190
FGR5340
FGR5350
FGR5360
FGR5370
FGR5380
FGR5390
FGR5400
FGR5410
FGR5420
FGR5430
FGR5440
FGR5450
FGR5460
FGR5470
FGR5480
FGR5490
FGR5500
FGR5510
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FGR5620
FGR5630
FGR5640
FGR5650
FGR5660
FGR5670
FGR5680
FGR5690
FGR5700
FGR5710
FGR5720
FGR5730
FGR5760
FGR5770
FGR5780
FGR5790
FGR5800
FGR5810
FGR5820
FGR5830

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80/80 LIST

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CARD			
0649		PRINT 802, SHEAR, SIG1, SIG2	FGR5840
0650		PRINT 802, SIGMA, BSTX	FGR5850
0651		PRINT 802, SIGMA, BSTY	FGR5860
0652		PRINT 802, WXA, W	FGR5870
0653		PRINT 802, WYA, W	FGR5880
0654		PRINT 802, FCRX, BSTX	FGR5890
0655		PRINT 802, FCRY, BSTY	FGR5900
0656		PRINT 802, SIGMA, SIGD	FGR5910
0657		PRINT 802, SIGMA, BSTX, WXA, W, FCRX, BSTX	FGR5920
0658		PRINT 802, SIGMA, BSTY, WYA, W, FCRY, BSTY, SIGMA, SIGD	FGR5930
0659		DO 688 J = 1, NNL	FGR5940
0660	688	PRINT 802, G(J)	FGR5950
0661		DO 689 J = 1, NNL	FGR5960
0662	689	PRINT 802, (GX(J,K), K = 1, MN)	FGR5970
0663	690	CONTINUE	FGR5980
0664		IF (NNL .NE. 0) GO TO 890	FGR5990
0665		G(1) = 0.0	FGR6000
0666		GC(1) = 0.0	FGR6010
0667		GX(1,1) = 0.0	FGR6020
0668		GX(1,2) = 0.0	FGR6030
0669		GO TO 880	FGR6040
0670	890	CONTINUE	FGR6050
0671		IF (NNL .NE. 1) GO TO 870	FGR6060
0672		J = NNL	FGR6070
0673		GCT(J) = 0.0	FGR6080
0674		DO 670 K = 1, MN	FGR6090
0675	670	GCT(J) = GCT(J) + GX(J,K) * X(K)	FGR6100
0676		GC(J) = G(J) - GCT(J)	FGR6110
0677		GO TO 880	FGR6120
0678	870	CONTINUE	FGR6130
0679		DO 772 J = 1, NNL	FGR6140
0680	772	GCT(J) = 0.0	FGR6150
0681		DO 770 J = 1, NNL	FGR6160
0682		DO 770 K = 1, MN	FGR6170
0683	770	GCT(J) = GCT(J) + GX(J,K) * X(K)	FGR6180
0684		DO 771 J = 1, NNL	FGR6190
0685	771	GC(J) = G(J) - GCT(J)	FGR6200
0686	880	CONTINUE	FGR6210
0687		SUM = 0.0	FGR6220
0688		DO 700 K = 1, MN	FGR6230
0689		SUM = SUM + FX(K) * X(K)	FGR6240
0690	700	CONTINUE	FGR6250
0691		IF (LF .EQ. 1) GO TO 760	FGR6260
0692		FC = F - SUM	FGR6270
0693		IF (NPR .EQ. 0) GO TO 827	FGR6280
0694		PRINT 802, FC, F, SUM	FGR6290
0695	827	CONTINUE	FGR6300
0696		GO TO 761	FGR6310
0697	760	FC = 0.0	FGR6320
0698	761	CONTINUE	FGR6330
0699		NPR = 0	FGR6340
0700		RETURN	FGR6350
0701		END	FGR6360

APPENDIX C

DETERMINATION OF $C = C_{11}$ FROM DEFLECTION

CONSIDERATION FOR CASE (2)--FORMULA

(3. 46) WITH THE EQUALITY SIGN

From deflection consideration, C can be determined as follows:

Letting

$$C_5 = \frac{C_2}{C_1} \quad (C1)$$

and noting Formula (3. 16), Formula (3. 21) can be rewritten as

$$C_5 \frac{MA^2}{EI} \leq C_3 A, \quad (C2)$$

$$\frac{M}{I} \leq \frac{C_3 E}{C_5 A} \cdot \quad (C3)$$

Let

$$C_9 = \frac{C_3 E}{C_5 A}$$

then

$$\frac{M}{I} \leq C_9 \cdot \quad (C4)$$

From strength consideration, Formula (3. 35) leads to

$$\frac{M}{I} = \frac{2\sigma_{all}^{1.25}}{0.5^{0.5} C_4^{0.25}} \frac{C^{1.25}}{A_t^{0.5}} \cdot \quad (C5)$$

Let

$$C_{10} = \frac{2\sigma_{\text{all}}^{1.25}}{0.5^{0.5} C_4^{0.25}} \quad (\text{C6})$$

then

$$\frac{M}{I} = C_{10} \frac{C^{1.25}}{A_t^{0.5}} \quad (\text{C7})$$

Combining Formulas (C7) and (C4),

$$C_{10} \frac{C^{1.25}}{A_t^{0.5}} \leq C_9 \quad (\text{C8})$$

$$C \leq \left(\frac{C_9}{C_{10}}\right)^{0.8} A_t^{0.4} \quad (\text{C9})$$

Let

$$C_6 = \left(\frac{C_9}{C_{10}}\right)^{0.8} \quad (\text{C10})$$

then

$$C \leq C_6 A_t^{0.4} \quad (\text{C11})$$

A_t can be expressed from strength condition (Formula 3.35),

$$\frac{M}{C \sigma_{\text{all}}} = S = 0.235 (\lambda A_t^3)^{0.5} \quad (\text{C12})$$

$$\left(\frac{M}{0.235 \sigma_{\text{all}}}\right)^4 = C^4 \lambda^2 A_t^6 \quad (\text{C13})$$

Let

$$C_7 = \left(\frac{M}{0.235 \sigma_{\text{all}}}\right)^4 \quad (\text{C14})$$

then

$$C_7 = C^4 \lambda^2 A_t^6 \quad (\text{C15})$$

Using λ from Formula (3.49),

$$C_7 = C^3 \frac{C_4}{\sigma_{\text{all}}} A_t^6 \quad (\text{C16})$$

or

$$A_t = \frac{1}{C^{0.5}} \left(\frac{C_7 \sigma_{\text{all}}}{C_4} \right)^{\frac{1}{6}} . \quad (\text{C17})$$

Let

$$C_8 = \left(\frac{C_7 \sigma_{\text{all}}}{C_4} \right)^{\frac{1}{6}} \quad (\text{C18})$$

then

$$A_t = \frac{1}{C^{0.5}} C_8 . \quad (\text{C19})$$

Substituting Formula (C19) into Formula (C11),

$$C \leq C_6^{0.833} C_8^{0.333} . \quad (\text{C20})$$

Let

$$C_{11} = C_6^{0.833} C_8^{0.333} , \quad (\text{C21})$$

then

$$C \leq C_{11} . \quad (\text{C22})$$

Substitution results in C_{11} expressed by all constants, and

$$C_{11} = \left(\frac{C_1^{\frac{13}{18}} C_4^{\frac{1}{9}}}{A^{\frac{5}{9}} \sigma_{\text{all}}} \right) \left(\frac{0.354 E C_3}{C_2} \right)^{\frac{2}{3}} \left(\frac{\text{pb}}{0.00305} \right)^{\frac{1}{18}} \quad (\text{C23})$$

APPENDIX D

DETERMINATION OF C_{12} AND THE MOMENT OF INERTIA, I , FOR CASE (3)

Case (3) implies that G_1 and G_2 carry equality signs. Rewriting G_1 (Formula 3.26 with equality sign) with

$$S = \frac{2I}{h} \quad (D1)$$

instead, G_1 becomes

$$\frac{h C_1 p b A^2}{2I} = \sigma_{all} \quad (D2)$$

Dividing Formula (D2) by G_2 (Formula 3.27 with equality sign) and rearranging, then

$$\frac{h}{A} = \frac{2 C_2 \sigma_{all}}{C_1 C_3 E} \quad (D3)$$

Defining

$$C_{12} = \frac{2 C_2 \sigma_{all}}{C_1 C_3 E} \quad (D4)$$

then

$$h = C_{12} A \quad (D5)$$

Substituting Formula (D5) into Formula (D2) and rearranging, the moment of inertia

$$I = \frac{C_1 p b A^3 C_{12}}{2 \sigma_{all}} \quad (D6)$$

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