A COMPUTER GROUND-WATER MODEL FOR THE TILLMAN ALLUVIUM IN TILLMAN COUNTY,

OKLAHOMA

Ву

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Thesis Approved:

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PREFACE

This study is concerned with the aquifer properties of the alluvial terrace and floodplain deposits in Tillman County, Oklahoma. The primary objective is to determine the maximum annual yield for the basin from July 1,1973, to July 1, 1993. A computer model is used to determine the maximum annual yield based on pumpage prior to July 1, 1973, and subsequent allocated pumpage until July 1, 1993.

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Finally, special gratitude is expressed to my wife, Azizah, and my children, Sabica, Aliya, Saud, and Dhuha, for their loving consideration and patience during the time of this study.

I dedicate this thesis to the memory of my beloved father and mother. May Allah rest them in His Paradise. Amen.

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CHAPTER I

ABSTRACT

The alluvial terrace and floodplain deposits in the western half of Tillman County are associated with the Red and North Fork of the Red River, and extend over an area of approximately 285 square miles. Ground water in these deposits supply about 850 wells for domestic and irrigation purposes. Cotton, wheat and alfalfa are the major irrigated crops.

A finite-differencing digital model was used to simulate well drawdown over a 20-year period between July 1, 1973, and July 1, 1993. A maximum annual yield was determined based on pumpage prior to 1973 and subsequent allocated pumpage at different trial rates. Calibration techniques were also compared. A pattern recharge of 2.87 inches was selected for optimum design. Computer input data include evapotranspiration and well rate at the surface; water level, land and bedrock elevations; specific yield, coefficient of storage and leakage rate of the river bed.

Computer output is used to show a rate of 1 acre foot per acre could be recommended as an allocation to each one-quarter section of the 285 square mile aquifer area. At this rate, only one-half of the aquifer area would go dry during the 20-year period. On this basis, 70,048 acre feet per year was established as the maximum annual yield. This annual discharge rate will yield a total volume of 1,400,967 acre

feet pumped from July 1, 1973, to July 1, 1993. The model results indicate that more than one half of the wells which belong to priorappropriative right owners would go dry if annual allocation was permitted at the recommended rate. Only four percent of the priorappropriative wells would go dry if annual allocation was not permitted.

CHAPTER II

INTRODUCTION

The Quaternary alluvial terrace and floodplain deposits along the Red River and the North Fork of the Red River in the western half of Tillman County comprise the major aquifer for the area. This aquifer supplies water for the municipalities of Frederick, Tipton, Davidson, and Manitou, as well as for domestic and irrigation uses in this area.

There are more than 800 water wells drilled in this area. Hydrogeologic information has been obtained from approximately 165 wells. The data collected from some of these wells include location, well elevations, water levels and total bedrock depth. A few pump tests have also been conducted. The locations of these wells are distributed throughout the area as shown in Figure 1.

The area has undergone extensive pumping during the last 20 years as a result of increasing irrigation development. Consequently, the water levels have been declining in some areas due to ground-water mining. In November, 1968, Tillman County was declared a critical ground-water area by the Oklahoma Water Resources Board.

Under Oklahoma Statute No.'s 82 § 1020.4 and 82 § 1020.5, the Oklahoma Water Resources Board is responsible for completing hydrologic surveys of each fresh ground-water basin or subbasin within the state of Oklahoma and for determining a maximum annual safe yield which will provide a 20-year minimum life for each basin or subbasin.

Oklahoma Statute No. 82 § 1020.5 states the following:

Figure 1. Well locations



After making the hydrologic survey, the Board shall make a determination of the maximum annual yield of fresh water to be produced from each ground-water basin or subbasin. Such determination must be based upon the following:

- The total land area overlying the basin or subbasin;
- 2. The amount of water in storage in the basin or subbasin;
- 3. The rate of natural recharge to the basin or subbasin and total discharge from the basin or subbasin;
- Transmissibility of the basin or subbasin; and
- 5. The possibility of pollution of the basin or subbasin from natural sources.

The maximum annual yield of each fresh ground-water basin or subbasin shall be based upon a minimum basin or subbasin life of twenty (20) years from the effective date of this act. An annual allocation in terms of acre feet per acre per year is to be determined based on the maximum annual yield and used as a basis for issuing permits to owners whose land is located within the aquifer area.

Objectives

The objectives of this study are to utilize the available hydrological data of the area and determine the maximum annual yield and annual allocation of fresh ground water that can be produced from the alluvial floodplain and terrace deposits of the Red River and the North Fork of the Red River in Tillman County, Oklahoma for the 20-year period between July, 1973, and July, 1993. These objectives were achieved using the following methods:

1. Selection of hydrogeologic data including water levels and well data supplied by the Oklahoma Water Resources Board to be used as input data for the mathematical model.

2. Assignment of spatially distributed hydraulic properties to alluvial deposits based on available hydrogeological data.

3. Modification, calibration and validation of an existing mathematical model for predicting changes in an alluvial terrace and floodplain aquifer over a 20-year period.

Previous Work

The terrace deposits of the western part of Tillman County were briefly mentioned by Clifton (1929) as an area of Quaternary or recent exposures consisting of sands and alluvium. In November, 1944, a 24hour aquifer test was conducted on the irrigation well at the Southwestern Cotton Substation near Tipton. In May, 1945, five test holes were drilled in search of a supplementary water supply for public use. Read and Schoft (1947) discussed the water level fluctuations observed between 1940 and 1947 in the irrigation well at the Southwest Cotton Substation. The Layne Western Company of Wichita, Kansas, drilled 40 test holes in the terrace deposits for the city of Frederick in 1948.

Barclays and Burton (1953) wrote a hydrological report on groundwater resources of the Tillman Terrace. This report was updated by the Oklahoma Water Resources Board which published a hydrologic atlas of Tillman County in 1974 (Wickersham, et al., 1974). This report describes the saturated thickness and the areal extent of irrigation associated with the terrace deposits.

Loo (1972) used a mathematical model to study the effect of vertical permeability variation on the Ogallala aquifer. DeVries and Kent (1973) used a digital model developed for the Texas High Plains in order to assess the sensitivity of the model to vertical variability of aquifer properties. Kent, et al. (1973) evaluated the coefficient of

permeability and specific yield of the Washita River alluvium and determined that permeability and specific yield values could be assigned to layered sediments described on drillers' logs. This latter approach was subsequently used in this study.

The alluvial deposits are described as being discontinuous layers of clay, silt and sand which constitute an unconfined water-table aquifer. Bredhoeft and Pinder (1970) and Pinder (1970) designed a basic mathematical model to simulate two-dimensional aquifer problems. This model was used to simultaneously solve the finite-difference equations related to artesian and water-table conditions. Trescott (1973) and Prickett and Lonnquist (1971) added several problem options and inputoutput features. These features included the method of treating the storage coefficient and leakage in conjunction with evapotranspiration for combined artesian, water-table problems. This model has been modified several times by the U. S. Geological Survey. The 1974 version of this model was used in this study.

CHAPTER III

DESCRIPTION OF STUDY AREA

Location

The study area is defined by the limits of the "Tillman Terrace" aquifer located in the western half of Tillman County in southwestern Oklahoma. It includes portions of T2N through T4S, and R17W through R19W as shown in Figure 1. It is bounded on the north by Kiowa County and on the south and west by an imaginary line extending north and south, passing just west of the city of Frederick. The aquifer extends over 285 square miles in Tillman County. Its maximum length from north to south is approximately 29 miles and its maximum width is about 13 miles.

Geology

The rocks exposed in the area range in age from Precambrian to Quaternary as shown in Figure 2. The oldest Paleozoic rocks were apparently deposited at the time when the area was relatively stable. Crustal deformation of the rocks occurred during the early Pennsylvanian resulting in the uplift of the Wichita Mountains. Sediments which might have been deposited between Permian and Quaternary times were eroded due to a long period of weathering and erosion.

The Precambrian granite is exposed at the surface in T2N, R18W, and is surrounded by the Quaternary sediments. The granite is believed to yield a very low quantity of water, except at the intersection of joints.





The Hennessey Formation of Permian age outcrops adjacent to and subcrops below the alluvial deposits. They are characterized by reddishbrown argillaceous siltstones intercalated with thin layers of gray and reddish-brown shale. The outcrop has a gentle regional dip to the southwest. The Hennessey Formation does not yield large quantities of water. Low to moderate yields might be obtained from lenticular sandstones.

The Quaternary sediments consist of alluvial and eolian sands which cover most of the area west of a low gradational escarpment formed by the contact between the alluvium and Hennessey shale (Figure 2). The contact trends north and south immediately west of the city of Frederick. The alluvium is predominately composed of terrace deposits. The terrace deposits consist of discontinuous layers of clay, sandy clay, sand and gravel. Generally the sand and gravel are not well sorted. These sediments vary in color from gray to brown and reddish brown. Scattered pebbles of quartz are found in the clay. The surface is gently undulated to flat, sloping gently westward to the North Fork of the Red River. Elevations range from 1,396 feet above mean sea level in the center of the area of the deposits to 1,131 feet above mean sea level in the southwest corner of the area.

The thickness of the terrace deposits vary from place to place due to irregularities of the underlying shale. Test drilling indicate that the average thickness of the terrace deposits is approximately 42 feet. These deposits thin northward to Kiowa County. Bedrock occurs at the surface in the south-central portion of the study area. Terrace gravels which overlie the bedrock in this area are shown in Figure 3. The terrace deposits are the major supply of ground water in the area. Wells

Figure 3. Hennessey shale and terrace alluvium. Terrace gravels typically occur on top of the Hennessey shale near the eastern edge of terrace alluvium in Sec. 34, T2S, R18W.



completed in these sediments may yield from 50 to 500 gpm and the average yield is 200 gpm. The average coefficient of permeability is 691 gpd/ft² and transmissivity ranges between 100 gpd/ft and 50,000 gpd/ft.

The floodplain alluvium ranges from less than one-half mile to one and one-half miles wide along most of the Red and North Fork of the Red River reaches. It is separated from the terrace deposits by a poorly defined topographic break. Its thickness varies from 27 feet to 47 feet, north to south, and averages 34 feet. The average well yield of the floodplain is 300 gpm. The average coefficient of permeability is 689 gpd/ft² and transmissivity ranges between 200 gpd/ft and 60,000 gpd/ft.

Eolian deposits discontinuously overlie the floodplain alluvium of the Red River and the east side of the North Fork of the Red River. The thickness ranges from 15 to 68 feet and generally occur above the water table. Being highly permeable, they serve an important role in allowing precipitation to infiltrate and recharge the floodplain portion of the aquifer.

Water Table

The terrace and alluvial deposits occur as an unconfined aquifer. A 1969 water table map is shown in Figure 4. The water table slopes at an average gradient of 18 feet per mile westward to the Red River forming an effluent stream condition. The North Fork of the Red River is generally influent with a gradient away from the river at 10 feet per mile. The water table slopes toward the Otter Creek tributary in the northern portion of the area.

Figure 4. 1969 Water-head elevations



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Climate

The area is characterized by a semi-arid climate. The average annual temperature for the city of Frederick is 63.3°F. The average annual precipitation for the period of 1954 to 1974 is 24.95 inches. The highest precipitation occurs in May and the lowest in January. Precipitation data for the cities of Frederick and Tipton is shown in Table I for the period of 1954 to 1974.

Recharge

The sandy soil of western Tillman County has a high rate of infiltration. Presence of discontinuous layers of clay and caliche within the terrace deposits above the water table does not regionally prevent infiltration of precipitation to the zone of saturation but they will generally decrease it. Hydrologic studies by the Oklahoma Water Resources Board (1975) have used nine percent of precipitation as an estimate of net change recharge to the water table.

The average precipitation for the cities of Tipton and Frederick as computed from Table I is 24.45 inches. A recharge of 2.2 inches per year $(6.6 \times 10^{-9}$ feet per second) can be computed based on the percentage of nine percent and the 24.45 inches of rainfall. When this recharge is prorated over the 189,760 acres of the aquifer area, natural recharge is estimated to be 34,726 acre feet per year. Secondary recharge to the aquifer is the return flow from irrigation. A conservative percentage of 15 percent is estimated based on studies by the Oklahoma Water Resources Board (1975).

Evaporation and transpiration are important factors to be considered due to the shallow water table and semi-arid conditions. These two

TABLE I

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PRECIPITATION DATA FOR THE CITIES OF TIPTON AND FREDERICK FROM 1954 TO 1974

Year	Frederick	Tipton
1974	28.43	27.34
1973	35.03	34.61
1972	26.24	22.77
1971	22.51	19.77
1970	18.37	13.18
1968	29.11	30.73
1966	19.27	17.69
1964	25.54	26.52
1962	31.48	29.07
1960	31.42	31.15
1958	23.12	22.69
1956	18.05	19.29
1954	15.82	16.45

factors have been combined together because of the difficulties in computing transpiration alone. In this study, evapotranspiration was considered as a part of net recharge and as evaporation occurring within the first one foot of the surface.

Irrigation

Farming is the major industry in this area. Cotton, wheat and alfalfa are the major crops. There is, however, small quantities of grain, grain sorghum, forage sorghum, oats and barley grown in the area. Pasture grasses are also grown for grazing during the fall, winter and spring months.

The irrigation period occurs predominantly in the months of June, July, August and September. The amount of irrigation differs from one crop to another. The water use for irrigation of different crop types in Tillman County are listed in Table II. The irrigation period as used in the model will include the months of June through September even though minor amounts of irrigation are applied during the early spring months.

TABLE II

	Ground	d Water	Surface Water		
Crop	Acres Irrigated	Water Use Acre-Feet	Acres Irrigated	Water Use Acre-Feet	
Alfalfa Grain Corn Silage Corn Cotton Horticulture Pasture Peanuts Wheat Small Grain Soybeans Grain Sorghum Forage Sorghum Other Crops	1,839 33 15 6,140 46 2,380 0 3,718 447 21 823 591 260	4,190 26 9 4,904 42 4,203 0 2,325 322 54 652 820 241	10 0 0 261 200 50 20 0 50 35 52	25 0 0 303 1,999 22 13 0 66 46 17	
TOTAL	16,313	17,788	678	2,491	
MUNICIPAL AND IN	DUSTRIAL				
Use		Water Use Acre-Feet		Water Use Acre-Feet	
Municipal Industrial Recreation and W Sec. Oil Rec. Other M and I	ildlife	1,306 0 0 0 0		0 0 0 0	
TOTAL	· .	1,306		0	
TOTAL/SOURCE	16,313	19,094	678	2,491	

COUNTY TOTAL ACRES IRRIGATED: 16,991 WATER USE (A.F.): 21,585

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1974 SURFACE AND GROUNDWATER USE FOR TILLMAN COUNTY (AFTER OKLAHOMA WATER RESOURCE BOARD, 1974)

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CHAPTER IV

METHODOLOGY

General

The hydrogeological data collected for the study area was analyzed and spatially distributed over the entire area. Steps employed to use the model for the desired results are summarized in Figure 5. The input data was divided into matrix and constant parameters. The matrix parameters included: water head elevations; land, top and bedrock elevations; coefficient of permeability; specific yield; river bed thickness and hydraulic conductivity; and well pumping rate. The constant parameters included storage coefficient of the river bed and rate and depth of evapotranspiration. Recharge rate was included in either category depending on the computer run used. These input data were transferred onto maps, gridded and punched on IBM cards.

A five-year computer simulation was performed for the period between 1969 and 1974 using the observed 1969 water-head elevations. This simulation was used to calibrate the model. Calibration was achieved by adjusting the recharge rate so that the simulated 1974 water-head elevations were within five feet of the observed 1974 water-head elevations. Three recharge versions were introduced: pattern recharge, calibration matrix recharge and constant recharge.

A 22-year computer simulation (1969 to 1991) was conducted using the calibration matrix and constant recharge versions. Two runs were made

Figure 5. Data input-out flow chart



for each version. Both runs were based on different pumping rates; one run uses the prior appropriative right ownership only and the other one incorporates both prior appropriative right ownership and the subsequent allocated pumping of 0.6 acre feet/acre/year. A final 20-year computer simulation was conducted for the 1973 to 1993 period using the pattern recharge version. Again, two runs were performed; one using prior appropriative right ownership only and the other run using prior appropriative right ownership and the subsequent allocated pumping of 1 acre feet/ acre/year.

Data output from these versions were plotted using the computer printer. Data was plotted for each five-year interval of the total simulation period. Computed output data included transmissivity, saturated thickness, and water-head elevations.

Coefficient of Permeability

To determine the coefficient of permeability and transmissivity, well pump test data for the cities of Tipton and Frederick were analyzed using the Prickett-type (1965) curve method. The method is described in Tables III and IV and results are shown in Figures 6 and 7. The Tipton test resulted in a value of transmissivity of 17,054 gpd/ft and a coefficient of permeability of 352 gpd/ft² as shown in Table III. The transmissivity value obtained from the Frederick test is 21,015 gpd/ft and the coefficient of permeability was 350 gpd/ft² as shown in Table IV.

Problems arose because the limited available data from the 123 wells within the study area could not be used to directly furnish the coefficient of permeability and transmissivity for the entire area. Therefore, another method was used to generate the coefficient

T.	AB	L	E	Ι	I	Ι	
		_	_	_	_	_	

PUMP TEST RESULT USING PRICKETT METHOD FOR SOUTHWESTERN SUBSTATION WELL, TIPTON


TABLE IV

PUMP TEST RESULT USING PRICKETT METHOD FOR THE CITY OF FREDERICK

$$W(U) = 0.62$$

$$\frac{1}{U_y} = 0.81$$

$$U_y = 1.23$$

$$Q = 210 \text{ gpm}$$

$$S = 0.71 \text{ ft}$$

$$t = 1450 \text{ min}$$

$$r = 300 \text{ ft}$$

$$T = \frac{114.60}{\text{ S}} W(U) = \frac{114.6 (210)(0.62)}{(0.71)} = 21,015 \text{ gpd/ft} (22,000)$$

$$S_y = \frac{TU_y t}{2693r^2} = \frac{(21,000)(1.23)(1450)}{2693(300^2)} = 0.155 (.087)$$

$$K = -\frac{T}{t} \text{ where } t = \text{thickness of aquifer} = \frac{21,015}{60} = 350 \text{ gpd/ft}^2$$

Figure 6. Water-table, fully penetrating, constant-discharge, timedrawdown type curves (modified after Prickett, 1965)



Figure 7. Pump test, time-drawdown graph for well "C" at Tipton, Oklahoma



of permeability and transmissivity for these wells and to distribute these values over the entire study area. Information related to thickness and lithology of the terrace deposits was obtained from driller's logs of 123 water wells. The driller's logs obtained from these wells describe the stratigraphic lithology contained in each well. The lithology is divided into four ranges: range one is associated with clay and silt; range two is very fine to fine sand; range three is fine to coarse sand; and range four is associated with coarse sand and gravel. A weighted average permeability was introduced by multiplying a weighting factor for the four ranges of size by the percentage of saturated thickness for each range and summing up the total for all the ranges. This method is described for the Tipton and Frederick pump test sites in Table V. The weighting factors for each range were obtained from the coefficient of permeability grain-size envelope developed by Kent et al. (1973) as shown in Figure 8. The ranges were chosen to represent various median grain sizes which correspond to the average coefficient of permeability for each range as shown in Figure 8. The values for permeabilities were converted from qpd/ft^2 to units of feet/second by using a multiple of 1.55×10^{-5} . The computed weighted average permeability values were compared with the coefficient of permeability derived from pump test analysis as shown in Table VI. Both methods produced similar results. This was considered to be justification for using the envelope in Figure 8 to determine an average permeability coefficient for each well. The computed average weighted permeability coefficients for the wells were used to generate a contour map to represent the distribution of permeability values at a scale of one-half inch per mile. The permeability map is shown in Figure 9. A one-quarter mile grid of the same scale was

TABLE V

WEIGHTED AVERAGE PERMEABILITY FOR THE CITIES OF TIPTON AND FREDERICK

City of	Tipton (Southwes	tern Cotton S	Substation)	· · · · · · · · · · · · · · · · · · ·
Range	Coefficient of Permeability (GPD/FT ²)	Interval Thickness	% Thickness	Weighting of Permeability Coefficient (GPD/FT ²)
1	18	0	0.00	0.0
2 ·	97	53	0.88	85.4
3	516	0	0.00	0.0
4	1,484	7	0.12	178.0
	•		· . ·	
				263.4 gpd/ft ²
			(K̄) Average Wei	ghted Permeability
				· · · · · · · · · · · · · · · · · · ·
City of	Frederick			
1	18	0	0.00	0.0
2	97	34	0.66	64.0
3	516	3	0.07	36.1

14

0.27

400.7

 (\overline{K}) Average Weighted Permeability

500.8 gpd/ft²

1,484

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Figure 8. Relationship between hydraulic coefficient ranges, medium grain size and the coefficient of permeability

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TABLE VI

	Tipton	Frederick
Computed Permeability from Pump Tests	352 gpd/ft ²	350 gpd/ft ²
Computed Weighted Average Permeability Using Envelope in Figure 8	500 gpd/ft ²	264 gpd/ft ²
Transmissivity from Pump Test	17,954 gpd/ft	21,015 gpd/ft
Computed Weighted Average Transmissivity Using Envelope in Figure 8	17,697 gpd/ft	15,840 gpd/ft

COMPARISON OF COEFFICIENTS OF PERMEABILITY AND TRANSMISSIVITY AS DERIVED FROM PUMP TESTS AND DRILLER'S LOGS

Figure 9. Coefficient of permeability map



overlaid onto the contour map. Coefficient of permeability values were assigned to each node by a perimeter-averaging technique described by Griffen (1949). This technique involved averaging interpolated values at the center of each node face with the node center. The digitized permeability values are shown in Figure 10.

Specific Yield

Specific yield values were obtained for each range shown on the envelope in Figure 8. The graph in Figure 11 was used to provide a relationship between median grain size and specific yield. The dominant grain size scale shown in Figure 11 was considered to be equivalent to median grain size. The values for specific yield along with the corresponding permeability coefficients of the four ranges were plotted on a semi-logarithmic paper as shown in Figure 12. A parabolic relationship between permeability and specific yield was developed. Values of specific yield were assigned to each node by using the curve in Figure 11 and the average permeability coefficient determined for each node. The resulting specific yield contour map is shown in Figure 13 and the corresponding digitized version is shown in Figure 14.

Aquifer Boundaries

The water levels of these wells were obtained from the Water Resources Board (1976). The 1969 and 1974 water levels and their well locations were transferred onto a quarter-mile grid and contoured. These maps are shown in Figures 4 and 15. Interpolated head values at the center of each node were used for the computer matrix.

The Hennessey shale underlies the aquifer as an impermeable boundary.



			PERMEASILITY	(GPD/FT.58.)				
	2 3 4 5 4 7 8 9 10 11 12 13	14 15	14 17 18	19 20 21	22 23 2	* 23 24	27 28 29	30 31 32 33 34 35
104	2							
	•							
NU.	, ,				. 1	•. 1••. 1••.	146. 146. 146. 1	
NON	¥ 4. ***********************************	*****	**********		167. 167. 14	e. 146. 146.	146. 146. 146. 1	**. 1**. 1**. 1**. 1**. 1**.
ROW	g		*****	133. 313.	313. 292. 22	·. 167. 146.	146. 146. 146. 1	
101	¥ • • • • • • • • • • • • • • • • • • •			542. 543.		4. 500. 313.	147. 144. 144. 1	46. 146. 146. 146. 146. 0 44.000
	·							
~-					•13. 151.14	2	346. 271. 146. 1	
NOW	g	.1042.1042.1	042.1042.1042.	1042.1042. 959.	1022-1313-14	9.1042. 834.	613. 313. 158. 1	··· 1
ROW	g • ••••• • • • • • • • • • • • • • • •	.1107.1230.1	251.1335.1418.	439.1459.1459.	1459.1480.137	6.1146. 938 .	730. 500. 167. 1	46. 145. **********************
ROM	# 10 ***********************************	.1146.1272.1	335.1397.1418.	1439.1459.1444.	1464.1459.125	1.1022. 455.	688. 500. 229. I	44. * *** * *** · · · · · · · · · · · · ·
	¥ 11 **********************************	.1022.1063.1	124.1188.1209.	1230-1230-1251.	1251.1167.10	4. 917. 750.		
NOW	12	. 792. 855.	938. 950.1022.	1022-1022-1000,	, 968. 938. 63	4. 750. 709.	543. 417. *******	
ROM	¥ 13 ***********************************		742. 792. 813.	413. 413. 772.	730. 467. 64	4. 584. 521.	417	
ROW	a 14 esementeres eseres eseres eseres eseres es 667, 467, 667, 688, 667	. 604. 543.	542. 5 64. 6 04.	563. 521. 480.	417. 417. JI	3. 271. 250.	147	*****
ROW	¥ 15 ***********************************	. 469. 300.	282. 229. 229.	187. 144. 125.	144. 292. 31	3. 187. 187.	209	
	-							
~								
ROW	¥ 17 ***********************************	. 511. 551.	500. 417. 301.	157. 157. 92.	146. 417. 50	3. 521. 521.	563.0 0000000000	·····
80 v	18 ************************************	71.	584. 511. 534.	500. 449. 313.	125. 417. 44	7. 730. 792.		
RDW	19	. 876. 750.	663. 628. 555.	547. 507. 354.	43. 313. 50	4. 772. 917.		**********
804	20 ************************************		701. 588. 549.	550. 525. 417.	209. 333. 50			
							•	. ,
RON	21	. 993. 876.	803. 588. 586.	550. 543. 584.	434. 354. 50	5. 640. 730.		
ROV	22 *********** 980.1167.1167.1251.1251.1251.1032.1126.1067.1000	. 774. 751.	701. 457. 340.	500. 563. 710.	513. 354. 50	». 	****	
RCN	23 ********* ***0.1175.1334.1459.1459.1459.1459.1459.1313. 845. 488	. 563. 500. 6	667. 667. 534.	500. 750. 938.	417. 354. 31	·		
10¥	¥ 24 ***********************************	. 54 3. 543.	·19. ·19. 507.	507. 457. 958.	521. 333. 31	3. 417		****
	29 111111111111111111111111111111111111		626. 626. 363.	•17. •2•. •L7.	• • 25 . • 38 . 21	2. 242		
NON	# 26 ***********************************		646. 626. 417.	333. 424. 917.	917. 626. 35	4. 292	******	****
ROW	# 27 ***** #34.1000.118#.1334.1251.1022. 792. 629. 626. 626. 626	750.	771. 709. 417.	250. 688. 917.	917. 855. 31	5. 292	•••••	
ROV	28 730. 938.1144.1334.1272.1105. 876. 709. 624. 626. 626	750. 1	134. 750. 417.	208. 417. 426.	626. 626. 41	7. 292. 292.	292	
80¥	2 424. 834.1105.1293.1334.1188.1000. 813. 404. 404. 404		626. 586. 208.	a. a. 107.	187. 187. 14	7. 292. 292.	292.9999999999999	
	30	. •17. •17.	•17. 313. 14 0.	o. o. o.	63. 129. 10	7. 292. 292.	292.00000000000000000000000000000000000	
ROM	# 31 DODDE 626. 626. 750.1346.1251.1334.1334.1042. 713. 521. #3		ø. ø. ø.	a. a. a.	213. 229. 25	0. 292. 417.	417	**********************
NON	32 ***** 424. 426. 742.1042.1167.1334.1374.1293.1042. 834. 424	04. 521.	a3. a3. 125.	2 08. 313. 396.	396. 292. 31	6. 417. AC4.	\$26 .* ····	****
-	33 ***** \$26. 521. 730. 934.1125.1272.1459.1459.1272.1146. 938	. 750. 584.	6. 8. 11.	354. 626. 626.	417. 417. 43	6. 424. 730.		
	W	780. 847						
~~~					• • • • • • •	••••		
ROW	/ 35 ***** 626. 626. 730. 938. 980.1105.1146.1188.1251.1105. 938	. 730. 521.	83. 208. 521.		417. 626. 83	•.•••••	******	*****************************
ROW	36 ******************* 626. 792. 855. 938.1042.1063.1105.1042. 896	. 709. 417.	83. 208. 417.	417. 417. 417.	626. 834.104	2		****
ROV.	37 ****************** 626. 667. 792. 834. 917. 938. 980. 959. 834.	24. 313.	83. 208. 417.	417. 480. 48 <b>8</b> .	917-1042-123	o. <b></b>		****
ROW	34 ************************************	. 521. 208.	<b>83.</b> 144. 417.	400. 626. 917.	1042.1251.129			
~	· · · · · · · · · · · · · · · · · · ·		··· ···	-04. 750.1000.	1230.1434.143			
RDW	1 40 ***********************************	. 313. 104.	83. 213. 436.	730. 938.1126.	1418-1459-139	7		*******
ROW	41 <b>899559565556555555555</b> 705. 629. 626. 626. 626. 626. 626. 626.	. 438. 417. 4	417. 417. 543.	e76.1042.1230.	1459.1355.110			*******
ADW.	42 ************************************	. 709. 688. 6	29. 646. 730.	·34.1146.1457.	1334 . 1334 . 102	2		
NOV	43 ************************************	. 1000. 1000. 10	00.1022.1042.1	251.1397.1459.	1126. 917.000		*****	***************************************
					1			
-	1000.1000.1000.1000.1000.1000.1000.1000.1000.	.1000.1000.10	00.1063.1146.1	355.1459.1313.	936. 730, ***			
PO#	45 ************************************		se. 730. 834.1	000.1042. 938.	730. 411.***			*************************
ROW	46 ************************************	. 417. 417. 4	417. 417. 542.	730. 772. 626.	542. 521. 52		•••••••	****
ROW	47 ************************************	208. 208. 2	208. 208. 417.	543. 438. 313.	354. 438. 43			****
	48 ADDRESS ADDR							
~		• • • • • •	<b>b</b> . <b>b</b> . <i>a</i> .	371. 200. 03.	313. 334. 33	••••••		
NON	49 ************************************	. 167. 83.	n. n. n.	a. a. a.	167. 208. 20			****************
NO.	30 ************************************	. 292. 167.	83. 83. 43.	B. 6). 6).	167. 167. 20		*****	
RON	51	417. 313. 1	67. 83. 83.	43. 43. 125.	167. 313. 62	. 709		
ROV	52 ····································		67. 83. 85.	B. B. 125	167. 417. 41		, 	*********************
-04	·	. 921. 313. 2	208. 208.	208. 208. 208.	208. 333. 83	•.1042.1042 <i>.</i>		
NOV	54 ************************************	· ·2 ·. · 17. 3	33. 292. 292.	313. 313. 250.	208. 208. 52	1. 626. 750.	750. 626.00000	
R0¥	55 ************************************	750. 459. 3	175. 354. 375.	417. 417. 417.	292. 208. 20	. 313. 208.	208. 208. ******	······
ROW	54 ************************************	917. 750. 4	26. 521. 521.	580. 424. 584.	417. 375. 29	2. 208. 208.	208. 208. ******	
	47							·
RON	54 ************************************	.1251.1167.10	84-1042-1042-1	042.1042. 917.	e34. 730. 62	6. 542. 626.	626.*********	••••••••••••••••
ROW	99 ***********************************	1251.1251.12	251.1167.1167.1	1	1042.1042. 83		834	
ROy	46 ····································	1251.1251.12	251.1251.1251.1	251.1251.1254.	1251.1251.104	2.1042. 1042.	1042	
NOW	4] ••••••••••••••••••••••••••••••••••••	1251.1251.12	\$1.1251.1251.4		1251.1251.125	1.1251. 1251.	1042	
101								

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Figure 11. Grain size distribution of cored samples from the Tia Juana Basin, California (after Johnson, 1967)







# Figure 13. Specific yield map



Figure 14. Digitized computer input of specific yield

						SPECIFI	IC VIELD IP	ER CENTS		<u>.</u>	
		• 10	11 12	13 14		16 17	18 19	20 21	22	23	24 25 26 27 28 29 30 31 32 53 34 35
NUW	,					********	*********	25. 24	. 26.	26.	26. 26. 26. 26. 26. 26. 26. 26. 26. 26.
NON		•••••						25. 27	. 27.	27.	24. 26. 26. 26. 26. 26. 26. 26. 26. 26. 26
ROW	5	•••••		••••••••••			*****	29. 29	. 29.	29.	28. 27. 26. 26. 26. 26. 26. 26. 26. 26. 26. 26
RGN		*******	*******	********	** *** *	*****		30. 31		31.	\$1. 30. 29. 27. 26. 26. 26. 26. 26. 26. 26. 26. 26.
ROW	7 ************************************	······		*******	*****	*******	********	31. 31	. 31.	ы.	31. 31. 31. 30. 24. 26. 26. 26. 26. 4.
PON.	•••••••••••••••••••••••••••••••••••••••	********	31. 31.	31. 31.	31.	м. м.	и. п.	31. 31	. ".	32.	32. 31. 31. 31. 29. 27. 26. 26. ******************************
P.08	9 ·····	•••• 32.	52. 32.	32. 32.	32.	32. 32.	32. 32.	32. 32	. 32.	32.	32, 32, 31, 31, 30, 27, 26, 26, ************************
ROW	10	•••• 31.	31. 31.	31. 32.	32.	32. 32.	32. 32.	32. 32	. 32.	32.	32. 31. 31. 31. 30. 28. 24.***********************************
ROW	11	м. м.	м. м.	м. м.	31.	31. 32.	32. 32.	32. 32	. 32.	32.	31. 31. 31. 31. 30. 28
RDW	12	31. 31.	31. 31.	н. н.	31.	м. м.	31. 31.	31. 31	. 31.	\$1.	31. 31. 31. 30. 30.*******************************
ROW	13	зі. зі.	31. 31.	31. 31.	м.	м. м.	м. м.	31. 31	. м.	31.	31, 31, 30, 30,
RDW	14	м. м.	31. 31.	31, 31,	31.	30. 31.	м. м.	30. 30	. 30.	30.	29. 29. 28. 27
ROM .	15 ************************************	31. 31.	31. 31.	31. 30.	29.	29. 28.	20. 27.	26. 26	. 26.	29.	29. 27. 27. 27. **************************
ROW	16 ************************************	м. м.	м. м.	30. 30.	28.	26. 24.	24 24.	29. 25	. 26.	29.	29. 29. 29. 33
NOV	17 ************************************	31. 31.	31. 31.	30. 30.	30.	30. 31.	29. 29.	29. 25	. 26.	н.	31. 30, 30, 31,***********************************
ROM	18 ************************************	31. 31.	31. 31.	31. 31.	3L. .'	31. 30.	30. 30.	30. 29	. 26.	31.	31. 31. 31
ROW	19 ************************************	31. 31.	31. 31.	31. 31.	м.	31. 21.	31. 31.	30. 29	. 25.	29.	31. 31. 31.*****************************
ROW	20 ************************************	31. 30.	30. 30.	31. 31.	31,	н. н.	м. м.	30. 30	. 27.	29.	30. 31. 31.*******************************
POW	21 ********** 31. 31. 31. 31. 31.	м. м.	31 31.	ar àr	31.	н. н.	31. 31.	н. н	. 30.	29.	30. 31. 31.*******************************
ROW	22 ********** 31. 32. 32. 32. 32.	32. 31.	32. 31.	м. м.	н.	31. 31.	30. 30.	31. 31	. 30.	29.	30. 31. 31
ROW	23 ********** 31. 32. 32. 32. 32.	32. 32.,	32. 31.	30. 31.	30.	м. м.	30. 30.	31. 31	. 30.	29.	30. 31.***********************************
ROW	24 ************************************	32. 32.	32. 31.	31. 31.	я.	м. м.	30. 30.	31. 31	. 30.	29.	29. 31.00000000000000000000000000000000000
40 M	25 ************************************	51. 31.	м. м.	м. м.	м.	м. м.	30. 30.	н. н	. 31.	30.	29. 29
ROW	26 ********* 31. 31. 32. 32. 31.	31. 31.	н. н.	31. 31.	я.	31. 31.	30. 29.	м. м	. м.	31.	29. 29. ********************************
ROW	27 ***** 31. 31. 32. 32. 32. 31.	31. 31.	31. 31.	31. 31.	31.	м. м.	30. 28.	31. 31	31.	н.	30. 29
ROW	28 ***** 31. 31. 32. 32. 32. 32.	31. 31.	п. п.	м. м.	я.	м. м.	30. 27.	30. 31	. 31.	м.	30. 29. 29. 29
ROM	29 ***** 31. 31. 32. 32. 32. 32.	м. м.	31. 31.	<b></b> . э.	31.	31. 31.	27. 25.	25. 27	. 27.	27.	27. 29. 29. 29
*D¥	30 ********* 31. 31. 32. 32. 32.	32. 31. ·	31. 30.	30. 30.	30.	30. 29.	26. 25.	25. 25	. 25.	26.	24. 29. 29. 29
RON	31 ***** 31. 31. 31. 31. 32. 32.	32. 31.	31. 30.	25. 25.	25.	z. z.	25. 25.	25. 25	. 27.	20.	29. 29. 30. 30
ROW	32 31. 31. 31. 31. 32. 32.	32. 32.	31. 31.	31. 31.	30.	25. 25.	26. 27.	29. 30	. 30.	29.	30. 38. 31. 31
ROW	33 31. 31. 31. 31. 32. 32.	32. 32.	32. 32.	31. 31.	я.	25. 25.	29. 29.	м. м	. 30.	30.	30. 31. 32. 31
ROM	34 ***** 31. 31. 31. 31. 31. 32.	32. 32.	32. 32.	31. 31.	30,	25. 27.	30. 31.	31. 30	. 30.	30.	31
ROW	35 ***** 31. 31. 31. 31. 31. 31. 31.	32. 32.	32. 31.	31. 31.	30.	23. 27.	30. 31.	25. 30	. 30.	м.	31
ROW	34 ************************************	м. м.	м. м.	н. н.	30.	25. 27.	30. 30.	30. 30	. м.	31.	31, ************************************
ROW	37 31. 31. 31. 31.	м. м.	м. м.	31. 31.	29.	29. 27.	30. 30.	30. 31	. 31.	31.	32, ************************************
ROW	38 ************************************	31. 31.	31. 31.	31. jo.	27.	25. 27.	30. 30.	н. н	. <b>3</b> 1.	32.	32
ROW	39 31. 31. 31.	м. м.	н. н.	31. 30.	25.	25. 25.	30. 31.	м. и	. м.	и.	32_*****
ROW	40 ************************************	м. м.	31. 31.	31. 29.	25.	25. 27.	30. 31.	31. 32	. 31.	32.	32
ROW	41 ************************************	30. 31.	м. м.	31. 30.	30.	30. 30.	м. м.	31. 32	. м.	32.	32.*****
POW	42 ************************************	м. м.	н. ж	31. 31.	31.	31. 31.	31. 31.	32. 32	. m.	32.	31,*****
POU	43 ************************************	n. n.	м. м.	31. 31.	31.	м. м.	31. 32.	32. 32	. 32.	м.•	• • • • • • • • • • • • • • • • • • • •
80¥	4	м. м.	31. 31.	31. 31.	м.	31. 31.	N. N.	32. <u>3</u> 2	. 31.	31.*	********
ROV	45 ····· 31. 31.	м. м.	м. н.	31. 31.	31.	м. м.	м. м.	31. 31	. м.	м.•	······································
ROW	46 ************************************	м. м.	31. 30.	30. 30.	30.	30. 30.	30. 31.	31. 31	. 30.	30.	30
ROM	47 ************************************	31. 30.	29. 27.	27. 27.	27.	27. 27.	30. 31.	. 30, 29	. 29.	30.	30
ROW	48 ************************************	30. 29.	27. 25.	25. 25.	25.	25. 25.	28. 30.	27. 25	. 29.	29.	29
RON	.,	31. 30.	30. 29.	27. 27.	25.	23. 23.	25. 25.	25. 25	. 27.	27.	27,
ROM	50 ************************************	я. я.	31. 30.	30. 29.	27.	25. 25.	v. v.	25. 25	. 27.	27.	27
ROW	51 ************************************	м. м.	м. м.	30. 30.	29.	27. 25.	25. 25.	29. 26	. 27.	29.	31. 31,
ROM	52 **** *******************************	м. н.	з. э.	м. м.	н.	27. 25.	25. 25.	25. 26	27.	30.	31. 32
RDW	53 ************************************	я. я.	м. м.	31. 30.	29.	27. 27.	27. 27.	27. 27	. 27.	29.	31. 31. 31.····
RON	54 ************************************	30. 31.	31. 31.	м. м.	30.	н. н.	29. 29.	29. 20	27.	27.	30. 31. 31. 31. 31.
ROY	55 ************************************	32. 32.	, 32. 31.	н. н.	30.	29. 29.	29. 30.	30. 30	. 29.	27.	27. 29. 27. 27. 27. *****
PON	56 ************************************	32. 32.	32. 32.	32. 31.	п.	31. 30.	30. 30.	30. 30.	. 30.	29.	29. 27. 27. 27. 27
40¥	57	32. 32.	32. 32.	32. 32.	я.	м. м.	м. м.	31. 31	. м.	30.	29. 29. 30. 30
RDW	st	32. 32.	32. 32.	32. 32.	32.	32. 31.	н. э.	м. м	. м.	м.	31. 30. 31. 30
ROW	59	32. 32.	32. 32.	32. 32.	32.	32. 32.	32. 32.	32. 31	. 31.	ы.	31. 31. 31. 31.
POW	•••••••	32. 32.	32. 32.	32. 32.	32.	32. 32.	32. 32.	32. 31	. м.	м.	31. 31. 31. 31
RON	61 ************************************	•••••	32. 32.	32. 32.	3 <b>2</b> .	32. 32.	32. *****	32	32.	32.	32. 32. 32. 31
ROW	62 ***********	•••••	•••••								
100	AN 20 YR. RUN 4/23/78 RECH-PATT, WELL-	+36 .51 44	6078								

#### TILLMAN TEARACE AQUEPER

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# Figure 15. 1974 water-head elevations



Figure 16 shows a representative Hennessey shale outcrop. It generally outcrops in the eastern half of Tillman County. The eastern boundary of the aquifer model was determined by using the topographic and geologic maps of Tillman County and verified by field investigation. The bedrock elevations for the study area under the terrace were obtained from the driller's logs. These data were contoured as subdatum elevations and assigned to the center of each node in the one-quarter mile grid. These data served as the lower boundary of the model. Contoured and digitized maps of the bedrock elevations are shown in Figures 17 and 18, respectively.

The Red River forms a discharge-recharge boundary at the southern and southwestern edge of the aquifer model. The North Fork of the Red River serves as a recharge boundary on the northwest side of the aquifer and as a discharge boundary on the north edge. The discharge-recharge relationship is time dependent to the configuration of the water table with respect to the river shown in Figures 4 and 15.

Land elevations were assigned to the quarter-mile grid by using a U. S. Geological topographic map. This information was used to establish a reference from which depth of evapotranspiration was measured. A printer output showing digitized values of "land" elevations is shown in Figure 19. The "top" elevation is a parameter incorporated in the program for denoting the top of a confined aquifer. A confined aquifer was assumed for the river boundary condition in the node. "Top" values, equivalent to two feet below land elevations, were used for all river nodes, whereas a value of 20,000 feet was applied to all other nodes. Other variables which were used in the computer program were thickness and hydraulic conductivity of the river bed. It was assumed that the

Figure 16. Hennessey shale with terrace alluvium gravels, Sec. 33, T2S, R18W



## Figure 17. Bedrock elevation map



Figure 18. Digitized computer output of bedrock elevation

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		,	,														104			•								
					· ·		•															<b>36 17</b>	20	29	30	31	u »	34. 33
ROW																												
804																						.1275		1305.1			····	1370.1390.
																								1278.1			•7.1354.	1378.1396.
																	1209.120		1265.	1265-1	265.11	\$5.1275.	.1275.	1242.1	\$10.1	124.13	49.1344.	1341.1408.
	÷.																					. 12 70.	1275.	1289.1			58.1377.	1370.0000
804											. 1265.	1244.1	249.12			. 1241		0.1245	1209.	1239-1				1240.1				
											.1747	1248.1											1280.1		20.1			•••••
804	10								3.124		. 1262.	1267.1	2 6 2 . 1 2											1305.1	134.1	.,,		
-							** ** 17	40.174	0.12	40.1255	. 1282.	1241.1	282.12			. 1240	240.174											
ROW	12								4.12	5-1250	. 1247.	1245-1	240.12		240.124	2-1250	248-127			1120.1	120.11							
					****			54.125	3.12	51.1247	. 1240.	1235.1	230.12	30.12		7.1295.1	270.128			1330-1	330-13	20.1325						
-	14	••••						4.129	1.12	50.1762	. 1211.	1775.1	230.12	40.13	10.125	7.1270.1	785.130	0.1305	1125	1330.1		20.1176						
-	19					•1244.1	244.12	44.124	4.124	5. 1235	.1230.	1235.1	251.120	60.12	70.128		300.131	0.1312	1920.	1330.1	130.11	24.1925.						
ROW	16	•••••				1244.1	244.12	41.124	0.123	5.1225	.1230.	1235.1	255.12	70.12		5.1305.1	310-131	4.1320.	1320.	1330-1	330-13	24.1330.						
ROW	17	••••			••1241	. 1 2 4 1 . 1	238.12	37.123	0.122	20.1230	.1245.	1244.1	268.12	15.12	292.130	3.1310.	1345.132	0.1320.	1320.	1327.1	330-13	30.1376.	•••••					
RON					₩1239.	.1239.1	238.12	34.122	7.12	. 1229	. 1241.	1255.1	271.12	42.13	901.131		317.133			1330-1	330.13	24.0000						
ROW	19	••••		•••••	+1238	.1236.1	235.12	33.122	3.121	.1220	.1238.	1265.1	275.12	90.13	510.132	0.1320.	320.133	5.1350.	1345.	1340.1	330-13	24	•••••					
ROM	20				1.1233	. 1 2 30 . 1	225.12	25.122	2-121	4-1218	.1230.	1249.1	240.12	80.13	07.131	0.1320.1	320.133	5.1345.	1345.	1335.1	330.13	22.0000						
*0¥	21	•••••		a. 1220		.1217.1	217.12	18.122	0.121	5.1215	.1220.	1240-1	2 54. 12	75.12		0.1320.	320.133	0.1340.	1336.	1325.1	320.13	20.0000						
ROW	22		*****12	. 122	.1215	. 1 210. 1	212.12	13.121	5.121	10.1207	. 1 2 2 9.	1238.1	257.12	75.12		0.1315.	320.133	0.1330	1330.	1320.1	320.13	20	••••••					
-	23	•••••	*****1 21	12.122	.1215	.1209.1	207.12	08.120	.120	5.1210	.1220.	1239.1	2 75. 12	15.12	. 130	0.1310.	320.132	5.1325.	1320.	1320.1	320.00		•••••					
ROW	24	•••••		•• 122	5.1217	. 1 2 1 3 . 1	205.12	, 01.120	0.121	10.1217	.1225.	1235.1	250.12	70.12	85-129	5.1310.1	320.132	0.1320.	1320.	1320.1	320		•••••					
RON	25			•• 1225	5.3215	1207.1	203.11	99.120	3.12	12.1220	. 1227.	1239.1	247.12	<b>44.</b> 12	80.129	5.1305.1	310.131	. 13 19.	1314.	1320.1	323,00							*******
ACH	26	•••••	*****12	5. 1220	0.1210	1205.1	200.11	97.120	0.121	0.1220	.1227.	1235.13	245.120	60.12	75.129	5.1305.1	310.131	3.1316.	1314.	1320-1	125							
RON	27		1223.12	0. 1215	.1207	1 200 . 1	197.11	97.120	0.121	3.1217	. 1225.	1233.1	240.12	57.12	70.129	.1299.1	310.131	9.1314.	1 320.	1328.1	330.**		•••••				******	*******
RON	28	•••••	1220-121	5. 2210	-1200.	1198.1	196.11	16.120	0.121	0.1215	.1220.	1225.1	2 39. 124	45.12	140.12T	5.1290.1	300.131	0.1310.	1312.	1320.1	330.13	38.1340.						
*0¥	79	•••••	1220.12	0. 1200	. 21 97	.1193.1	195.11	96.120	0.120	7.1213	.1217.	1225.1	235.12	45.12	.127	.1280.	299.130	0.1310.	1310.	1320.1	323-13	M.1335.	•••••		•••••			
ROV	30	• • • • •	•••••125	0.1295		.1 192 . 1	195.11	97.120	3.120		. 1220.	1240.1	255.120	60.12	19.127	5.1280.1	290.129	5.1300.	1910.	1310-1	315-13	20.1327.						
101	31	•••••	1207-11	5. 1190	. 11 89.	.1195.1	198.12	03.120	7.121	5.1224	.1240.	1260.1	275.12	<b>13.</b> 12	90.129	.1290.1	290.129	5.1300.	1305.	1310.1	310-13	10.1310.	•••••					
#Dw	12	•••••	1200-11	0.1181		.1195.1	203.12	08.121	5.122		. 1260.	1280.1	2 90. 1 2	<b>10.1</b> 2	90.129	.1290.1	290.129	.1295.	1300.	1300.1	300-13	05.1305.	••••••	•••••	•••••	*****		******
ROY	33	•••••	1192.114	. 1165		.1195.1	205.12	15.122	0.123	5.1250	.1265.	1275.14	290.121	<b>90.</b> 12	90.129	.1290.1	290.129	.1290.	1294.1	295.1	300.13	00.1300.	•••••		•••••			
P.Dar	ж	•••••	1187.110	9.1142	2.11 87.	1195.1	203.12	10.121	a.122	25.1230	. 1245.	1260. Li	280.129	90.12	90.129	.1290.1	285.128	5.1285.	1286.	1290.0	•••••		•••••					******
<b>RO M</b>	35	••••		0.1180		1192.1	200.12	05.120	.151	3.1223	.1230.	1250.1	275.12	90. 1 Z	90.129	0.1290.1	280.128	.1280.	1280.	245.0	•••••		• • • • • •		••••		******	*******
ROW	34	•••••		** 1177	. 12 83.	1188.1	194.11	. 120	3.120	7.1209	.1220.	1245.13	264.129	10.12	90.129	.1290.1	280.128	.1279.	1278.	.280	•••••		•••••					
ROM	37	•••••		** 1177	-1180.	1 100 - 1	191.11	95.120	3.120	. 1215.	. 1220.	1240.13	260.127	15.12	85.129	. 12 90 . 1	263.126	.1278.	1276.	274.0			••••••		•••••	•••••		******
ROW	38	•••••		•••••	*11 80.	1105.1	190.11	95.120	3.120	9.1213	.1220.	1235.14	250.124	45.12	70.128	.1290.1	283.128	. 1276.	1272.	266.*	••••••	••••••	•••••		•••••	****	******	*****
NOV	39	•••••			<b>•1177</b> .	1 143. 1	198.11	95.120	1.120	5.1207	.1210.1	1225.11	240.125	50.12	65.127	.12 00.1	280.128	.1275.	1248.	1260.0	•••••		•••••	••••	•••••		•••••	
ROW	40	••••		•••••	***	1 180. 1	105.11	•0.11*	.120	0.1200	.1200.	1212.13	2 25. 124	40.12	55.126	0.1275.1	280.128	. 1272.	1250.	245.*	••••••		•••••					******
RON	41	•••••			*****	1177-1	182.11	8.119	2.119	5.1194	.1198.1	1210.14	23.124	40.12	50.126	.1270.1	275.127	. 1250.	1250.	233.0	•••••	••••••	•••••		••••	•••••		******
RON	42	•••••		•••••		1175.1	160.11	5.118	7.119	0.1192	.1198.	1215.12	2 50. 124	40.12	50.126	0.1240.1	270.124	.1250.	1225.	225.0	•••••		•••••		••••			
ROV	43	•••••			•••••	1170.1	175.11	0.118	2.118	5.1190	1200-1	1220.1	240.124	45.12	50.126	. 12 60. 1	260.126	.1239.	1220.4	•••••	•••••		•••••	•••••	••••	** ** **		******
RON	*	•••••		•••••	*****	1148.1	172.11	***117	9.11a	5.1195	. 1210.	1230.12	245.124	47.12	51.125	.1240.1	255.124	.1220.	1200.0	••••			•••••		••••		•••••	*******
ROM	45	•••••			••••	1165.1	169.11	73.117	7.11	5.1200	. 1215.	1235.12	242.124	45.12	48.124	5.1240.1	235.122	.1210.	1190.4		•••••		•••••	*****	••••	•••••	******	
NON	**	•••••		•••••	*****	1 165.1	169.11	72.110	0.11	5.1200.	.1215.1	230.13	**1.124	42.12	43.123	.1220.1	213.120	.1197.	1185.1	192.0			•••••		••••	*****	•••••	******
ROW	47	•••••		•••••		1 140.1	167.11	75.118	0.118	3.1196	.1215.	1275.12	2 35. 1.24	41.12	42.123	.1210.1	195.119	.1185.	1177.1	185.0	•••••		•••••		••••		•••••	
ROW	48	•••••		•••••	***	1155.1	165.11	75-118	0.118	2.1185	.1200.1	1215.12	25.123	30.12	21.121	.1205.1	105.117	.1175.	1170.	180.0	•••••	•••••	•••••	*****	••••	•••••		
ROW	•	•••••	••••••	•••••	*****	1152.1	160.11	12.117	5.117	7.1177	. 1188. 1	1200.11	210, 121	10.12	00.119	. 11 95 . 1	145.117	.1170.	1165.1	175.+	•••••	••••••	•••••	•••••	•••••	••••	••••••	
ROM	50	•••••		•••••	*****	1145.1	155.110	5.117	0.117	1.1173.	.1177.1	1188.11	92.119	90.11	#2.118	2.1185.1	180.117	.1165.	1160.1	165.0		•••••	•••••	*****	••••	•••••		*******
ROW	51	•••••	•••••	•••••	*** ***	1139.1	145.11	53.116	0.114	3.1163.	. 1170.1	1175-11	75.114	5.11	45.1145	.1170.1	173.110	.1160.	1155.1	157.1	145.**	•••••	•••••	*****	••••	*****	******	******
908	2	•••••		•••••	••••	1125.1	130.11	.114	5.115	5.1158.	.1165.1	1168.11	155.114	•7.11	50.114	. 1145.1	150.115	.1150.	1148.1	140.1	39.00	••••••	• •• •• •	*****	••••	•••••	••••••	*******
RDW	53	•••••		•••••	*****	1 1 20. 1	120.11	25.113	5.114	7.1153.	.1263.1	1160.11	50.114	.0.11	34.113	.1135.1	133.113	2.1130.	1125.1	125.1	135.11		•••••		••••	*****		
RON	*	•••••		•••••	*****	1116-1	116-113	21.112	5.113	8.1148.	.1155.1	1152.11	34.113	35.11	33.1130	. 11 25 .1	125.112	.1120.	1115.	115-1	120-11	35.1143.	1145.0	*****	••••	*****	*****	
RON	55	•••••		•••••		1112.1	110.111	5.112	0.112	5.1135.	.1140.1	138.11	30.112	25.11	22.1120		116.111	.1114.	1114.1	105.1	117.11	25.1140.	1142.4		••••	•••••		
*0*	54	•••••		•••••	*** * **	1 107.1	110.111	0.111	5.112	0.1125.	1125.1	1120.11	10.111	15.11	13.111		110.111	.1100.	1098.1	100.1	105.11	15.1137.	1139.0	*****	••••	*****	******	********
*04	57	•••••		•••••	•••••	1103.1	105.110	5.1110	0.111	5.1120.	.1115.1	1125.11	10.111	10.11	10.1110	.1110.1	110.110	. 1100.	1090.1	095-1	100-11	08.1125.	•••••		••••	•••••	******	
ROY	8	•••••		••••••	•••••			00.110	0.111	0.1110.	1110.1	1110.11	10.110	05-11	05.110	.1103.1	130.110	.1090.	1090.1	090.1		00.1117.	•••••					
	»"						10					100.11	30.110					.1090.	1090.1	J40.1		00.1105.						
*0*	•••						10				1100.1					. 1095.1		-1090.	1090.1	090.1								
	•1								101	w1095.	1095.1				-5.1005			.1390.	1090.1			-0.1095.						
- 04	~																											

Figure 19. Printer output of digitized land elevations

-----2 3 -4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21 22 25 24 25 26 27 28 29 30 31 32 33 34 35 ---------1244, 1272, 1275, 1275, 1285, 1295, 1300, 1295, 1300, 1315, 1325, 1345, 1345, 1375, 1376, 1376, 1376, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1345, 1 NON 13 *** 80 23 **********1242, 1235, 1245, 1246, 1250, 1255, 1255, 1240, 1265, 1275, 1290, 1305, 1315, 1330, 1340, 1355, 1344, 1370, 1370, 1372, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 1375, 137 R0w 27 *****1230.1230.1240.1240.1240.1240.1240.1240.1245.1255.1270.1274.1274.1274.1287.1295.1305.1313.1350.1350.1360.1345.1345.1345.1350..497.*** ROM 30 *********1221.1229.1225.1226.1224.1224.1240.1240.1240.1246.1276.1246.1276.1315.1324.1345.1345.1350.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.1357.1354.135 ----

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river bed consisted of silts and clays and thus served as a local aquitard to the underlying aquifer material. Therefore, the river and nodes were used to represent vertical seepage movements in either direction. Water-level elevations for the river nodes were also obtained from the topography map.

A value of 0.01 was placed at the outer edge of all aquifer boundaries on the water-head elevation matrix (STRT). The value actually represents the areas where transmissivity is equivalent to zero. The specific yield and bedrock elevation matricies were left blank outside of these same boundaries.

Pumping rates were entered as a variable in the model. Two matricies were used. One matrix included pump rates reportedly used prior to July 1, 1973. These were established by the Oklahoma Water Resources Board as prior appropriative right owners. Distribution of these rates is shown in Figure 20. A second matrix was used to enter a constant pumping rate which was assigned to all nodes other than those with prior appropriative rights. An example of this matrix is shown in Figure 21. Those prior appropriative pumping rates with less than the assigned constant value were automatically assigned the larger rate. All other prior appropriative pumping rates remained unchanged. Prior appropriative ownership rates were converted from acre feet per year for the number of permitted acres to acre feet per acre per year and cubic feet per second. The annual rate was restricted to a four-month pumping session between June 1, and October 1 (one-third year). This required the annual rate to be increased by three times for the shorter period. All pumpage rates include a net return flow of 15 percent of total pumpage.

Figure 20. Distribution of prior appropriative right ownership and corresponding pump rates (pattern recharge)


Figure 21. Prior appropriative and allocated pumping rates

;		n an
-		PRICE APPROPRIATION AND ALLOCATION OF 3.46 ACRE FOOT
ł		ARRUAL PURPAGE (ACAE FEET)
	·	2, 3 4 5 6 7 8 1, 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 10 31 12 33 34 35
	ROW	2
	ROW	3 *************************************
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		5 0.94 0.94 0.94 0.94 0.94 0.94 0.94 0.94
	-	6 ************************************
	ROW	7
		8 ************************************
	RGN	***************************************
	LOW	19
		11
		12
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	ROV	14 ····································
	ROW	17 ************************************
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	* D*	17 ************************************
	ROW	28
	ROW	21 ••••••••• 5.46 7.46 8.46 8.46 8.46 8.46 8.46 1.46 7.47 1.56 8.46 8.46 8.46 8.46 8.46 8.46 8.46 8.4
		27 ********* 0.46 0.76 0.46 0.46 0.46 0.46 0.46 1.19 7.05 0.46 3.46 0.46 0.46 0.46 0.46 0.46 0.46 0.46 0
	ROW	29 ********* 0.14 0.14 0.44 0.44 0.44 0.45 0.31 7.14 0.14 0.14 0.14 0.14 0.14 0.14 0.14 0
		24 ************************************
	RON	25 ************************************
		24
		27 ***** 0.46 0.46 1.04 1.00 1.25 0.46 1.40 3.56 0.46 0.46 0.46 0.46 1.64 0.46 0.46 0.46 0.46 0.46 0.46 1.51 0.56 1.51 0.56 1.55
		28 ***** 8.76 8.76 9.76 9.76 9.76 1.77 8.96 9.76 9.76 9.76 9.76 9.76 9.76 9.76 9
		29 ***** 0.96 3-16 3.96 3.46 3.96 1.87 1.16 0.16 1.46 1.46 5.96 3.96 0.96 3.46 0.16 3.96 0.16 3.96 3.16 0.96 3.16 0.96 3.16 0.96 3.16 0.96 3.16 0.96 3.17
	ROW	39 ********* 3-76 0-76 9-76 9-76 9-76 1-24 0-76 1-24 0-76 0-76 0-76 0-76 0-76 0-76 0-76 0-76
	-	31 0.46 0.76 0.76 0.46 0.46 0.46 0.46 0.46 0.46 0.46 0.4
	•0×	37 ***** 0.46 0.46 0.46 0.46 0.46 0.46 0.46 0.46
	ROV	33 ***** 0.4% 2.33 J.38 J.4% 5.4% 0.4% 0.4% 3.4% 0.4% J.9% 3.4% 0.4% D.9% 0.4% 3.4% J. 17 3.1% 0.4% 0.4% 0.4% 0.4% 0.4% 0.4% 0.4% 0.4
		34 40400 0.16 0.75 1.00 0.45 0.46 0.46 0.46 0.46 0.40 0.40 0.40 0.40
		35 ***** 0.14 1.00 1.00 0.14 0.44 0.44 0.44 0.44
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		W embernation 5.44 5.44 5.44 5.44 5.44 5.44 5.44 5.4
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	,#0¥	1 ····································
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		54 ************************************
	80¥	58
	ROW	41 ************************************
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	m	IN 20 TR. NUM 4/23/78 NECH-PATT, WELL-936 .51 (407)

### Simulation Period

The model was used to simulate pumping and corresponding water-level changes over a five- and twenty-year period. This is shown in the flow chart on Figure 5. The model was originally run between October 1, 1969, and October 1, 1974, in order to calibrate the model. The second period was changed to the interval between July 1, 1973, and July 1, 1993. The latter change was made because the Oklahoma Water Law No. 82 § 1020.5 requires that new allocations are to be assigned for a 20-year period between these two dates:

The five-year calibration period included only prior appropriative The 20-year simulation included two separate runs; one using pumping. prior appropriative pump rates only and a second using prior appropriative rates combined with constant rates assigned to all other nodes. The model was designed to automatically turn the pumping period on or off at the beginning and end of each pumping period, respectively, for either the five-year or twenty-year periods. Therefore, ten pumping periods (five periods with pumps on and five periods with pumps off) were used for the five-year calibration period and 40 pumping periods were employed for the 20-year simulation. Simulated withdrawal of water was designed to automatically cease if the water-head elevation dropped to an elevation within five feet of the bottom (bedrock). It was assumed that a submersible pump would be placed within the bottom five feet. Pumping would cease as the water head dropped within this interval because air would be drawn and, consequently, eliminate the lift capacity of the pump.

Data Input

The input data were digitized by punching values assigned to each node onto computer (IBM) cards. These data included other constant values punched as separate input data: QET--evapotranspiration rate; ETDIST--depth (1 foot) at which evapotranspiration ceases below land surface; ERR--error criteria for convergence of the mathematical solution (0.1 foot); ITMAX--maximum number of iterations per time step (50); NPER--number of the pumping perids; SSRIV--specific storage of river bottom; NUMT--number of time steps in pump period (assume time step of ten days); TMAX--number of days of the pumping periods; DELX--grid spacing in X-direction (2,640 feet); and DELY--grid spacing in Y-direction (2,640 feet); S--storage coefficient for river nodes only (2.0 x  $10^{-8}$ ); DIML--number of rows used in the model (63); and DIMW--number of columns (36). Other input cards were followed by variables entered as matricies: LAND--elevation of land surface; TOP--elevation of top of aquifer or the top of the bedrock; PERM--coefficient of permeability; SY--specific yield which ranges from 0.245 to 0.320; STRT--the 1969 water-table elevations; RATE--hydraulic conductivity of river bed; M--thickness of river bed; QRE--recharge used in calibration; and WELL--pump rates used when pumping Complete listings of the data input is shown in the Appendix. is on.

#### Calibration

Calibration was achieved by comparing the 1974 observed head (water table) elevations with the computed values from the five-year computer simulation between October 1, 1969, and October 1, 1974 (Figure 5, Flow Chart). Recharge (QRE) was adjusted in order to reduce the calibration

error or residual values between observed and computed head elevations to ± five feet. Three approaches were used for calibration which included matrix, constant and pattern recharge. Where the matrix was used, recharge (QRE) was adjusted for each node. Matrix recharge values and the resulting calibration error are shown in Figures 22 and 23. In the constant recharge version, the mean of the matrix recharge values was used as a constant recharge for all nodes  $(7.5 \times 10^{-9})$  feet per second). Pattern recharge included the mean of matrix recharge values which were selectively assigned to nodes. The pattern recharge concept was introduced to eliminate some errors in recharge that might have been introduced in the calibration recharge version. This approach involved the adjustment of the rate of recharge for the calibration matrix nodes in such a way that nodes with negative recharge values were replaced by 0 or positive recharge values. Although the residual error could not be restricted to within  $\pm$  five feet, the resulting error distribution was indicative of areas where varying degrees of reliability could be assumed in the simulation results. This error distribution is shown in Figure 24. The error represents the total effect of inaccuracies which may be associated with the input parameters or the assumptions used. However, the mean value of all matrix recharge values was 2.87 inches per year as compared to 2.2 inches per year estimated by nine percent. The similarity between calculated and estimated recharge is indicative of a successful model calibration.

Some of the 1969 and 1974 water-head elevations located near the bedrock boundaries occurred below bedrock elevations. They were treated as a boundary condition for the model in the matrix and constant versions. Some of the nodes started dry and became wet after five years of pumping.

Figure 22. Recharge values in inch/year (matrix recharge version)

	70

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		TILLRAM TERRACE AQUIPER	
	۰	LIBRATION (INCHES PER YEAR)	÷
	2 3 4 5 6 7 8 9 10 11 12 13 14 15	16 17 18 19 20 21 22 23 24 25 26 27 28 29 30	31 32 33 34 35
	MCH 2 ***********************************		0. 0. 0. 0. 0.
	1011 3 *********************************	**************************************	. 0. 0. 0. 0. 0.
	10 u 4 10		o. o. o. e. o.
			0. 0. 0. 0. 0.
	Nov 6		0. 0. 0. 0.*****
	Ngu 7 +++++++++++++++++++++++++++++++++++		0. 0
	ROL 1 ***********************************	0. 0. 0. 0. 0. 3. 2. 2. 1. 0. 1. 1. 0. 1. 0.	0
	Ray 9	3. 2. 1. 1. 2. 2. 3. 2. 0. 0. 0	0
	10w 10	4. 2. 1. 1. 2. 4. 4. 10. 310. 2. 3. 13. 9. 0.	
	•11 · · · · · · · · · · · · · · · · · ·	0. 1. 1. 0	
•			+
	MOW 14 ***********************************	1. 3. 3. 2. 14. 6. 6. 2. G. 16. 13	
	NCW 15 ***********************************	4. 1. 3. 3. 2271397. 5. 12.00000000000000000000000000000000000	**********************
	RDM 16 ***********************************		************************************
	RTH 17 ***********************************	-915119. 13. 4. 4. 4. 5. 17. 39.00000000000000000000000000000000000	
	MON 18 ********************************* 0. 2. 1. 1. 0. 0. 00. 154. 27.	19. 26. 6. 9. 8. 2. 6. 14. 13. 19. 15	*************************
	ROM 19 *********************** 0, 2, 3, 2, 0, -4, -19, -23, -8, 38,	19. 28. 12. 10. 12. 11. 4. 3. 4. 125.***********************************	***************************************
	#DM 20 ************ 0, 0, 5, 5, 3, 0, -4, -8, -25, -14, -15, -	12. 32. 1. 8. 12. 10. 3. 273438	**********************
	RDW 21 ********* 0. 0. 41. 6. 4. 3. 048119. 15	1419. 0. 6. 6. 13. 6. 3. 4. 12.***********************************	**** * * * * * * * * * * * * * * * * * *
	MOW 22 ********* 0. 5. 5. 9. 2. 33. 048. 0. 0. 7	92. 2. 4, 11. 13. 10. 5. 4. 12. 22.0000000000000000000000000000000	********
	ROW 23 ********* 0. 0. 3. 211232. 82. 1	23723. 31. 7. 4. 8. 5. 36	****
	MOW 24 ************ 0. 213. ++45. 0. 6. 312.	-45412. 13. 4. 34. 8. 5. ********************************	****************
	NOW 25 ***********************************	32416. 13. 3. 3. 11. 29	
	NOW 24 ********* 0. 0. 4. 11434322. 0, -	0. 32. 110. 1. 5. 4. 1. 1.******************************	
	104 27 ***** 0. 3. 4. 5. 4. 12245342:	222. 0. 0. 2. 2. 303	*********************
	MOW 28 ***** 0, -2. 7. 4. 5. 11145543.	713. 3. 4. 4. 3. 5. 1. 2. 7. 13	****************
	MON 29 ***** 0. 0. 3, 5. 5. 2. 1. 024656.	444. 2. 4. 2. 5. 703. 5. g.e	
	NOW 30 ******** 0. 2. 2. 2. 2. 2110642.	1230. 4. 4. 5. 1. 2. 4. 9. 6	******************
	KOW 31 ***** 0. 0. 42. 1. 3. 3. 2. 1. 123. 2.	4. 0. 2. 3. 7. 6. 0. 5. 3. 6. 13. 10.***********************************	
	ROM 32 ***** 0. 3. 1381. 3. 2. 3. 14. 0. 0. 0. 4.	3. 3. 6. 6. 62. 3. 9. 6. 7. 15. 3	
•	ROW 33 ###### 04. 30. 0. 2. 5. 0. 0. 0. 0. 0. 0.	0. 0. 0. 0. 0. 0. 8. 9. 7. 8. 2. 3	
	NOW 34 ***** 0. 212. 1. 1. 3. 2. 10. 4. 0. 0.	0. 0. 0. 0214. 13. 15. 8	***************************************
	RDM 35 ***** 0. 0. 00. 01. 2. 4. 2. 5. 11. 3. 0.	0. 0. 000. 3. 5. 10. 4.***********************************	
,	MCW 36 ************** 027. 0015. 2. 2. 12. 3. 0.	0. 0. 0. 0. 3. 10. 7. 5. 0	
	#D# 37 ************ 0. 0. 125. 0. 3. 5. 4. 2. 0.	0. 0. 0. 8. 6. 27. 0	**** **** * * * * * * * * * * * * * * *
· ,	ROW 34 ***********************************	0. 0. 0. 0. 0. 10. 303.***********************************	****
	RDM 39 ***********************************	0. 0. 0. 0. 0. 0. 3. 0. 0	
4	kow 40 ***********************************	۔ ۱. –۱. ۱. ۵. ۵. ۵. <b>4. –۹. –۱4.*************************</b> ***********	****
	RD¥ 4] ***********************************	1. 5. 31. 0. 4. 3330.************************************	
	RO# 42 ***********************************	2373. 0. 402537.000000000000000000000000000000000000	
	RDW 43 ***********************************		
÷	ROW 44 990090009 49000000000 0, -2, -0, -1, 1, 3, 4, -4, -3, -	·····································	
	ROW 45 ***********************************	0. 0. 0. 0. 0. 7. 2023	
	RDw 46 ***********************************	J. D. D. DLB. 4. 5. 5. *****************************	
	104 47 **********************************	0. 0. 0. 0521. 6. 0	· · · · · · · · · · · · · · · · · · ·
	RDW 48 ***********************************	0. 0. 0. 01. 1. 1. 3. 0	
	804 49	0. 0. 0. 0. 5. 10. 5. 0. <b>0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0. 0</b>	
	RDN 50 ***********************************	0. 3. 0. 0. 0. 2. 2. 0.00000000000000000	
	ROW 51 ***********************************	<b>a.</b> 3. 02. 0. 0. 011. ^	
	RDW 53 ***********************************	+- +- +- +- +	······································
	NUM 35 0. 3. 4. 6. 12. 32. 43. 3021	03, 3, 2, 1, 2, 1, 1, -1, -3, -0, 0, 0,**********	****
	RUM 35 ***********************************	4. 13. 9. 7. 6. 12. 10. 3. 4. 4. 2. 1.444444	
· ·	KUW 59	<b>6.</b> 11. <b>6.</b> 7. 5. 6. 5. 2. 3. 4. 5. 4	****
	NCM 60	8. b. 0. 0. 0. 0. 7. 7. 14. 14. 14. 26.0000000000	***********
• •	ROW 61 ***********************************	0. 0. 0.********* 0. 0. 0. 0. 0. 3. 42.**************	
• • •	ROW 42 ***********************************	••••••••••••••••••••••••••••••••••••••	
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Figure 23. Calibration error--matrix recharge version (computed 1974 head elevations-observed 1974 head elevations)

#### TILLMAN TERRACE ADUIPER FOR 1975 DESERVED SATURATED ABOVE FIVE FEET

	2 3 4 5 4 7	• •	10 1	1 12	13 14	15	16 17	18 19	20 21	22	. 13	24 2	25 .	26 27	28	29 3		32 3	
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NON	3 **********			•••••	•••••		•••••				oo.	-3.	-2.	-13.	-1.*		•••••		•••••
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RON	5 <b></b>				•••••				•••••		2. 1.	•.	-0.	-00.	۰.	2			•••••••
-				<b></b>			<b></b>		,	• •		-1.	-0.	02.	-2.	-1.***			
ROW	7 *********								•••••		so.	-1.	-1.	-10.	<b>-</b> •.	-3.***			
AD M											1.	-1.	۰.	-01.	-1.	-1			
ROW	• • ••••••			<b></b> 3.	10.	-2.	-0. 1.	1. 0.	-01	1	10.	٥.	z. '	21,	-4.	-1			
FOM	10			2. 3.	j. 1.	-1.	-1. 0.	10.	-24	:	ss.	->.	2.	-0).	-4	-16			
ROW				1. 2.	2. 3.		3. 3.	2. 1.	1. 2			-3.	i.	03.	-u	-26			
	12		o	22.	-1. 1.		1. 2.	2. 0.	20				٠.	-25.	-15.00				
804	13		2.	2. 1.	o. o.	1.	-12.	2. 3.	->0		D10.	-1.	-2.	-23.				******	
-				0. 2.	-01.		3. 1.	-33.	31		13.	۱.	ı.	-11.					
	15				23.		31.	-21.	2. 2	1	2.			19.					
~								-11.		1			-2.	14.					
					-1. 1.								•	-1. 2.	<b></b>				
			-0.								·· ··	-1.		-0					
*0*		•• •••	-0.		· · ·														
#DN	[,	-11.	۰.	1. 3.	3. 3.			11.											-
ROW	20 ************************************	-11.	-0	21.	•. •		0. ;3.	s. 2.	1. 1	• •	2. 1.		-3.				•••••		
ROW	21	ı. ı.	<b></b>	1. 3.	0. 1.	. 0.	13.	1. 2.	•• -0		2. 2.	-1.	1.		~				
ACM	22 ************************************	32.	-2.	o. 2.	20.	-0	••• -0.	1. 3.	2. 0		0. 2.	2.	-1.	-1			•••••		••••••
'AD N	23 ************************************	•. •.	1	02.	-50.	3.	-2.*****	-3. 0.	0. 1		40.	•-	-2. **			****			••••••••
ROV	24 ************************************	•. •.	3	24.	-41.	0.	0. 1.	-21.		••••	3. 3.	1.	3. ••	*******				******	•••••
PON	25	2. 2.	3.	15.	-2. 0.	. 2.	11	12.	-1. 1	••••	21.		3. ••			****	•••••		
<b>NOW</b>	26 ************************************	1. 2.	-0.	3. 1.	-11.	3.	-1+.•	-0.	•• •	·	20.		0. ••				•••••		
· RON	27		-2.	1. 0.	-1. 0.	1.	-41.*	1.			1. 0.	2.	3.00	•••••		••••	•••••	*****	
RON	24 ********* 4433.	1. 1.	-2	1. 1.	1. 0.	• •	51.	21.	-11	••••	11.	۰.	0.	02.	•••••	****	•••••		•••••
PON	29 ************************************	01.	-2.	o. 2.	1. 1.		2. 2.	4. 2.		11	00.		2.	12.		••••			••••••••
RON	30	01.	1	03.	2. 0.	. L	1. 1.	1. 1.	-0.	. (	0. 1.	-0.	-2.	-112.				•••••	
AD4	31 ************************************	-11.	۰.	12.	-1. 1.	. 2.	1. 0.	-15.	-00		20.	1.	- 3.	48.	•••••		•••••	•••••	
PON	32 ********* -011. *.	-2. 0.	1	1		• 2.	ı. ı.	oz.	0.	••••	ıı,	-9.	-0.	<b>4.</b> -11.			•••••	••••••	•••••
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		-0. 0.	-5,000											1816.		******			
ROW	34 ********* -0. 2. 23.	-0. 0. 00.	-5.000	1. 2.	-2		•••••		-2.	·	12.	0.**	•••••	••••••				•••••	
NON Row	34	-0. 0. 00. 20.	-1. -3.	1. 2. 01.	- 2. •••••				· -2. :	2: p:	12. 00.	0		·····			•••••	•••••	
ROM ROM	34 ************************************	-0. 0. 00. 20.	-3. -1.	1. 2. 01. 0. 2.	-2. •••• -22 -21				• -2. ; • 11 • 01	2: 0: 1. :	12. 00. 0. 5.	0		••••••			••••••		
ROM ROM ROM	34 4000000 -0. 2. 20. 35 40000000 -0. 2. 20. 35 4000000000000000000000000000000000000	-0. 0. 00. 20. -0. 0. 2. 3.	-1. -3. 4 1.	1. 2. 01. 0. 2. 11.	-2. •••• -22 -21 -00				2 • 1 1 • 0 1 • 21	2: 0: 1. :	12. 00. 0. 5.	0. <b>••</b> 1.••							
ROM ROM ROM ROM	3.         -0.         2.         2.         -0.           35	-0. 0. 00. 20. -0. 0. 2. 3. 3. 3.	-1. -3. 4 1 2	1. 2. D1. D. 2. 11. 10.	-22. -22. -21. -00.				2 1 1 • 0 1 • 2 1	2: 0: 1. : 0. :	12. 00. 0. 5. 2	0 1							
лон Ком Ком Ком Хом	3.	-0. 0. 00. 20. -0. 0. 2. 3. 3. 3. 3. 3.		1. 2. 01. 0. 2. 11. 10. 21.	-2 -22. -21. -00. -0. 0. 11.				2	2: 0: 1. : 2. : 2. :	12. 00. 0. 5. 2	0 1							
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			-J. -J. -J. J. -J. J. -J. -J. -J	1. 2. 2. 3. 4. 3. 4. 3. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4. 4.	-2, -2 -2, -2 -2, -2 -1, -0 -0, 0 1, -1 -1, 0 -1, 0 -1, -3 -1, -3 -3, -0 -3, -3 -3, -3, -3, -3 -3, -3, -3, -3 -3, -3, -3, -3, -3, -3 -3, -3, -3, -3, -3, -3, -3, -3, -3, -3,		-10, -0, -0, -1, -0, -0, -0, -0, -0, -0, -0, -0, -0, -0						0 2 0 1 2.  2.						
					-2, -2 -2, -2 -2, -2 -1, -0 1, -1 -1, 0 -1, -1 -2, -1 -1, -3 -1, -3 -1, -3 -1, -3 -1, -3 -3, -0 -3, -0 -		-10000000000						0						

Figure 24. Calibration error--pattern recharge version



This phenomenon occurs due to recharge from neighboring nodes along the hydraulic gradient. In some cases, where pumping automatically stops within five feet of the bottom, a node is recharged by neighboring nodes. Recharge from neighboring nodes was generally reoccurring because of changing gradients between nodes. Because the total drawdown (downward change in water-head elevation) was affected by recharge from adjacent nodes and the surface, it was difficult to determine surface recharge values to calibrate the model using the matrix version; therefore, permeability adjustments were needed for this calibration in some northcentral portions of the study area.

A new formula was introduced to determine a surface recharge rate for the calibrations. The effects of lateral flow between nodes was considered by comparing the effects of drawdown in two or more adjacent nodes. Calculation of a new recharge rate is as follows:

(1)  $\triangle Re_{New} = (DD_{N_1} * S_{\gamma_1} * C + DD_{N_2} * 0.01 * C)$ (2)  $\triangle Re_{01d} = (DD_{N_1} * S_{\gamma_1} * C + DD_{N_2} * S_{\gamma_1} * C)$ Subtracting (2) from (1) we obtain:  $\triangle Re_{New} - \triangle Re_{01d} = DD_{N_2} * (0.01 * C - S_{\gamma_1} * C)$  $= DD_{N_2} C (0.01 - S_{\gamma_1})$ 

Re = Recharge;  $DDN_{N_{1,2}}$  = Drawdown in nodes 1 and 2;  $S_{\gamma}$  = Specific Yield; The constant C is an emperical weighting factor. Simulation - 20 years.

Two 20-year computer runs were made for each of the three calibration versions as shown in Figure 5 (Flow Chart). The two runs included

simulation of prior appropriative right pumpage only and prior appropriative right pumpage in conjunction with allocations of 0.6 and 1.0 acre feet/acre/year, respectively.

The simulation period for the matrix and constant recharge versions was between October 1, 1969, and October 1, 1991. The third version was generated, using the pattern recharge approach, from July 1, 1973, to July 1, 1993. A new water-head matrix for July 1, 1973, was generated for the pattern recharge version after calibration was completed. This matrix is shown in Figure 25. An allocation of 0.6 acre feet/acre/year was used in the matrix and constant rechage runs while 1.0 acre feet/acre/ year was used for the pattern recharge.

Model simulation was mathematically based on computations of change in storage and corresponding water-head elevations of each node. These changes were simultaneously computed in order to represent the effects of lateral flow to and from adjacent nodes. The volume of water in each node was computed using Darcy's Law to calculate lateral flow (Q) at each node face:

Where:

$$\pm Q_n = T_n I_n W_n$$

 $Q_n$  is the lateral flow of water into (+) or out (-) of the node at the nth face; units in ft³ per second.

 $T_n$  is the average coefficient of transmissivity of two adjacent nodes; units in  $ft^2$  per second per foot.

 $I_n$  is the hydraulic gradient which was the difference in waterhead elevations (feet) in two adjacent nodes divided by the distance (2,640 feet) between the two nodes.

 $W_n$  is the width (2,640 feet) of the common  $n^{th}$  node face.

Figure 25. 1973 generated water-head elevation

PRIOR APPROPRIATION

													1	ATER HEAD	ELEW T	0									
		2	3	٠	•	• 7	•	•	10	11 12	13	14 15	14	17 18	19	20 2	1 22	23 2	• . *	26 2	28	29 3	0 31	32 3	ы <b>м</b> и
NO 14	2				*****			******	*** ***		******	*****	*****		******	*****		******	••••••				••••••	*******	***********
NOY	3	•••••			*****		*****				••••••	• • • • • • • • •					.1300.1	208.129	6.1299.1	1300.129	.1300.1	1310-192	7-1310-1	344.136	9.1341.1397
RON				****											** ** *12	97.129		295.129	5.1295.1	1296.1297	.1298.	301.131	9.1333.1	349.137	0.1384.1402.
NOV	•								*****			• • • • • • • • •				9.129	5.1294.1	295.129	5.1293.	1295.129	.1298.	299.231	5.1337.1	358.138	0.1392
•OH	,			****			•••••		•••••	•••••				·····	*****12	93.129	1.1293.1	294.129	.1295.	1292.1290	.1297.	1303.131	9.1346.1	377.***	
NON	•	•••••			****	•••••	••••	•••••••	••••12	71.1272.	1273.12	75.1277	1280.1	243.1285.	1287.12	0.129	0.1291.1	293.129	4.1295.1	1296.1291	.1 299.1	303.132	4.1362.4	*****	••••••
NON	٠	••••			*****	******		•••••1	268.12	70.1274.	1275.12	77.1274	1201.1	283.1284.	1286.12	87.128	.1290.1	292.129	.1297.	1298.1291	.1299.1	1910.133	9.1379.4		*****
NOV	10	•••••	•••••					***** 1	267.12	71.1273.	1275.12	74.1278	1280.1	282.1283.	1284.12	15.12 <b>8</b>	6.1288.1	292.129	a.1302.	1305.1304	.1306.1	324.135	5		
ROW	u	•••••		****	••••	•••••	*****	1264.1	265.12	70.1272.	1273.12	74. 1276.	1279.1	201.1283.	12 #5 .12	5.128	.1289.1	297.131	0.1314.	1314.134	5.1323.	1338. ***			********
ROM	12		******	*****				1262.1	266.12	67.1269.	1271-12	73.1275	1278.1	281.1284.	12 85.12		1.1294.1	1317.132	6.1328.	1328.132			••••••	*******	******
								1260.1	263.12	• 3- 1 2 6 6-	1267.12	71.1274	.1278.1	282.1285.	1289.12		4.1324.1	329.133	••1397. 2.1347.	1334.133					
	15			****		**1250.	. 1 2 5 3 .	1256-1	257.12	53-1258.	1262.12		.1284.1	293.1302.	1312.13	20.132		1340.134	3. 1340.	1335.133					
*0*	16					***1246.	.1251.	1249.1	249.12	53.1257.	1261,12	70.1282	.1293.1	305.1313.	1325.13		6.1342.I	1347.134	7.134z.	1336.133					****
ROW	17				***12	45.1246.	. 1249.	1249.1	247.12	49.1252.	1262.12	74.1286	.1302.1	316.1329.	.1336.13	44.134	8.1352.1	1353-135	2.1348.	1339.1331					*****
RON	18	•••••		••••	***12	.3.1246.	. 1247.	1247.1	244.12	45.1249.	1262.12	76-1290	1305.1	321.1334.	.1343.13	48.135	3.1354.1	1357.135	5.1352.	1350. ***			••••••		
ROW	19	•••••	••••••		•••12	.2.1244.	.1244.	1244.1	242.12	40.1245.	1257.12	72.1288	.1303.1	320.1335.	13,44.13	50.135	5.1363.1	342.135	<b>6.1355.</b>	1353	******				*****
RDW	20	•••••		•••• I i	39.12	•0.1240.	.1240.	1239.1	236.12	35.1236.	1252.12	<b>65.128</b> 0	.1296.1	316.1331.	.1343.13	49.135	5.1342.1	1363.135	9.1357.	1355.***			••••••	*******	
PON	21	•••••		37.LI	34.12	96.1235.	.1235.	1234.1	233.12	31.1236.	1246.12	60.1276	.1290.1	309.1328.	1340.13	47.135	3.1359.1	1362.136	1.1360.	1358	*****				*******
ROW	22	*****	*****12	34-13	34.12	33.1232.	1230.	1230.1	230.12	30.1234.	1245.12	58.1272	.1285.1	303.1322.	1336.13	44.135	0.1356.1	1360.136	1.1361.	1340	*****		*** ****		
ROW	23	•••••	•••••12	35.12	34.12	32.1237.	.1228.	1227.1	227.12	29.1233.	1246-12	57.1271	. 1283. 1	300.1316.	.1329.13	40.134	6.1352.1		0.1341.	••••••				*******	
*0¥	24				33.12	74.1225.	. 1726.	1225.1	225.12	29.1235.	1247-12	54.1244	.1283.1	295.1304.	. 1324.13	30.133	5.1340.1	1346.135	1.1355.						
ROW	26			30.12	31.12	24.1223	.1221.	1219-1	220.12	27.1234.	1241.12	54.1246	. 1278. 1	290.1300.	. 1313.13	27.133	3.1336.1	1342.134	9.1353.				******		
NON	27		1227.12	28.1	23.12	21.1219.	1219.	1217.1	221-12	27.1235.	1243.12	54.1263	.1275.1	286.1297.	.1314.13	25.133	1.1335.	1336.134	2.1348.		•••••		******		
ROW	28	•••••	1225.12	24.13	22.12	20.1218	.1218.	1216.1	222.12	27.1230.	1246.12	55.1266	. 1 2 7 4 . 1	286.1297.	.1310.13	23.132	8.1332.		7.1347.	1351.135			******		
ROV	29	•••••	1224.12	22.1	20.12	17.1217.	1218.	1217.1	221.12	28.1237.	1247.12	58.1269	.1279.1	289.1300.	. 13 15 . 13	21.132	7.1330.	1324.193	1.1335.	1334.134				*****	
POw	30	•••••	*****12	20.1	19.12	17.1217.	. 1218.	1218.1	222.12	29.1236.	1248.12	61.1275	. 1285.1	2 96 . 1 306	. 1313 . 13	18.132	1.1324.	1325.132	7.1325.	1325.132			•••	******	
ROH	31	•••••	1210.12	19.13	217.12	16.1217.	. 1 2 1 8 .	1219.1	224 - 12	30.1237.	1249.12	76.1294	.1302.1	308.1312.	.1309.12	<b>99.</b> 131	4.1316.	1319.132	1.1916.	1315.131				*******	*******
RON.	32	•••••	1216.12	15.13	214-12	14.1215	.1218.	1220.1	225.12	32.1243.	1263.12	83.1297	.1309.1	301.1331.	1296.12	96.130	4.1309.	1312.131	3.1311.	1309.130		••••••	******		
40×	33	•••••	1214.12	09. Li	10-12	12.1214.	.1217.	1220.1	225.12	37.1251.	1267.12	78.1293	.1295.1	295.1295.	1295.12	96.130	1.1306.	1308.130	7.1806.	1305.130		•••••••		*******	******
ROW	34		1212.12	10.1	208.12	10.1213.	. 1216.	1220.1	226.12	33.1239.	1250-12		.1295.1	295.1294.	1295.12	95.129	7.1248.1	1302.129	2						
-01					204.12	05.1208	. 1213.	1219-1	224.12	29.1235.	1239-12	50.1271	.1294.1		. 1294.12	93.129	1.1290.	1290.124							
*04	37				232.12	99.120.	. 1210.	1216.1	223.12	29.1234.	1239.12	47.1263	.1280.1	289.1293.	1294-12		6.1284.	1282.127				••••••	••••••		
ROW	38				••••11	98.1203.	. 1230.	1216.1	223.12	29.1234.	.1238.12	43.1254	. 1 2 7 0 . 1	275.1284.	1292.12	87-128	4.1282.1	1277.127	1			•••••	*****	*****	
ROW	39	•••••	*******		••••11	96,1194,	1205.	1214.1	222.12	29.1233.	1237.12	4 1 . 1245	.1256.1	273.1275	.1284.12	\$4.128	4.1280.	1273.126		•••••		•••••	••••		*****
#0¥	40	•••••		••••	••••	•••1192	. 1200.	1210.1	219.12	27.1232.	1236.12	40.1245	.1251.1	261.1273	. 1279.12		2.1275.	1256.126	·····	•••••••		******			
ROW	41	•••••	******	****		•••1190	. 1 19 8.	1200.1	215.12	24.1235.	1235.12	41.1248	.1257.1	263.1270	. 1272. 12	78.127	4.1267.	1262.126	0		••••••	••••••	*****	******	******
ROW	42	•••••	******	****			.1194	1201.1	212.12	21.1278.	1234.12	41.1249	. 1256. 1	261.1267	. 12 70 . 1 2	73.124	•.12 <b>6</b> 0.	1256.125	4	••••••	*****	•••••			
ROW	•3	•••••		****	•••••	***1185.	.1193.	.1201.1	209.12	18.1225.	.1231.12	39.1249	.1255.1	260.1265	. 1264.12	67.126	2.1251.	1249. ***	•••••	•••••	•••••••	••••••			******
ROV	45					***1177.	. 1104.	.1194.1	206-12	12.1219.	1227-12	38.1240	. 1259.1	258, 1251.	. 1744.17	34.123	2.1230-1	1238.000							
*04	44	•••••					. 1 1 0 2 .	1191.1	200.12	08.1216.	1226.12	34.1244	.1247.1	247.1241	.1227.12	19.121	4.1215.	1210.119							
PON	47	•••••	•••••	••••	•••••	•••1 175	.1176.	1107.1	197.12	04.1210.	1220.12	30.1239	. 1 2 4 5 . 1	245.1233.	. 12 15 . 1 2	08-120	5.1201.	1220.119	2						
ROW	4	•••••		***			.1175.	1185-1	194.12	01.1203.	1214.12	20.1230	.1234.1	225.1219.	. 1209 . 1 1		2.1190.	1189.118	5	•••••	•••	•••••	******		
ROW	49	•••••		****		***1 166.	. 1 1 7 3 .	1182.1	191.11	97.1201.	1206.12	05.1214	.1214.1	205.1203	1200-11	90.118	1.1176.	1177.117		•••••	••••••		••••••		******
PG¥	50	•••••	•••••	••••	••••	•••1164.	.1171.	1178.1	186.11	<b>91.</b> 1196.	1199.12	02.1203	.11%.1	190.1188	.1190.11	\$5.117	8.1172.	1169.117	0	•••••	••••••	• • • • • • •			
ROW	51	•••••	•••••	••••	*****	•••1162	. 1167.	1173.1	179.11	<b>#5.</b> 11?^.	1193.11	96.1196	.1193.1	189-1186.	.1176.11	70.117	0.1165.	1160.115	8.1167.	•••••	••••••	•••••	•••••	******	******
POX	52	•••••	•••••	****	••••	•••1160.	.1164.	1148.1	172.11	78.1182.	1186.11	88.1185	.1145.1	182.1176.	1172.11	69.116	4.1157.1	1153.114	9.1149.	•••••		*******	******		
	53			•••••	•••••	***1157.	. 1161.	1163.1	167.11	70.1175.	1178.11	*0.1176	.1174.1	172.1168.	.1164.11	•1.115	•.1152.		7.1146.		•••••••••			*******	
	· ··					•••1153.	. 1 1 54 -	1156.1	157.11	58.1158.	1159.11	58.1157	.1155.1	152.1148.	. 1145.11	-3-115 47.114	**********		9.1137.	1132-114	•••1146 -				
RDW	54					•••1150.	. 1151.	1151.1	152.11	53. 1153.	1152-11	51.1150	.1148.1	144-1142-	.1139.41	39.114	0.1139.		5.1132.	1129.114	i				
*0¥	57	•••••	•••••	••••	••••		.11•/.	1145.1	148.11	49.1148.	1147.11	47.1145	. 1 1 4 4 . 1	141.1139.		35.113	<b>4.1133.</b>	1132.118	0.1128.	1127.112	•	••••••			
#OH	58	•••••	•••••	••••			•••••	1143.1	144-11	<b>45.</b> 11 <b>4</b> 4.	1143.11	42.1141	.1139.1	197.1194.	.1.32.11	30.112	e.1127.	1 1 2 7 . 1 1 2	<b>*.</b> 1124.	1123.112		•••••	••••		
RDW	59	•••••	·	••••	••••		•••••	1142.1	142.11	<b>41.</b> 1140.	1140.11	38. 1136	.1124-1	131.1128.	.1126.11	23.112	2.1122.	. 122.112	1.1120.	1129.111	•.••••	••••••			•••••
RON	60	•••••			••••	•••••	•••••	1140.1	138.11	36.1137.	1136-11	34.1131	.1129.1	125.1118.		14-111	2.1116.	1116.111	5.1114.	1114.111	• • • • • • •	******	*** ****		*****
80¥	6L	•••••	••••••		••••	••••		•••••	••••11	34.1133.	1132.11	30.1125	. 1 1 2 2 . 1	120.1119.			0.1109.1	1128-110	7.1109.	1108.110		******	******		· • • • • • • • • • • • • • • • • • • •
ROW	62	• • • • •	•••••	••••	••••	******	•••••	•••••	*****	• •• • • • • • •				********	** ** **	•••••	******		******	1198.000	••••••	*****	****		******

Transmissivity is calculated in the model by multiplying the value of permeability by the saturated thickness of each node. The distribution of transmissivity values is shown in Figure 26. The net flow into or out of each node is the algebraic sum of the lateral flows determined for the node faces as well as outflow due to pumpage and evapotranspiration and inflow due to surface recharge (calibration).

The computed values for inflow and outflow are summarized as a mass balance inTables VII, VIII and IX. A conceptual input-output model is shown in Figure 27 to represent the relationship between parameters used in the mass balance. The mass-balance tables are computed for each model time step. A model time step of ten days was used for each set of computer calculations of head change. The head change was computed by using the following relationship:

Where:

$$\pm \Delta H_n = (Q_{net, n, t=1} - Q_{net, n, t=2})$$
 Sy

 $\Delta H_n$  = change in water-head elevation (drop = - and rise = +); units are in feet.

Qnet,n, t=1,2 = net flow into or from each nth node as computed at the end of consecutive time steps (t); units are in cubic feet.

Sy = Specific Yield; unitless.

The above relationship was used in sets of simultaneous equations for all nodes during each model time step. Subsequently, a relaxation procedure was used to adjust the resulting head elevations (former head  $\pm \Delta H_n$ ) to within a model error of 0.1 foot.

Figure 26. Transmissivity distribution map



# TABLE VII

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### PRIOR APPROPRIATION ONLY, OCTOBER 1, 1974 - OCTOBER 1, 1991 CONSTANT RECHARGE

		Average An Acre Ft	nual		<b>ا</b>	「otal Acre Ft	
	In		Out		In		Out
Pumping	0		-16,737		0		-267,799
Leakage	724		- 8,227		11,580	•	-131,630
Constant Flux	91		- 68		1,458		- 1,083
Evapotranspiration	0		- 3,091		0	•	- 49,462
Recharge	34,481		•••••••••••••••••••••••••••••••••••••••		551,701		0
Return Flow	2,511		0		40,170		. 0
TOTAL	37,807		-28,123		604,909		-449,973
Net Storage Change		+9,684		•		+154,936	

# TABLE VIII

# PRIOR APPROPRIATION, JULY 1, 1973 - JULY 1, 1993 PATTERN RECHARGE

	Aver A	age Annual cre Ft		Total Acre Ft				
	In	<u>Out</u>	In	Out				
Pumpage	0	-23,923	0	-478,453				
Leakage	1,114	- 8,465	22,285	-169,299				
Constant Flux	91	- 68	1,823	- 1,354				
Evapotranspiration	0	- 5,799	0	-115,981				
Recharge	45,760	0	939,310	0				
Return Flow	3,588	0	71,768	0	•			
		· · · · · · · · · · · · · · · · · · ·	· · · · · · · · · · · · · · · · · · ·					
TOTAL	50,553	-38,255	1,011,078	-765,087				
Net Storage Change	+	12,298		+245,991	· .			

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### TABLE IX

# TILLMAN TERRACE MASS BALANCE, PRIOR APPROPRIATIVE + 1.0 ACRE FT/ACRE ALLOCATION JULY 1, 1973 - JULY 1, 1993 (PATTERN RECHARGE)

*,* 

	Averag Acr	e Annual e Ft	Total Acre Ft						
	In	Out	In	Out					
Pumpage	0	-93,971	<b>0</b>	-1,879,420					
Leakage	17,205	- 634	344,105	- 12,685					
Constant Flux	91	- 68	1,823	- 1,350					
Evapotranspiration	0	- 302	0	- 6,035					
Recharge	45,760	0	915,202	0					
Return Flow	17,096	0	281,913	0					
TOTAL	77,153	-94,975	1,543,043	-1,899,490					
Net Storage Change	-17	,822	-3	356,447					

# Figure 27. Mass balance distribution

#### MASS BALANCE DISTRIBUTION

#### TILLMAN TERRACE



### CHAPTER V

### RESULTS

Calculated saturated thickness for the three versions were compared for October 1, 1991, (matrix and constant recharge) and July 1, 1993, (pattern recharge). The resulting saturated thicknesses were subdivided into ranges as shown in Figures 28, 29, 30, 31, 32 and 33. The nodes which fall in zone 6 (saturated thickness range from 0 to 5.49 feet) are considered dry because of the assumed pump position at the bottom of the well.

The percentage of the total nodes (1,186) are calculated for each version. They are listed in Table VII. As expected, the percent of dry nodes is directly proportional to the additional allocations.

Results shown in Table X indicate that the constant and calibration matrix versions yielded similar results. However, the prior appropriative run, using pattern recharge, did reduce the dry area by 14 percent to 21 percent. This suggests that the pattern recharge calibration procedure produces optimistic results. A 24 percent to 25 percent increase in dry area is caused by the one acre foot per acre allocation as compared to the constant and matrix versions using 0.6 acre feet per acre annual allocation. Although the constant and matrix recharge simulation runs were terminated on October 1, 1991, instead of July 1, 1993, test runs indicate that only a small difference in results would be produced by extending the simulation time to July 1, 1993.

Figure 28. 1991 calculated saturated thickness prior rights only (matrix recharge)



Figure 29. 1991 calculated saturated thickness--prior rights plus 0.6 acre ft/acre annual allocation (matrix recharge)



Figure 30. 1991 calculated saturated thickness--prior rights only (constant recharge)



Figure 31. 1991 calculated saturated thickness--prior rights plus 0.6 acre ft/acre annual allocation (constant recharge)



Figure 32. 1993 calculated saturated thickness--prior rights only (pattern recharge)



Figure 33. 1993 calculated saturated thickness--prior rights plus 1.0 acre ft/acre annual allocation (pattern recharge)


### TABLE X

#### COMPARISON OF CALCULATED SATURATED THICKNESS AND DRY NODE PERCENTAGES FOR THREE RECHARGE VERSIONS OF COMPUTER SIMULATION

#### OCTOBER 1, 1991--CALCULATED SATURATED THICKNESS, % DRY NODES FOR (0-5.49) FT SATURATED THICKNESS INTERVALS 1 Constant Recharge, Prior Appropriation $\frac{211}{1186}$ = 18% $\frac{574}{1186}$ 2 Constant Recharge, Prior Appropriation + 0.6 acre ft/acre/year = 48% <u>301</u> 1186 Calibration Matrix, Prior Appropriation 3 Ξ 25% $\frac{580}{1186}$ 4 Calibration Matrix, Prior Appropriation + 0.6 acre ft/acre/year 49% =

JULY 1, 1993--CALCULATED SATURATED THICKNESS, % DRY NODES FOR (0-5.49) FT SATURATED THICKNESS INTERVALS

5 Pattern Recharge, Prior Appropriation 6 Pattern Recharge, Prior Appropriation + 1 acre ft/acre/year*  $\frac{861}{1186} = 73\%$ 

* % dry nodes for (0-5.01) ft saturated thickness intervals = 50%

The pattern recharge version was selected as the version to represent the final results (Figure 33). These results show that the areas in the northeast corner, along the North Fork of the Red River, became dry due to subsequent allocated pumping. The average saturated thickness for the year 1973 was 18.4 feet as compared with the remaining 7.1 feet of saturated thickness in 1993 as inferred from the pattern recharge version in Figure 33. Computed initial and final areas and volumes of water which were determined from the pattern recharge version are shown in Table XI.

Maximum annual yield was determined by adjusting the amount of allocated pumpage which would cause 50 percent of the nodes to go dry by the end of the simulation period (pattern recharge version). Several simulation runs were made to obtain the 50 percent dry area. This is shown graphically in Figure 34. The maximum annual yield was determined to be 70,048 acre feet per year using a pumping allocation of 1 acre foot/ acre/year. This value was produced by dividing the total pumpage (1993) by the period of simulation of 20 years. A 20-year sequence of areas which became dry are shown in Figures 35, 36, 37, 38 and 39 for the pattern recharge version. The final depth (1993) to the water table is shown in Figure 40. Dry nodes were found in the central and northcentral part of the study area and in areas which are closer to the bedrock boundary. The areas along the river generally remained wet with little change in saturated thickness and transmissivity. Recharge from the river to the nearby nodes contributed to the recharge of the area. Few of the appropriated right owners would go dry during the 20-year period; however, additional allocation was permitted and more of their wells went dry as expected. Figures 41, 42 and 43 indicate the areas where prior

### TABLE XI

	· · ·	· · · · · · · · · · · · · · ·	•••••	·····		
SATURATED THICKNESS	% AREA	AREA	AVERAGE SATURATED	SPECIFIC YIELD	STORED WATER	
(FEET)		(ACRES)	(FEET)		(AC.FT.)	
0-10	26.3	49,920	5.1	.3	77,066	
10-20	29.9	56,800	15.6	.3	265,371	
20-30	28.8	54,560	24.6	.3	401,874	
30-40	12.1	23,040	33.8	.3	233,571	
40-50	2.8	5,280	42.4	.3	67,146	
50-60	0.1	160	50.0	.3	2,400	
TOTALS	100.0	189,760	18.4 (AVE.)	- 1	,047,429	

### INITIAL MASS DATA JULY 1, 1973

MASS DATA FROM SIMULATION RUN USING PATTERN RECHARGE JULY 1, 1993 1.0 AC.FT./AC./YR. AND/OR PRIOR APPROPRIATION

0-10	86.7	164,640	4.9	.3	242,140	
10-20	7.5	14,240	14.4	.3	61,629	
20-30	3.2	6,080	25.0	.3	45,614	
30-40	1.7	3,200	35.3	.3	33,877	
40-50	0.8	1,600	43.9	.3	21.073	
50-60	-	-			-	
	100.0	100.0 189,760			404,332	
		•			•	

Figure 34. Cumulative percentage of dry area using the pattern recharge version



# Figure 35. Dry areas in 1973



## Figure 36. Dry areas in 1978



Figure 37. Dry areas in 1983



## Figure 38. Dry areas in 1988



## Figure 39. Dry areas in 1993



# Figure 40. 1993 water depth from the surface



Figure 41. Prior rights affected by annual allocation of 0.6 acre ft/ acre (constant recharge)



Figure 42. Prior rights affected by annual allocation of 0.6 acre ft/ acre (matrix recharge)



Figure 43. Prior rights affected by annual allocation of 1.0 acre ft/ acre (pattern recharge)



appropriative owners would go dry due to additional allocations. Approximately 31 percent, 33 percent and 69 percent of the prior appropriative owners would be adversely affected according to constant, matrix and pattern simulation runs, respectively.

A well spacing of one-half mile was determined using the pump test results and subsequent model simulation using the pattern recharge option. This is shown in Figure 44. A rate of one acre foot/acre/year was applied to a single well within each one-quarter section of a square mile (160 acres) and a 2,640 foot spacing between wells. The pump rate was equivalent to a continuous pumping of 100 gpm for one year or 300 gpm continuous pumping for four months during the irrigation season. Smaller spacings and pumping rates are recommended when more than one well is used in a single quarter section.

Simulated water-head elevations for 1973 and 1993 are shown in Figures 45 and 46. Comparison between 1969 head elevations and simulated head elevations indicate a significant decline in water level. Assessment of the predicted 1993 saturated thickness maps in Figure 33, and the water-head elevation maps, indicate that the possibility of pollution from high salt ( $CaSO_4$ ) concentrations in the river exist within one and one-half miles of the North Fork of the Red River tributary and the Red River. This would result from an influent condition (gradient away from river) created by pumpage near the rivers where effluent conditions (gradient toward the river) currently exist.

Figure 44. Recommended well spacing for maximum annual yield

			• → 300 gpm	gpm
Determinat	ion of Maxinum Annual Y	ield		
		133	← 1320'→ ← 2640'→	
		0	Cross Section of Expected well Drawdown	
1320'	2640'	- 1320' > A	Maximum Annual Yield (4 mo.) Indicated by Pump Test	
			Data and Model Results	
			Well Spacing for	Well Spacing for
		264	by 1/4 section	by 1/8 section. 330'
		0		• • • • • • • • • • • • • • • • • • • •
			<b>6 € 1 3 2 0 € 6 6 0 1</b>	
+-+		{	1320	
		13		
		20'	<b>0 1 6 6 1 1 1 1 1 1 1 1 1 1</b>	
	5,280'	>	2640'	<b>←</b> 2640' <b>──</b> →
	Maximum Annual Yield		Mayimum Annual Vield	Maximum Annual Viald
	Pump Rate: One well	1/4 sec.	Pump Rate: 4 wells/1/4 sec.	Pump Rate: 16 wells/1/4 sec
	100 gpm/well (1 300 gpm/well (1/	year) 3 year)*	25 gpm/well (1 year) 75 gpm/well (1/3 year)*	6 174 gpm/ well (1 year) 18.75 gpm/well (1/3 year)*
* Pun	npage during irrigation	season only (J	une 1, to September 30)	

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#### RECOMMENDED WELL SPACING FOR MAXIMUM ANNUAL YIELD TILLMAN TERRACE

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### Figure 45. 1973 water-head elevations



JULY I, 1973 WATER HEAD ELEVATION CONTOUR INTERVAL = 10' Figure 46. 1993 simulated water-head elevations (pattern recharge only)



### CHAPTER IV

#### SUMMARY AND CONCLUSIONS

Computer simulation is an effective tool for determining the maximum annual yield for the ground-water basin (aquifer) in the western half of Tillman County. The prior appropriative right owners and those who are allocated one acre foot/acre/year between July 1, 1973, and July 1, 1993, can use a total rate of 70,048 acre feet per year. The mass balance is summarized in Figure 47. A total volume of 1,400,967 acre feet represents the cumulative amount of water pumped from storage at a discharge rate of one acre foot per acre during a 20-year period. A cumulative volume of 478,453 acre feet is pumped by the prior appropriative owners during the same period. A cumulative volume of 2,288,486 acre feet is stored over the 20-year period. A ground-water storage of approximately 1,047,429 acre feet is computed to have existed in 1973. The computations are shown in Table XI. An additional 1,241,057 acre feet was accumulated due to recharge, especially during the non-pumping periods (8 months per year), as well as to river leakage and boundary flow from the north edge of Tillman County. A recovery factor of 81 percent of cumulative aquifer storage is also computed in Figure 47. This percentage represents the amount of ground water pumped from cumulative ground-water storage as of July 1, 1993. The recommended well spacing and corresponding pump rates are shown in Figure 48.

The reliability of the output results of the model, however

Figure 47. Mass balance distribution and recovery factor for maximum annual yield (pattern recharge version)

#### MASS BALANCE DISTRIBUTION





Figure 48. Recommended well spacing for maximum annual yield

		TILLMAN	TERRACE			•
-Well Spac	ing-used for		אר אר 300 gpm	ar → 300 gpm	A'	
Determina	tion of Maxinum Annual	Yield				• • • ·
		1320	<			
1320'	2640'	$V = \frac{1320'}{A'} $	Section of Expected well D ximum Annual Yield (4 mo Indicated by Pump Test	rawdown )		······································
·			Data and Model Results Well Spacing for Proportionate pumping by 1/4 section	· · · · · · · · ·	Well Spacing Proportionate by 1/8 secti	for
		640'	1320'		0 0-	660'
					0 - 0	- o o
		1320				
	5,280 Maximum Annual Yiel Pump Rate: One wel 100 gpm/well (1 300 cpm/well)	d 1/1/4 sec. year) /3.vear)*	Maximum Annual Yield Pump Rate: 4 wells/1/4 25 gpm/well (1 year 75 gpm/uell (1) a	sec. P	264 aximum Annuel ump Rate: 16 6 1/4 gpm/ 8,75 gpm/uel	Yield wells/1/4 s well (1 yea
* Pu	mpage during irrigation	season only (June 1, to	\$eptember 30)			· · · · · · · · · ·
1 1 1						1 1

#### RECOMMENDED WELL SPACING FOR MAXIMUM ANNUAL YIELD TILLMAN TERRACE
sophisticated, can only be as good as the data input upon which they are based. It is, therefore, essential in the modelling process to devote considerable care to the collection, interpretation and validation of these data.

In conclusion, the principal parameters from which the maximum yield was determined are:

 The total land area is 189,760 acres overlying the terrace and floodplain deposits in the aquifer (basin);

(2) The volume of water in storage in the aquifer as of July 1,1973, is 1,047,429 acre feet; the cumulative volume of water in storagefor 20 years is 2,570,399 acre feet;

(3) The estimated rate of natural recharge is 2.87 inches per year based on calibration results;

(4) The average specific yield for the basin is 0.30; and

(5) An average transmissivity (1973) is 13,230 gpd/ft.

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## APPENDIX

# COMPUTER LISTING OF DATA INPUT FOR GROUND-WATER MODEL (PATTERN RECHARGE VERSION)

//GWMDL EXEC PGM=LDADER, // PARM='SIZE=600K', REGION=600K /*JOBPARM K=0 ND PAGING /*JOBPARM F=9001,N=2 //SYSLIB DD DISP=SHR, DSN=SYS1.FORTLIB //SYSLOUT DD SYSDUT=A //SYSLIN DD DISP=SHR. // DSN=OSU.ACT11236.AQUIFER1(GWMDL26) // DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(MNPROG26) // DD DISP=SHR. // DSN=OSU.ACT11236.AQUIFER1(DATAI26) // DD DISP=SHR . // DSN=DSU.ACT11236.AQUIFER1(COMPUT25) // DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(COEF25) // DD DISP=SHR. DSN=OSU.ACT11236.AQUIFER1(CHECKI25) 11 // DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(PRNTAI) // DD DISP=SHR, // DSN=DSU.ACT11236.AQUIFER1(BLDATA25) // DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(PLTXI) // DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(PLTEI) VI DD DISPESHR, // DSN=OSU.ACT11236.AQUIFER1(PLTTI) // DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(PLTAI) // DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(PLTUI) // DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(PLTFI) //FTO6FOOI DD SYSOUT=A //FT08F001 DD SYSDUT=A,DCB=RECFM=UA //FT51F001 DD SYSOUT=A,DCB=RECFM=UA //FT52F001 DD SYSDUT=A,DCB=RECFM=UA //FT60F001 DD SYSDUT=A,DCB=RECFM=UA //FT90F001 DD DISP=SHR, // DSN=OSU.ACT11236.AQUIFER1(PLTDF) //FT22F001 DD DISP=OLD, // DSN=TEMP.ACT11236.FILE1 //FT20F001 DD DISP=(NEW,CATLG), // DCB=(RECFM=FB,LRECL=80,BLKSIZE=2480),

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INPUT DATA FILE FOR CONTINUATION

OUTPUT DATA FILE

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12601270	1270127512	7512771280	12851305133	61377	12501257125012401	240124012521240
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13001305	1325133013	3013281325	12461246124	612451235	123012351251126012	270128012951300
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131413201	1320133013	3013261330	12441241124	012351225	12301235125512701	289129913091310
13201320	1320132713	12411241	12381237123	012201230	124512441268127512	292130313101315
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## Abdulaziz Jasem Al-Sumait

VITA

Candidate for the Degree of

Master of Science

# Thesis: A COMPUTER GROUND-WATER MODEL FOR THE TILLMAN ALLUVIUM IN TILLMAN COUNTY, OKLAHOMA

Major Field: Geology

Biographical:

Personal Data: Born in Kuwait, March 2, 1946, the son of Mr. Jasem Mohammad Al-Sumait.

Education: Graduated from Shuaikh Secondary School in June, 1964; received Bachelor of Science degree from Northern Arizona University, Flagstaff, Arizona, in May, 1970, with a major in Geology; completed requirements for the Master of Science degree at Oklahoma State University in December, 1978.

Professional Experience: Employed in the Ministry of Electricity and Ground-Water Department in Kuwait as a geologist between 1970 and 1972, and again from 1973 to 1975; attended six-month course in hydrology in Padova, Italy, from January, 1973, to June, 1973.