## SIMILITUDE INVESTIGATION OF VERTICAL

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### EXAGGERATION IN STRATIFIED

LAKE MODELS

By

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Bachelor of Science

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1972

Submitted to the Faculty of the Graduate College of the Oklahoma State University in partial fulfillment of the requirements for the Degree of MASTER OF SCIENCE July, 1973

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LAKE MODELS

Thesis Approved: McLaughlin Dennis K Thesis Adyi ser Dean of the Graduate College

## ACKNOWLEDGMENTS

The author wishes to express his appreciation to his major adviser, Dr. Dennis McLaughlin, for his guidance and assistance throughout this study. Appreciation is also expressed to Dr. Peter Moretti for his interest and assistance with this investigation. Part of my graduate study was financed by the Oklahoma Water Resources Research Institute.

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# NOMENCLATURE

đ	characteristic depth of lake model
F	Froude number, u <sup>2</sup> /gd
F	densimetric Froude number, u <sup>2</sup> /g'd
FR	Froude-Reynolds number, $(g^{\frac{1}{2}}d^{3/2})W'$
FAR	densimetric Froude-Reynolds number, $((g')^{\frac{1}{2}} d^{3/2})/\sqrt{2}$
g	gravitational constant
g'	effective gravitational constant, $g\Delta \rho / \rho$
H	depth
L	characteristic length
R	Reynolds number, UD/W
Ri	Richardson number, $(\rho u^2)/(g\Delta \rho H)$
t	characteristic time
u	characteristic velocity
U	velocity
U	densimetric velocity, (g'H) <sup>1</sup> 2
V*	nondimensional velocity, U/U <sub>i</sub>
У	vertical dimension
<b>Y</b> *	nondimensional depth, $D/D_t$
W*	nondimensional width, W/W <sub>t</sub>

Greek Letters

 $\Delta$  difference

 ${\cal U}$  dynamic viscosity of the fluid

- p density of the fluid
- ☐ shearing stress per unit area

# Subscripts

- A deep model 12 inch
- B shallow model 6 inch
- i inlet
- in interface

t total

### CHAPTER I

#### INTRODUCTION

## Problem Statement

The purpose of this experimental investigation was to determine the effect of vertical scale exaggeration in free surface hydraulic models. Specifically, this was to determine whether a hydraulic model could be scaled in a manner where the scale factor of the vertical direction would not be as small as for the horizontal direction.

The word model is used in the restricted sense of an attempt at a smaller scale simulation of a large and nearly always unique, natural or artificial hydraulic system. For this investigation the hydraulic system is taken to be an idealized stratified lake with a river channel inflow configuration.

In hydraulic systems such as lakes with relatively large horizontal lengths, the depths are usually quite small. When these systems are modeled by scaling these dimensions down with the same scaling parameter, this results in models that are very shallow, so that frictional forces dominate, resulting in an inaccurate model of the prototype. Therefore, exaggeration of the vertical scale to give an acceptable depth in the model has been utilized in various hydraulic studies, but this technique is not universally supported as being accurate.

The lack of universal acceptance of the concept of vertical scale exaggeration is the motivation for this experimental study.

In this investigation, flow patterns were established in a prototype model of a stratified lake and the flow patterns were simulated in another stratified lake model which had an exaggeration of the vertical scale. It was then attempted to relate the observed flow pattern and velocity profiles at selected locations in the prototype model to those at the same corresponding locations in the exaggerated model.

#### Background

The lack of universal acceptance of the vertical scale exaggeration concept stems from the fact that any modeling technique employed is a compromise at best. The relative importance placed on the interaction of the various pairs of forces in a fluid flow situation is the basis for the dispute.

The power of the method of models in solving problems in free boundary hydraulics has been demonstrated in thousands of successful studies. If these modeling techniques are examined in detail, it is found that where gravitational forces predominate and a fixed containing boundary is used, there is consistent agreement between the model and the prototype, so long as various well established precautions are taken in the design and use of the model. Where frictional forces have the same order of influence as the gravitational forces, success has also been met with, but to a lesser extent. In many cases involving large but

relatively shallow prototypes, success has been dependent on the method of exaggeration of the vertical scale in comparison with the horizontal scale of the model.

At present, the school of thought that backs the utilization of vertical scale exaggeration are mainly researchers in England and Australia. In open literature there are many studies contributed by the researchers in these countries that have proven to be quite successful. Most notable of these studies are those by Barr  $\sqrt{3}$ , 4, 5, 7 and 8 $\sqrt{7}$ , and the work by Price and Kendrick  $\sqrt{26}\sqrt{7}$ . Most of these model studies were made to simulate the effects of thermal discharge into rivers and tidal estuaries and the mechanism of exchange flow in estuaries. They have reported that due to the large horizontal dimensions involved, vertical scale exaggeration could be utilized very successfully with careful consideration of the effects of exaggeration.

The researchers that dispute the validity of exaggeration are mainly from France. Most of their investigations deal with model studies of large hydraulic systems, such as large estuaries and rivers, with regard to the overall redesign of these systems. Since they dispute the problem concept, their models are scaled on the basis of the natural model scaling parameter, i.e., all dimensions are scaled proportionally with regard to the actual hydraulic systems dimensions. This has resulted in models that are quite large, because of the need to have the vertical dimension large enough that frictional effects will not be inaccurate.

In the United States there have been reports by researchers

who follow both lines of reasoning. Fischer and Holly  $/ 13_7$  reported that the vertical scale exaggeration concept is not feasible, but they approached this argument with a one-dimensional analytical study. Miner, Hinley and Cayot  $/ 22_7$  have reported success with the use of the exaggeration concept in a comparison study between a model and a prototype which investigated thermal discharges. No reports appeared in the literature on the utilization of vertical scale exaggeration with stratified lake models as pursued in this report. The only studies related to this investigation dealt with the consideration of a homogenous body of water with an inflow of a different density.

#### Scope of the Present Study

To validate the concept of vertical scale exaggeration in stratified lake models, two idealized models were constructed for testing purposes. Specifically, the lake models were designed to allow the visualization of the distribution and mixing patterns resulting from dye traced inflows into various types of lake stratification.

The two lake models were constructed to have geometric similarity in all respects except for the vertical scale. One lake was considered as the prototype model with a natural scaling parameter and the other model had a vertical scale exaggeration of twice the natural scale.

The investigation is limited because only one type of vertical scale exaggeration was utilized, but with a large range of variation in flow patterns available. Along with the various

types of stratification that could be set up in the lake models, it was felt that this would lead to a determination of the critical parameters and their effect on the proper simulation between models.

To determine the validity of the scale exaggeration concept it was crucial to determine the velocity profiles for selected locations corresponding between the two models for the same basic flow situations. This was accomplished by testing one model with a specified flow configuration and recording the visual traces of the dye patterns. The same flow configuration was tested on the other model with the flow parameters adjusted suitably and again the flow patterns were recorded. The data was then corrected with regard to proper scaling and the velocity profiles for the specified locations compared.

To properly model a specified flow configuration in the exaggerated model it was necessary to develop a modeling technique that would adjust the various control parameters to yield a flow situation as developed in the prototype model. The development of the modeling theory and technique is presented in Chapter II of this report.

A detailed explanation of the experimental testing facilities that were constructed to determine the validity of the problem considered, is presented in Chapter III. Also presented are sections concerned with the testing procedures, the data collection systems, and the data reduction techniques.

Chapter IV gives the results of the experimental testing

along with the types of tests utilized. A detailed discussion of the results and the significance of the variance of the similitude parameters is presented in Chapter V. A summation of the work of this investigation and the major conclusions developed during this report is presented in Chapter VI.

### Relevance

This experimental investigation has application to the development of modeling techniques for stratified lake models. Validation of the vertical scale exaggeration concept for these types of models would enable researchers to design models that would be of laboratory size, whereas natural scale models would tend to be quite large and thereby would require a greater expenditure in terms of funds and space required for the same type of investigation. This would aid in the development of accurate models of lakes and reservoirs, in particular, the study of the fluid mechanics within these models. The use of stratified lake models is relatively a recent research development. This concept in lake modeling is a more realistic method in terms of developing an accurate model of a real lake situation, since most lakes are stratified to some extent. Most of the previous model studies in this area dealt primarily with a lake model of one density, i.e., a homogenous lake, and simulating various types of inflows by introduction of a fluid of another density.

From these model studies the many aspects of the lake's dynamic flow system can be determined with regard to various types of inflows. Proposed lake modification of designs could be

tested by the simulation of the dispersion and mixing patterns of different inflows. The results of this type of simulation could then be extrapolated to a real lake situation, thus enabling the best design to be selected without actual testing in the real lake.

The success of a lake or reservoir design could be enhanced by the use of lake models utilizing vertical scale exaggeration. These types of models could also be used in the development of lake or reservoir preservation programs and for water quality maintenance.

#### CHAPTER II

#### MODELING THEORY

This chapter presents the development of the modeling theory utilized in this investigation. Consideration is given to the various forces acting on the fluids in this problem and the development of similitude numbers which are representative of these forces.

#### **Previous** Research

Exchange flows in stratified systems have aroused spasmodic interest during the past forty years or so; O'Brien and Cherno  $\langle 23_7 \rangle$ , Yih  $\langle 31_7 \rangle$ , and Keulegan  $\langle 17_7 \rangle$  have described experimental studies, while Keulegan  $\langle 17_7 \rangle$  and Schijf and Schönfeld  $\langle 27_7 \rangle$  have given analytical approaches to this type of flow. It is one facet of the group pnenomena variously known as density currents, stratified flows, sub-surface flows or internal flows. The existence of parallels between sub-surface and free surface hydraulic occurrences has been stressed by various writers, notably Keulegan  $\langle 17_7 \rangle$  and more recently Harleman  $\langle 15_7 \rangle$ . It is, however, important to remember that in practical circumstances small density difference phenomena are normally observed as between miscible fluids; fresh and salt water or warmer and colder water, warmer

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and colder air, or between air and other gases. In the laboratory miscible liquids may be used, as in the studies mentioned above.

The utilization of scale exaggeration was reported in Koulegan's /17/ studies and also by Barr /7/ as being a workable concept so long as the conclusion of the effects of exaggeration were taken into account. Fischer and Holly /13/disagreed with the concept in their paper on dispersion studies.

#### Theory Development

To investigate the validity of vertical scale exaggeration, a development of the modeling theory must be accomplished to aid in the design of the testing facility and the control of the flow parameters. This section is concerned with the basic principles involved and the development of the similitude parameters relevant to modeling a stratified lake situation.

Fluid flows, in general, are controlled by several types of forces. Among these are forces due to local accelerations, gravity, fluid friction, rotation, and buoyancy of one part of the fluid relative to another. If a model of a fluid flow is truly to scale, the ratios of all these forces to each other in the model must be the same as the corresponding ratios of the much larger forces in the full-scale prototype. If this could be done then the properties of the flow would be precisely the same in the model as in the prototype, and extrapolation from one to the other could be confidently made. Unfortunately it is impossible to have every type of force in scale so that it is usual to select pairs of forces and to scale these correctly ignoring the others.

The selection of these pairs and thereby determination of the velocity, time, and density scales depends upon the purposes for which the model is made.

In this study two lake models were constructed for testing purposes. One of which, model A, is intended to simulate the other, model B. Model A is geometrically similar to model B except the vertical dimensions are exaggerated to twice the scale of model B. Therefore in comparing the two hydraulic models three types of similarity must be appropriately utilized.

The first type of similarity considered is that of geometry, where the ratios of corresponding linear dimensions of the compared systems should be equal. In this open lake study this is largely imposed by the accuracy with which the models were constructed. The next type of similarity is kinematic; this is where the ratios of the components of velocity at corresponding points of the two geometrically and kinematically similar systems are equal. The third similarity is dynamic, where the ratios of corresponding forces in the two geometrically and kinematically similar systems are equal.

It is necessary to develop the modeling laws applicable for the lake models which will yield similarity in these three areas. The approach taken in this study is that since the geometric similarity was imposed on the lake models by the design and construction methods, this factor will be constant throughout the experimental work. This leaves two types of similarity which must be met in order to acheive an accurate modeling between the lakes. The method employed here is to develop a modeling law

which will yield dynamic similarity between models. The modeling theory will be utilized in the development of the flow parameters for both models. These flow parameters will lead to a kinematic similarity which can be experimentally verified.

To achieve dynamic similarity between the model A and model B, the ratio of the inertial and the active forces must be the same. To obtain a model law intended to give dynamical similarity it is convient to compare corresponding elementary masses.

The active forces applicable in this free surface hydraulic study are gravity, viscous friction, and surface tension. Therefore to form a basic model law applicable to this study, it must be assumed that one of these forces predominates. In this study gravity is taken as the active force that dominates the other forces.

Using the MLT system of dimensions and inserting the dimensions in brackets, a mass M(M) undergoes a gravitational force Mg (M.L.T.<sup>-2</sup>). Density,  $\rho$ , is defined as mass per unit volume (M.L.<sup>-3</sup>). The ratio of inertial forces on the corresponding elementary masses as between the two models (using here the subscripts A and B) is,

$$\frac{\mathcal{P}_{A}l_{A}^{3}l_{A}/t_{A}^{2}}{\mathcal{P}_{B}l_{B}^{3}l_{B}/t_{B}^{2}} = \frac{\mathcal{P}_{A}u_{A}^{2}l_{A}^{2}}{\mathcal{P}_{B}u_{B}^{2}l_{B}^{2}}$$

where l, t, and u are chosen as characteristic length, time interval, and velocity respectively. The operation of taking l/t = u does not affect the value of the ratio.

The ratio of the gravitational forces on the elements is

$$\frac{\rho_{\rm A}\,\ell^3{}_{\rm A}\,{}^{\rm g}{}_{\rm A}}{\rho_{\rm B}\,\ell^3{}_{\rm B}\,{}^{\rm g}{}_{\rm B}}$$

Therefore for dynamic similarity, where the ratio of gravitational to inertial forces is maintained is,

$$\frac{u^2_A \ell^2_A}{u^2_B \ell^2_B} = \frac{g_A \ell^3_A}{g_B \ell^3_B} \quad \text{or} \quad \frac{u^2_A}{g_A \ell_A} = \frac{u^2_B}{g_B \ell_B}$$

which is the standard form of the Froude Number,

$$\overline{F} = u/\sqrt{gR}$$

 $g_{A}$  and  $g_{B}$  are necessarily the same if a homogenous fluid is considered.

In this model study the fluid is not considered homogenous, but stratified into layers of different densities. Therefore it is necessary to define a new gravitational constant that will take into consideration the different density layers. This is the effective gravitational constant, g', which is  $g\Delta\rho/\rho$ .

Now comparing the inertial and the gravitational forces on characteristic elements in a similar manner to that made in the derivation of the standard Froude number leads to a densimetric Froude number,

$$F_{\Delta} = \frac{U}{((\Delta \rho / \rho) gl)^2}$$

The derivation suggests that this may be used as a criterion of similarity for internal or differential movement within bodies of liquid where internal gravitational forces are considered to be the predominate active forces. The different liquid masses in such systems are coupled; the displacement of one element or mass involves its replacement by another.

The foregoing argument leading to the densimetric Froude number is concerned with the overall similarity of compared systems such as the two lake models considered in this study. Such comparisons are, however, sometimes made on the basis of the Richardson number,  $\overline{Ri}$ , although the Richardson number was originally intended as a criterion for the degree of local impedance of turbulence by a stable and continuously varying stratification. The derivation for a non-compressible fluid is given by Frandtl  $\sqrt{257}$  and leads to,

$$\overline{Ri} = - (g/p) dp/dz$$
$$(dU/dz)$$

where  $d\rho/dz$  and dU/dz are density and velocity gradients respectively in the vertical (z) direction.

This has been developed into an "overall" form by taking a characteristic length (l) over which the velocity varies by U and the density by  $\rho$ , thus giving,

$$\overline{Ri} = \frac{(g/p)(\Delta p/l)}{(Ul)^2} = \frac{g\Delta pl}{pU^2} = \overline{F_{\Delta}}^{-2}$$

It is however interesting to derive the model law for this study directly from energy concepts and using the idea of corresponding elements. The ratio of kinetic energy available for mixing in the compared systems, whether the motion of the elements represents an overall change of velocity of flow in either horizontal or vertical directions, or the vertical fluctuations of eddying motion is,

$$\frac{1}{2} \frac{P_A}{P_B} \frac{l^3_A}{l^3_B} \frac{U^2_A}{U^2_B}$$

Still assuming similarity between the compared systems, if work is done against a density gradient by the movement of the elementary mass, the ratio as between the two models is,

$$\frac{l^{3}_{A} g_{A} \Delta p_{A} l_{A}}{l^{3}_{B} g_{B} \Delta p_{B} l_{B}}$$

Since the ratio of work done should equal that of available energy,

$$\frac{P_{A} U^{2}_{A}}{P_{B} U^{2}_{B}} = \frac{g_{A} \Delta P_{A} \ell_{A}}{g_{B} \Delta P_{B} \ell_{B}}$$

To obtain the Reynolds number model criterion in a similar manner, it is assumed that viscous active forces predominate and the ratios of the active forces on corresponding elementary masses are equated with the ratios of the inertial forces.

$$\frac{\mathcal{P}_{A} \ \mathbf{u}^{2}_{A} \ \boldsymbol{\ell}^{2}_{A}}{\mathcal{P}_{B} \ \mathbf{u}^{2}_{B} \ \boldsymbol{\ell}^{2}_{B}} = \frac{\mu_{A} \ (\delta \mathbf{u}/\delta \boldsymbol{\ell})_{A} \ \boldsymbol{\ell}^{2}_{A}}{\mu_{B} \ (\delta \mathbf{u}/\delta \boldsymbol{\ell})_{B} \ \boldsymbol{\ell}^{2}_{B}}$$

where  $\mu$  is the coefficient of viscosity (or dynamic viscosity) defined by  $\Gamma = \mu \, du/dy$ ,  $\Gamma$  being the shearing stress per unit of area for a laminar flow gradient dx/dy at right angles to the "surface" considered.  $\delta U/\delta l$  is the chosen velocity gradient in the compared dynamically similar systems, Therefore

can be replaced by

$$\frac{\mathbf{U}_{\mathbf{A}}}{\boldsymbol{\ell}_{\mathbf{A}}} \neq \frac{\mathbf{U}_{\mathbf{B}}}{\boldsymbol{\ell}_{\mathbf{B}}}$$

Hence

$$\frac{\mathcal{P}_{A} \quad u^{2}_{A} \quad l^{2}_{A}}{\mathcal{P}_{B} \quad u^{2}_{B} \quad l^{2}_{B}} = \frac{\mu_{A} \quad u_{A} / \quad l_{A} \quad l^{2}_{A}}{\mu_{B} \quad u_{B} / \quad l_{B} \quad l^{2}_{B}} \quad \text{or} \quad \frac{u_{A} \quad l_{A}}{\nu_{A}} = \frac{u_{B} \quad l_{B}}{\nu_{B}}$$

where  $\gamma'$  is the kinematic viscosity,  $\mu/\rho$ , this being the Reynolds number criterion. Rearranging these model criteria and taking the square root in the case of the Froude number.

$$\frac{U_A}{U_B} = \frac{g'_A^{\frac{1}{2}} l_A^{\frac{1}{2}}}{g'_B^{\frac{1}{2}} l_B^{\frac{1}{2}}}$$
Froude or Richardson number  
model criterion  
$$\frac{U_A}{U_B} = \frac{V_A l_B}{V_B l_A}$$
Reynolds number model criterion

To satisfy both criterion

$$\frac{g'_{A^{3}}}{g'_{B^{3}}} \frac{l_{A}}{l_{B}} = \frac{\gamma_{A}}{l_{B}} \frac{g'_{A^{3}}}{q'_{A}} \frac{l_{A}}{l_{A}} \frac{3/2}{q'_{B}} \frac{g'_{B^{3}}}{s'_{B}} \frac{l_{B}}{l_{B}} \frac{3/2}{s'_{A}}$$

where

$$\overline{F_{\Delta R}} = \frac{g^{\frac{1}{2}} l^{3/2}}{\sqrt{2}}$$

can be termed the Froude-Reynolds number,  $\overline{FR}$ , and specifically in this case the densimetric Froude-Reynolds number,  $\overline{F_{\Delta}R}$ .

From this theoretical development it is shown that for the models to be similar then the Froude or Richardson number criterion must be the same for both. Also the Reynolds number criterion must be the same for each model. It then follows that the densimetric Froude-Reynolds number will also be the same for both model A and model B.

Utilizing these model parameters, then each lake model's variable parameters can be adjusted accordingly to achieve an equality in the model criterion. These variables are the density differences, which accordingly vary the effective gravitational constant for each model, and the inflow velocity for each model.

The characteristic length, k, utilized in the theoretical development is taken to be the depth of the model considered, d. With this parameter held constant due to the design of the models, the limits of the range of equal modeling parameters are determined by the experimental procedures utilized in obtaining the density differences and the inflow rates. These conditions were basically limited by the design of the experimental apparatus.

### CHAPTER III

#### EXPERIMENTAL TECHNIQUES

This chapter presents the details of the laboratory facilities, the lake models, the flow visualization technique and the collection and reduction methods. The experimental procedure and testing methods are also described.

#### Lake Models and Inflow System

Two lake models were constructed for experimentally determining the validity of the vertical scale exaggeration concept. Both models were constructed from  $\frac{1}{2}$ -inch plexiglas, to allow observation of the bulk of the lake model's volume. Figure 1 shows a sketch of the experimental set-up, with the lake model shown in the center.

Both lake models are eight feet in length and have a horizontal dimension of eighteen inches. The vertical dimension is twelve inches for the exaggerated scale model and six inches for the natural scale lake model. This is a vertical exaggeration of twice the natural scale of model B. The lakes are designed to have a water depth of six and three inches respectively.

Both lake models have their own support stands which are identical in size and construction. Each lake model has an inlet contour molded into the upstream portion of the model vol-

ume. The contours are of arbitrary shape, to represent an idealized lake bottom contour. Both contours are geometrically similar with only the vertical dimension exaggerated in the large model to coincide with the proper simulation of the total lake's similarity.

Figure 2 shows the schematic of the inflow system and the dye injection system. Three 45 gallon plastic storage tanks are set on the upper deck of the laboratory to yield the required head necessary for the flow system to operate by gravity feed. The three storage tanks feed directly to the control panel, which is a central location of all values and flow meters utilized in the project.

The flow lines for the entire system are made from either Polyvinyl Chloride (P.V.C.) pipe,  $\frac{1}{2}$ -inch in diameter or  $\frac{1}{2}$ -inch rubber garden hose to minimize the corrosion problem inherent with saline solutions. The valves utilized for the storage tank lines, the fresh water line and the drain are all made of stainless steel and brass for the same reason.

The values from the different water sources are connected to a manifold constructed of  $\frac{1}{2}$ -inch P.V.C. pipe and the outflow from this manifold is connected to a plastic  $\frac{1}{2}$ -inch gate value for flow control, then through a Fisher-Porter precision bore rotameter with a flow range from 0.2 GPM to 2.0 GPM. From the flowmeter the flow is introduced to the inlet channel entrance pipe.

The inflowing fluid enters the inlet channel which is shown

in Figure 3. The inlet channel width can be varied from a maximum of two inches to a minimum of one inch width through the use of  $\frac{1}{2}$ -inch plexiglas inserts.

The flow then leaves the inlet channel and flows into the lake model upstream section. This is the section of the lake model that contains the contour of the lake bottom. The contours are constructed from  $\frac{1}{4}$ -inch fiberboard laminated to form the desired shape, then contoured to achieve geometric similarity. The interior portion of the lake models as well as the contours are sealed with a clear silicone sealing compound to ensure that the models are watertight.

The outflow end of the lake model has a weir attachment installed. The weir attachment is designed to accomodate either a flat edge (or straight edge) weir or a V-notch weir. The arrangement allows variation in the vertical direction to insure proper lake depths. The weir system has a calibration system embossed on its surface to allow readings of the outflow rate.

### Flow Visualization

To visualize the flow patterns of the different inflows into the stratified lake models, a dye injection system was constructed. A dye container was positioned by the control panel on a movable stand, to allow a variation in the gravity head for the dye system. The dye container was fitted with a small ball valve on the bottom side to allow regulation in the dye flow rate. The dye was then transmitted through 1/8 inch plastic tubing to a large hypodermic needle. The needle was inserted into the

desired storage tank flow line for dye injection into the flow.

This arrangement was designed to allow complete mixing of the dye with the inlet fluid before it reached the lake inlet, and also to allow the inflow fluid in the storage tank to be kept clear. This was necessary because for each experimental test, the selected inflow fluid was introduced into the lake model in the un-dyed form until the flow became developed, thus allowing the dye traced fluid to be introduced that would accurately visualize the flow pattern.

### Data Collection System

To record the dye-traced flow patterns of the various inflows a photographic system was devised to record all pertinent information of the flow field. Since the lake models were constructed of transparent plexiglas, visual observation of the flow patterns could be made from both a top and a side view. Utilizing a 35mm Ashia Pentax single lens reflex camera all pertinent data was recorded on a single frame of film for a certain period of time during the experimental test.

This was accomplished by means of mirror systems shown in Figure 4. Two mirror systems were utilized, one for the top view and one for the side view of the test section. The top mirror system consisted of a 24  $\times$  36 inch double plated mirror inclined at a 45° angle. The side mirror system consisted of four 8  $\times$  24 inch mirrors so arranged as to yield the same focal length for both the side and the top view.

The 35mm camera was positioned approximately twelve feet

from the lake model on a tripod. A 150mm lens was used to eliminate the depth of field problem inherent in this type of optical arrangement. The film used was Kodax TX-135 and PX-135, with ASA numbers of 400 and 125 respectively.

The lighting arrangement for the photographic set-up consisted of a four bulb fluorescent light as a back light for the side view of the lake model. Three 300 watt photoflood lights were positioned on the top and the bottom of the model to provide adequate illumination.

Both lake models had a grid pattern attached to the side surfaces with 1/16 inch black circuit tape to yield a grid pattern of one inch squares to aid in the determination of the location of the dye patterns. A similar grid system was used on the bottom of the lake models. Placed on the top of the side mirror system was a data information board and a digital timing clock with a readout in seconds.

A typical photograph recorded both a top view and a side view of the lake flow pattern at one time. By taking a series of photographs at approximately one to two second intervals a complete record of the flow pattern was recorded for each test.

#### Data Reduction Technique

Each test sequence was approximately 20 photographs in length. The film was developed in the laboratory darkroom, and checked to determine if all test results were recorded properly and if the test conditions were met.

If the developed film was suitable for data purposes,

enlargements were made of all the frames of each test film roll. The enlargements were usually four by five inch prints, but on occasion when the flow pattern was unusual or unique, the prints were made at a larger size, usually eight by ten inch prints.

The prints of each run sequence were inspected to verify that all test data was shown and in proper focus. Then each print was analyzed and the dye front trace was determined from the dye front pattern against the reference grid system and the location of points was recorded in terms of vertical and horizontal positions. This method was utilized for both the top and the side view.

After the dye front profiles for each photograph (at successive periods of time in the test) were recorded, velocity profiles were calculated from this data. This was accomplished through the use of a velocity computation program that is outlined in Appendix B. The program is rather simple and is used with the Hewlett-Packard series 9820 computer.

The velocity at a given location is determined by the standard time of flight technique. The test photographs record the dye front traces at a number of successive positions in the flowfield. The velocity at the point of interest is then computed from the data obtained from the two dye fronts which are in sequence ahead and behind the selected location. A collection of the velocities at specified points was obtained through the use of this computing method. The data was then plotted as nondimensional velocity versus non-dimensional distance for specified

locations down the length of the lake model. Figure 5 is a typical example of a side view velocity profile plot obtained in the deeper lake model.

These velocity profile plots were obtained for both models for the same test configuration and combined to yield a comparative plot of both the individual velocity profiles at the smae specified location. This was the method employed to determine whether the two lake models were similar to each other for the test configuration chosen.

#### Experimental Procedure

The experimental procedure consists of the methods used to determine how the various control parameters are determined and how they are introduced into the lake models. Also the testing sequence is outlined with discussion of various points that are considered significant.

The testing program was developed from the basic theory presented in Chapter II. Considering the shallow lake model, a density stratification configuration was determined with regards to the number of levels layers of fluids needed along with the density value of each layer. In the small lake it was essential to develop a stratification scheme that utilized the largest spread available in density difference. Since sodium chloride, i.e., salt, was used as the density altering agent, the maximum density range was from 62.4 pounds per cubic foot to 63.6 pounds per cubic foot. The large range was necessary in light of trying to develop the same Richardson number and the same Froude-Reynolds number for both models. Recalling from the development

of the modeling theory that the Richardson number,

$$\overline{Ri} = \frac{U}{(g'H)^{\frac{1}{2}}}$$

and the Reynolds number in this study is taken to be,

$$\overline{R} = \frac{U H}{\gamma \gamma}$$

which leads to the overall Froude-Reynolds number,

$$\overline{F_{\Delta R}} = \frac{(g')^{\frac{1}{2}} (H)^{\frac{3}{2}}}{\sqrt{2}}$$

The effective gravitational constant is defined as,

and the densimetric velocity is,  $U_A = (g'H)^{\frac{1}{2}}$ .

Therefore, for the models to have the same Richardson and the same densimetric Froude-Reynolds number, the only parameter that is variable is the effective gravitational constant, g'. This is because the depths, H, are set for both models by the model construction. It is then obvious that for the small model to have the same similitude parameters as the large model, the density range of the small model must be as large as possible to allow for the small density ranges needed for the large model. Due to the limitation in accurately measuring the density of the fluids by means of a hydrometer, a minimum density range that can be tolerated is from 62.4 pounds per cubic foot to 62.53 pounds per cubic foot.

Due to these limits in the density ranges only two basic configurations of lake model stratification could be imposed on either model. The other flow parameter controlled was the inflow velocity. This was controlled by means of the flowmeter mounted on the control panel. By setting a desired inlet width and lake depth, the flow rate could be adjusted to yield the desired inflow velocity.

The testing sequence consisted of determining the desired lake densities and loading the separate storage tanks accordingly. This was done by filling the storage tank with fresh water and adding the proper weight of salt to the water. The solution was then mixed and a hydrometer reading was taken to determine the specific gravity of the solution. When the desired density was reached the solution was allowed to set for one hour and measuments were taken at 15 minute intervals to insure that the the correct density was obtained. All required density solutions were made up in this manner.

The lake model was filled with fresh tap water initially and allowed to set for a period of three to eight hours. This was to let all initial turbulence in the model die out. A temperature measurement and a specific gravity measurement was made of the lake model. If the density of the lake model was different than the desired density, the solutions made up in the storage tanks were changed to yield the proper density difference required for the test. When the lake had settled and the densities were correct, the heavier density fluid was then introduced into the lake model through the inlet hydraulic system. This was done at the flow rate desired for the test to check the inlet depth and lake level for the correct set-up of the test conditions. If any discrepancies were noted they were corrected at this time. After the desired depth of the heaviest inflow solution was reached the flow was shut off and the lake was allowed to settle for approximately one hour. If another density layer was required, it was then introduced into the model at this time. The introduction of the heavier solutions into the fresh water at relatively low flow rates resulted in very little mixing between the density layers and yielded a stratified lake model in a relatively short period of time.

The test was initiated by turning the lighting system on. The proper flowmeter setting was then made to start the flow of the desired solution into the inlet section of the lake model. The inflow was allowed to run for a period of 20 seconds, this was determined by previous tests that determined when the inflow became developed and the initial transient condition passed.

When the flow was considered developed the dye injection system was turned on, introducing a dye, usually red or green food coloring, into the inflow fluid. When the dyed fluid progressed to the inlet exit, the timing clock was initiated. Photographs of the flow pattern were then taken as soon as the dye-traced fluid entered the test section. The test section was at a position of 28 inches to 46 inches down the length of the lake model from the inlet. The photographs were taken at one

to two second intervals. This was continued until the flow had passed out of the test section.

When the test was in progress measurements of the inflow rate, the inlet depth and the depth of the lake were continuously monitored. Also readings were taken of the outflow rate at the weir exit.

At the conclusion of the test, the dye injection system and the inflow were discontinued. The lake model was then drained and flushed with fresh water to remove any saline solution that was left. The storage tank system was also drained and flushed. The equipment was then ready for the next test.

When the other lake model was to be used in the next test, the model in place was removed and the other model moved into it's place, then leveled and connected to the hydraulic system. The lake model was then filled with fresh water and the flow parameters for the next test were then set. This was done by the adjustment of the outlet weir to obtain the desired lake model depth. Also the inlet channel was aligned and tested to check for misalignment which would result in improper inflow. Then the testing procedure was again repeated.

### CHAPTER IV

#### EXPERIMENTAL DATA

This chapter presents the experimental data obtained from various density inflows into the two stratified lake models. Briefly, the experiments can be classified in the following way. Mean velocity profiles were obtained by plotting the instantaneous velocities at specified distances down the lake models for a number of experimental tests and taking simple averages.

Experiments were performed on the two lake models at the same Richardson and Reynolds numbers. Since the only parameter that was different was the extent of vertical scale exaggeration this provided a critical test concept of vertical scale exaggeration. Table I is a summary of the test conditions and the data acquired during this study. It should be noted that each test was conducted repeatedly to yield a number of instantaneous velocity points at each selected location. The number of individual runs in each test is indicated in the table.

### Mean Velocity Profiles

Mean velocity profiles were determined from composite plots of the velocity points obtained from the dye front photographs. Figures 6 through 12 indicate the mean velocity profiles determined for the selected locations down the length of the lake
models. The velocity plots were non-dimensional so as to yield a comparative profile between the models. As shown in Table I the test series B refers to the same test repeated on the shallow, 6 inch depth model three times. This test was used as the base data in this study. Test A was the experiment on the deep lake model, which was tested at the same Richardson and Reynolds number as the shallow lake model.

The test section interval starts at a position 34 inches from the inlet and continues down the lake model for 24 inches. The locations selected for determination of velocity profiles start at the 34 inch location and continue in two inch increments to 42 inches. This test section yields initially part of the mixing zone and also part of the stable zone of flow.

These velocity profiles can then be used to indicate the degree of similitude obtained between the lake models for the various test conditions, as described in Chapter III. A discussion of these profiles and their significance is presented in Chapter V of this report.

Measurement of the velocity at the specified locations was difficult due to the design of the data acquisition system which severly reduced the accuracy of the measurements. The end result of this system was that the flow situation was recorded on film which when enlarged to the largest size (this was determined by the resolution), locations of the dye front could only be made within one tenth of an inch. Since the dye fronts were never more than two inches in height only twenty-four positions could be taken, which results in basically a rough outline of the

Test	Number	Lake Model	Total Depth (H <sub>t</sub> ) (in)	Inlet Depth (H <sub>i</sub> ) (in)	Density Difference <u>AP</u> P	Flow Rate (Q) (GPM)	Inlet Velocity V i (ft/sec)	RT	FAR	
	A	Deep	6	ł	0.00118	0.578	0.3714	2.695	6892.00	
	B	Shallow	3	4	0.00943	0.58	0.744	2.695	6892.02	

# TABLE I

TESTING PROGRAM DATA

velocity pattern. Since the inflow fluid was in the turbulent region the data points collected for each test showed a wide range of velocities. The only method to determine a mean velocity profile from this type of data was to repeat the same test conditions a number of times to yield a composite plot of velocity data, and from this a mean velocity profile could be obtained. It should be noted that this method is very rough and the amount of time needed to complete a velocity profile is excessive.

## Dye Front Profiles

From the photographic data obtained during the test program, a record of the dye front profiles was obtained for each test. It is interesting to develop a comparative plot of these dye fronts at selected locations and times, to yield insight into the mechanism of the inflow fluid. Figures 13 through 17 are comparative plots of the nondimensionalized dye front profiles between tests A and B. It was observed that it was necessary to dye the inflowing fluid in order to be able to distinguish between the different density fluids. To the eye and to the camera, the dye colored water is dominant over the undyed water existing in the lake model.

The tests were made for both cases of dye coloring, for the case as mentioned above and for the case of having two distinct density layers in the lake model at different colors and a clear inflow fluid. This method resulted in photographs which yield an unsatisfactory record of the flow situation. This was due to the dilution of the lake dyes with the incoming fluid yielding

a varying color scheme between the two initial lake layers, and inaccurate recordings of the inflow profiles.

Although the differential coloring method is unsuitable of recording the flow data on black and white film it yields interesting data when visually observed. The degree of mixing between the different density layers can be observed and the types of flow patterns also can be seen.

# CHAPTER V

### DISCUSSION

Considerable effort has been devoted to determining if the flow in the two lake models is similar when the vertical scale of one model is exaggerated. By using the nondimensional parameters of Richardson and the Froude-Reynolds numbers as the basis of the modeling criteria, experimental tests were made to determine the validity of this concept. In this chapter some of the experimental evidence is discussed including the various aspects of the inflow patterns as related to inflow rates, slope distortion, mixing rates of the fluid, and density variation.

## Discussion of Visual Observations

The primary information in this investigation is the time sequenced photographs of dye-traced inflow fluids. As seen in Figures 13 through 17, which are comparative plots of the dye front profiles for test A and B for selected locations, there is a difference in the flowfields of the two tests. If vertical scale exaggeration had no effect on the flowfields (suitably nondimensionalized) there would be no difference in the data from tests A and B since they are run at the same Richardson and Reynolds numbers. The gross features of the flowfields are similar, for example in both cases the inflow tends to "lens" out

over the denser layer. However there are some noticeable differences indicating the similarity is not exact. In test B, the shallow model with the larger density difference, interfacial waves are observed at the lower interface. But in the upper interface there is the predominance of the force of gravity which increases the slope of the inflow fluid and this tends to dampen the interfacial wave growth.

Browand and Winant /10/ have described this mechanism in their paper on laboratory observations of instabilities of shear layers in stratified fluids. As observed in the lake models, there are initially instabilities introduced into the inflow fluid when it reaches the density interface in the lake model. The inflow fluid acting under a gravitational force increases its inertial energy as it flows down the contour slope. When the fluid reaches the density interface the buoyancy forces tend to counteract the inertial forces, which introduces instabilities into the inflow fluid. This area in the lake model is referred to as the mixing zone. In this stratified shear layer the instability is redistributed along the interface. This redistribution generates interfacial waves, which can be noted in the dye front profiles. These interfacial waves and the residual turbulence decay and the shear flows approach laminar state. This can also be seen in the dye front profiles at distances farther down the lake model. This instability is related to the Richardson number of the inflow, with a higher Richardson number being more stable than a smaller Richardson number.

Also observed in this model study was the development of "rollers" in the inflow fluid. These "rollers" were the rolling up of the inflow fluid about a horizontal axis oriented towards the side of the lake model. This observation was not evident from the photographs of the lake models but could be seen visually. These "rollers" were quite large in both tests and did not appear to be similar. Various attempts to reproduce these "rollers" were not successful. This leads to the reasoning that these are not reproducible and therefore the flow will not be similar in this respect.

Due to the method of coloring the inflow fluid only tip velocities can be obtained for any one location and internal velocities are prevented from being obtained from this coloring scheme. As regards mixing actions, two types were noted. At the tip, especially the underflow tip, there was a rolling up process similar to that shown by Prandtl's 1952 illustrations  $\sqrt{25}$ . This was observed when the interface was otherwise completely smooth, which suggests laminar conditions of flow at the interface at least. In some cases the "roller" was seen quite distinctly as in Frandtl's idealized figure. As the increasing of depth or of initial density difference, or both, led towards more turbulent conditions, the rolling layers no longer appeared to have distinct existence, but the general pattern of movement seemed to be the same.

The other type of mixing action which was clearly observed, occurred behind the tip of the inflow profile, when the depth and density difference were such to give values of  $\overline{\mathbb{F}_{\Delta}\mathbb{R}}$  of the

order of 5,000 or less. Interfacial waves, on the point of breaking, were observed in the region behind the underflow front in the inflows of the smaller lake model. When the  $\overline{F_{\Delta}R}$  was slightly increased, breaking of the waves could be seen and on further increase the individual waves could no longer be distinguished. The general impression gained by the author after watching many experiments, was that the turbulent mixing between the density layers grew more intense with increasing values of  $\overline{F_{\Lambda}R}$ .

This type of interfacial wave formation was also noted by Ellison and Turner /12/ as occurring behind the nose of an underflow layer progressing down a slight slope. It seemed reasonable to assume that similar waves could be obtained behind an inflow front, though such were not actually observed.

Another interesting observation was that due to the exaggeration of the vertical scale the contour in the deeper model is distorted. This distortion appears to be a critical factor in the design of the exaggerated scale model. This is because when the slope of the contour is greater than seven degrees the inflow tends to become detached from the contour surface. This results in a flow pattern in the deeper model which does not model the shallow model's flow pattern.

### Similarity of Flowfields

Utilization of the Richardson and the Froude-Reynolds number as the basis for the development of the model parameters between the lake models is one of the main aspects of this report. From

the experimental data it can be noted that there is some similarity between the mean velocity profiles and the dye front traces for the tests in the deep and shallow models which were run at the same Richardson and Reynolds number. It is believed that due to the inaccuracy of the data collection system, that there can be no definite conclusions drawn, outside of the fact that the flow patterns tend to be similar but not exactly so. In the figures noted for comparison, the velocity profiles can be seen to differ between tests in both a vertical displacement as well as a horizontal displacement. This is probably due to the slight variations in the test conditions, but for a generalized view this is not considered critical. The basic information to be incurred from the comparative plots of both the velocity profiles and the dye front traces is that when vertical scale exaggeration is used, the flow patterns are closed but not exactly similar.

The intersurface phenomena occurring is thought to be not as dependent on total depth of the lake as originally stated in the development of the Richardson number. Perhaps it would be best to base the Richardson number on a characteristic length of a horizontal dimension. No definite statement can be made on this point with the results obtained in this study. Therefore it is suggested the further investigation of this area be considered and it is possible the comparison of the two types of investigations would lead to additional insight into this phenomena. This work might be directed in such a way as to vary the Richardson number over a wide range and utilize both a

horizontal and a vertical characteristic length.

Due to the limited amount of data and also the limited variations attempted, no positive statements as to the validity of the vertical scale exaggeration can be made. But it is worthwhile to describe the observation noted during the testing and this leads to a better understanding of the problems in-. volved in the use of an exaggerated scale model.

As the data tentatively indicated, there is a degree of similarity achieved between the two lake models when the test conditions are set in accordance with the Richardson and Froude-Reynolds number criteria. Since both geometric and dynamical similarity were imposed on the models, then this indication of similarity between velocity profiles also tends to indicate a degree of similarity between the kinematic aspects of the flow in both models.

### CHAPTER VI

### SUMMARY AND CONCLUSIONS

#### Summary

The objectives of this study were to experimentally determine the validity of the concept of vertical scale exaggeration with stratified lake models. This was developed by establishing a modeling technique and visualizing the flow of an incoming fluid of an intermediate density to the two densities of the lake model stratification layers.

Numerous tests were carried out with both photographic recording of the flow and visual observation of the tests. The photographic data showed in some detail the interfacial waves, the rolling motion of the inflow front, and the mixing and dispersion patterns of the various tests.

Comparisons of the mean velocity profiles for the various tests were made to determine whether the lake models indicated kinematic similarity. Also dye front profiles were recorded to outline the major flow characteristics of the inflows. Visual observations of the many experiments were made to yield an insight into the flow mechanism that could not be properly recorded by the photographic system utilized in this report.

The problem inherent to vertical scale exaggeration, slope

distortion, is discussed and the effects of this distortion are critical to an exact modeling of the lake models.

# Conclusions

The conclusions of this study may be stated as follows:

1. Modeling a stratified lake according to the Richardson and Froude-Reynolds number concept, a degree of similarity can be obtained for a model utilizing vertical scale exaggeration.

2. To properly model a stratified lake when vertical scale exaggeration is used requires cautious design in regard to the distortion inherent in the exaggeration. In this study the distortion factor was introduced in the contours and leads to a non-similarity of flow patterns. The critical factor was that when the contour slope exceeded seven degrees the flow became detached which was prior to the point that would have modeled the shallow model.

3. There appears to be a rolling motion in the inflow which is quite large in structure and is non-reproducible between the models. This is considered to verify that there exists a dissimilarity between the lake models tested.

4. Further investigation needs to be made into the consideration of developing a modeling technique based on a horizontal length instead of a vertical length for use as the characteristic distance parameter.

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# APPENDIX A

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# FIGURBS AND ILLUSTRATIONS



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Figure 1. Lake Model and Inflow System.







MODEL B

Top View Same As Above



Figure 3. Cross Section Showing the Lake Model's Inlet, Contour, and Test Section.





Figure 5. Typical Velocity Profile Developed From Succesive Runs of the Same Test.

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Figure 6. Mean Velocity Profiles for the Two Tests at X = 34 inches,

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Figure 7. Mean Velocity Profiles for the Two Tests at X = 36 inches.



Figure 8. Mean Velocity Profiles for the Two Tests at X = 38 inches.

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Figure 9. Mean Velocity Profiles for the Two Tests at X = 40 inches.

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Figure 10. Mean Velocity Profiles for the Two Tests at X = 42 inches.

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Figure 11. Mean Velocity Profiles for the Two Tests at X = 34 inches, Top View.

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Figure 12. Mean Velocity Profiles for the Two Tests at X = 42 inches, Top View.

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Figure 13. Dye Front Profiles, a Comparison Between the Same Test in Both Models.

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Figure 14. Dye Front Profiles, a Comparison Between the Same Test in Both Models.



Figure 15. Dye Front Profiles, a Comparison Between the Same Test in Both Models.

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Figure 16. Dye Front Profiles, a Comparison Between the Same Test in Both Models.



Figure 17. Dye Front Profiles, a Comparison Between the Same Test in Both Models.

# APPENDIX B

COMPUTER PROGRAM LISTING

### APPENDIX B

#### COMPUTER PROGRAM LISTING

This program is utilized with the Hewlett-Packard 2908B desk computer.

```
PRT "DELTA TIME": PRT "CALCULATIONS"; 80-A; SPC 2 /
00
01
     ENT "TIME(MIN.) R 80=", R30; ENT R81, R82, R83, R84, R85, R86,
     R87, R88, R89 /
     PRT "DELTA TIME ="; SPC 1 /
02
03
     A + 1 \rightarrow A \neq
     1 * (RA - R(A - 1)) - R(A - 10) \neq
04
     FXD 0: PRT A - 79 /
05
     FLT; PRT R(A - 10); SPC 1 \neq
06
07
     IF A = 89; GTO 9 \neq
     GTO 3 🖌
08
     SPC 5; ENT "Y POSITION =", R50; PRT "FOR POSITION Y =", R50;
09
     SPC 54
     16 > A; PRT "DELTA X"; PRT "CALCULATIONS": SPC 2 /
10
     ENT "X POSITION(IN.)R16=", R16; ENT R17, R18, R19, R20, R21,
11
     R22, R23, R24, R25 /
     PRT "DELTA X ="; SPC 1 \neq
12
     A + 1 \rightarrow A \neq
13
     RA = R(A = 1) = R(A = 10) \neq
14
     FXD O; PRT A - 15/
15
     FLT ; PRT R(A - 10); SPC 1 \neq
16
     IF A = 25; GTO 19 \neq
17
     GTO 13 4
18
     SPC 5 /
19
     6 > A; PRT "VELOCITY"; PRT "CALCULATIONS"; SPC 2 /
20
21
     70 ) B /
     PRT "VELOCITY ="; SPC 1 /
22
23
     A.+1 → A ≠
     B + 1 \rightarrow B \neq
24
25
     RA/RB \rightarrow R(B - 41) \neq
     FXD O; PRT A - 5 \neq
26
     FLT ; PRT R(B - 41); SPC 1 \neq
27
     IF A = 15; GTO 30 \neq
28
     GTO 23 /
29
30
     SPC 5 /
31
     PRT "AVERAGE VELOCITY"; PRT "COORDINATES"; SPEC 2/
32
     6 \rightarrow A/
     £ + 1 - A /
33
```

 $R(A + 9) + RA/2 \rightarrow X \neq$ 34 35 FXD O; PRT A -5/36 FLT; PRT X; SPC 1/ IF A = 15; GTO 39/37 38 GTO 33/ 39 SPC 5; 29 > A; ENT R51/ PRT "DIMENSIONLESS"; PRT "VELOCITY AT"; PRT "POINTS ABOVE"; 40 SPC 24 PRT "DIMENSIONLESS"; PRT "VELOCITY ="; SPC 2/ 41 42 A + 1 + A/ 43 RA/R51 - R(A + 10)/44 FXD O' PRT A > 28/ FLT; PRT R(A + 10); SPC 1/ 45 IF A = 38; GTO 48/46 47 GTO 42/ 48 SPC 5; PRT "FOR NEW Y VALUE,"; PRT "PUSH RUN PROGRAM"; PRT "THEN ENTER 1"/ SPC 3; PRT "FOR FINAL VEL"; PRT "CALCUL., PUSH"; PRT "RUN 49 PROGRAM AND"; PRT "ENTER 2"/ DSP "READ TAPE"; STP / 50 51 ENT A/ 52 IF A = 1; GTO  $9 \neq$ SPC 54 53 PRT "FINAL VELOCITY"; PRT "CALCULATIONS"; SPC 2/ 54 ENT R94, R95, R96, R97, R98/ 55 ENT "Y POSITION =", Y/ 56  $(R95 - R94)*((R96 - R97)/(R98 - R97)) + R94 \rightarrow C/$ 57 58 ENT "NON DIM PARAM =", Z/ 59 Y/Z + R52 / 60 PRT "Y\*=", R52/ PRT "V\*=", C≠ 61 SPC 1/ 62 63 GTO 55/ STP / 64 END/ R272 65
#### VITA

## Stephen J. Vogel

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